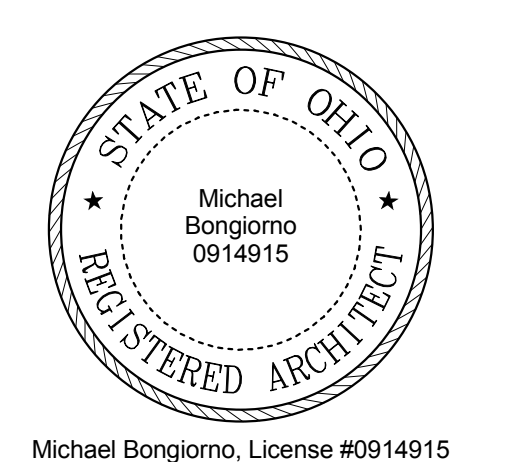




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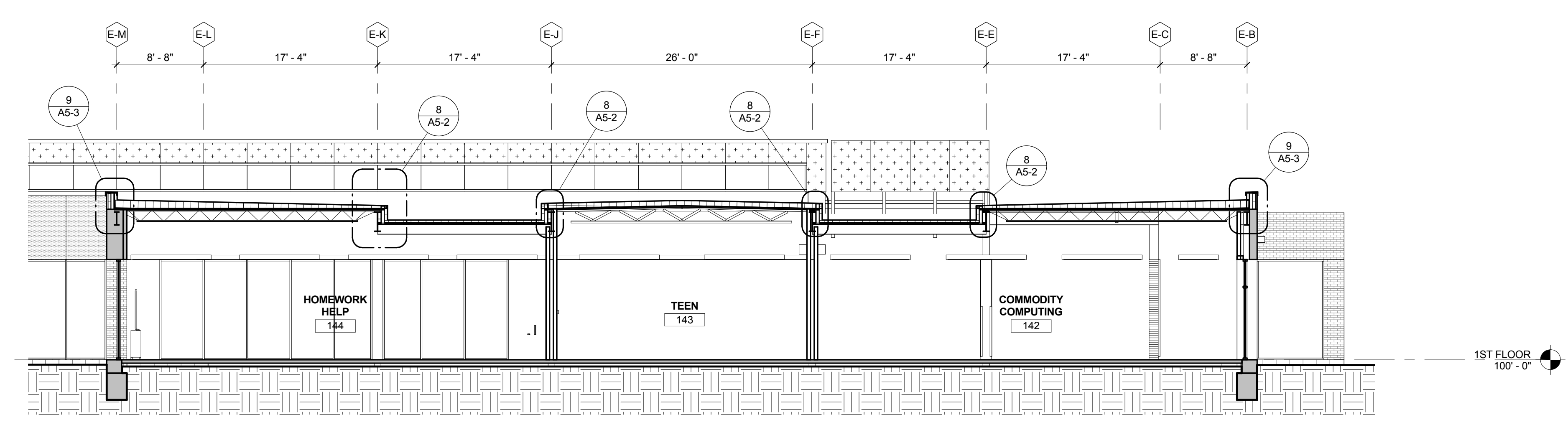


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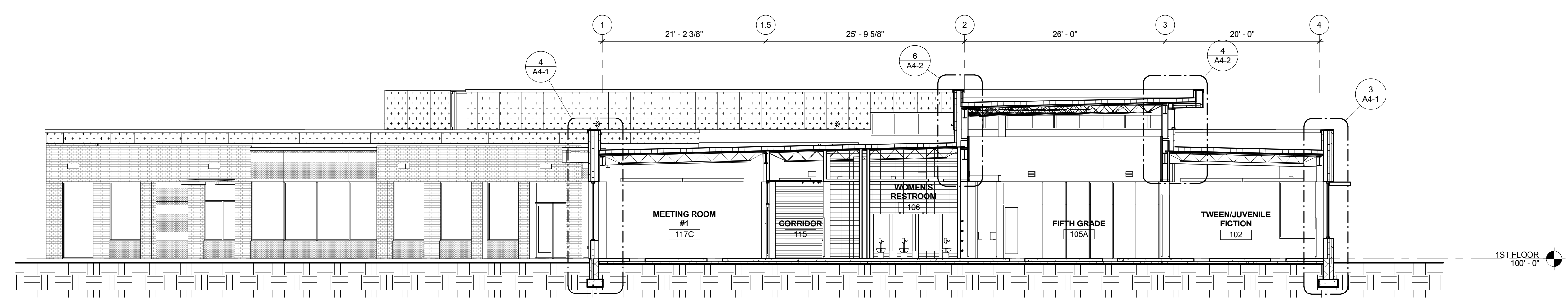
LANDSCAPE ARCHITECT
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462 South Ludlow Alley phone 614 621 2709
Columbus, OH 43215 fax 614 621 3004

CIVIL AND MEP ENGINEER
Kongb
1650 Watermark Dr. phone 614 487 1650
Columbus, OH 43215 fax 614 487 8881

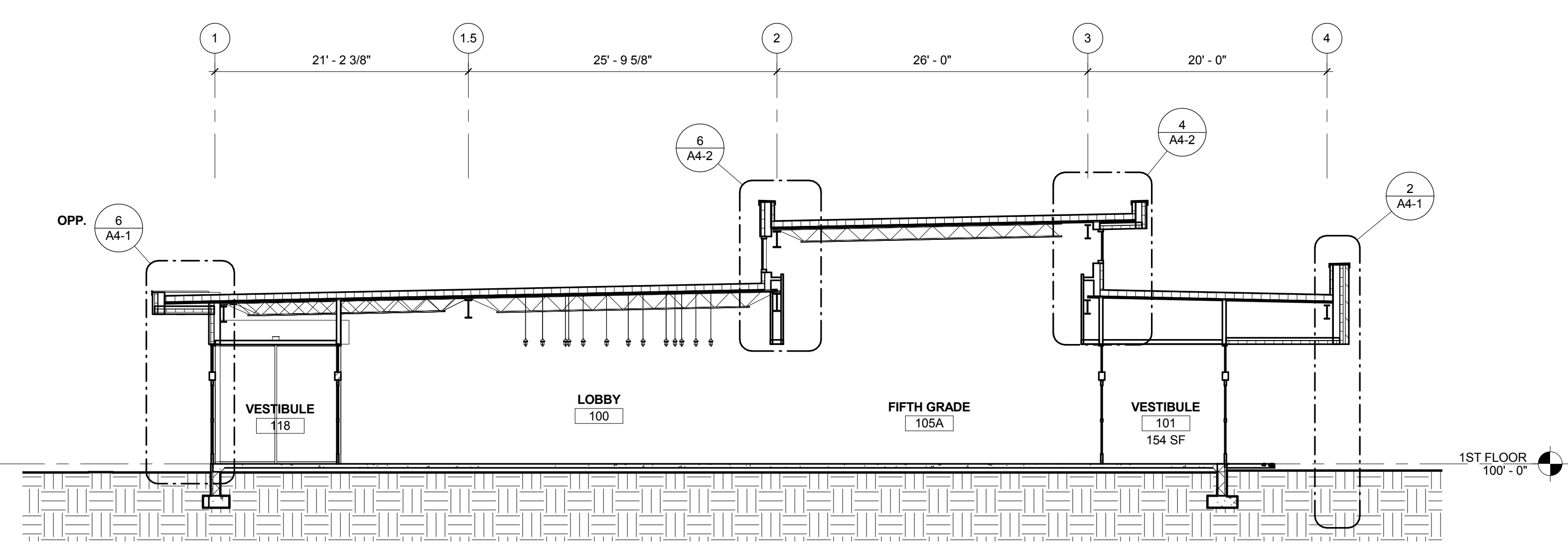
STRUCTURAL ENGINEER
SMSH
1166 Dublin Rd., Ste 200 phone 614 481 9800
Columbus, Ohio 43215-1038 fax 614 481 9303



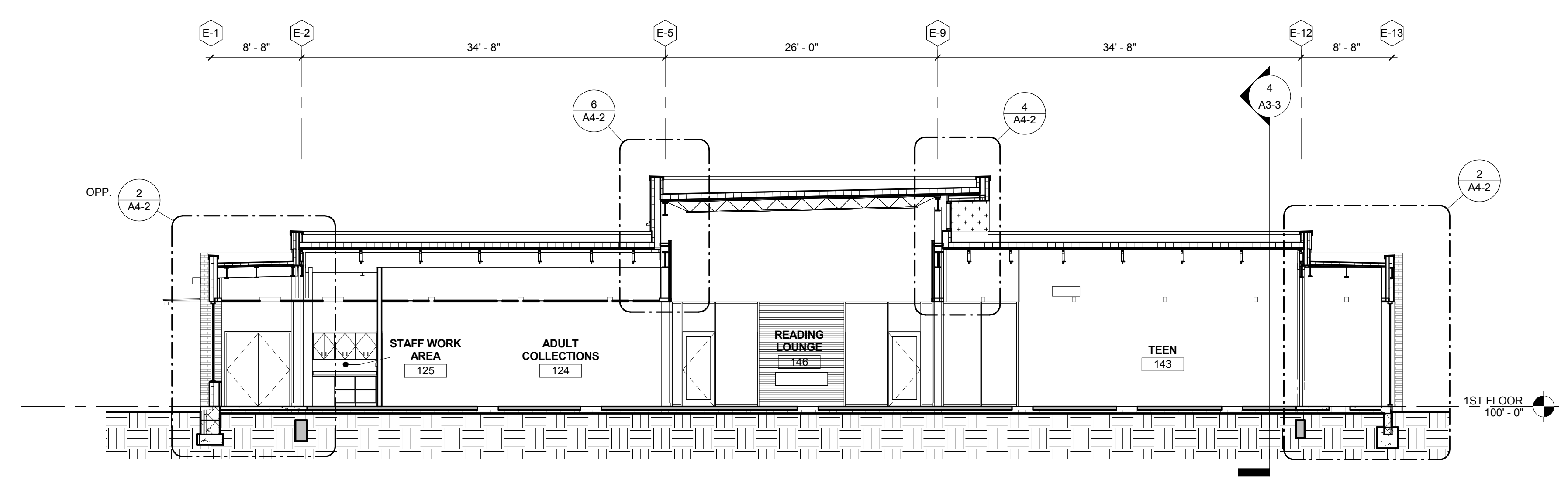
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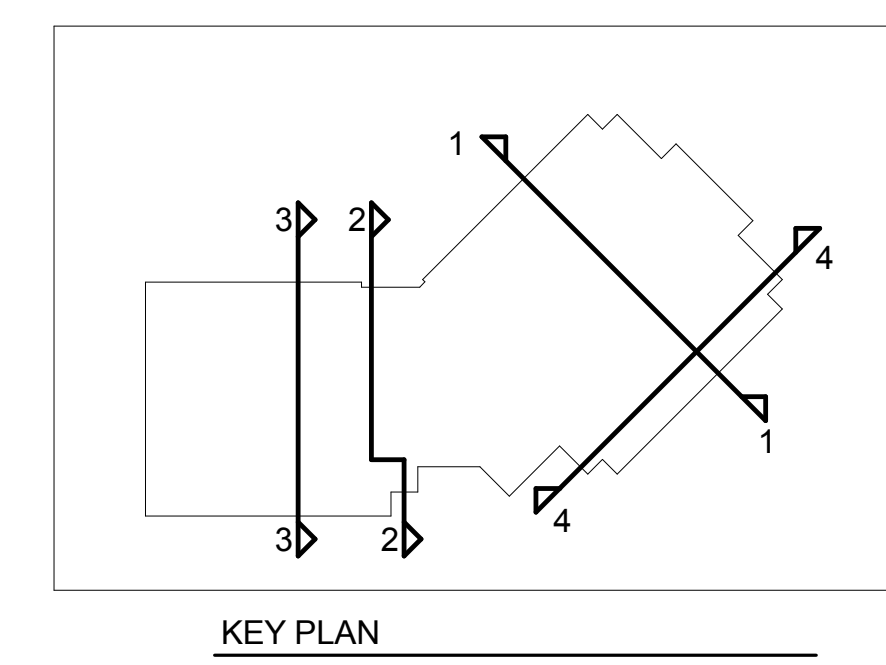
SECTION THROUGH NEW ADDITION 3
1/8" = 1'-0" A3-3



SECTION THROUGH LOBBY 2
1/8" = 1'-0" A3-3



SECTION THROUGH EXISTING BUILDING 1
1/8" = 1'-0" A3-3



KEY PLAN

DATE DESCRIPTION
CLIENT

COLUMBUS METROPOLITAN LIBRARY
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BUILDING SECTIONS

commission no. 1333.00
scale: As indicated
date: 06.25.15
revised:

BID DRAWINGS

A3-3

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1 10 Mar 2016 Bulletin 035

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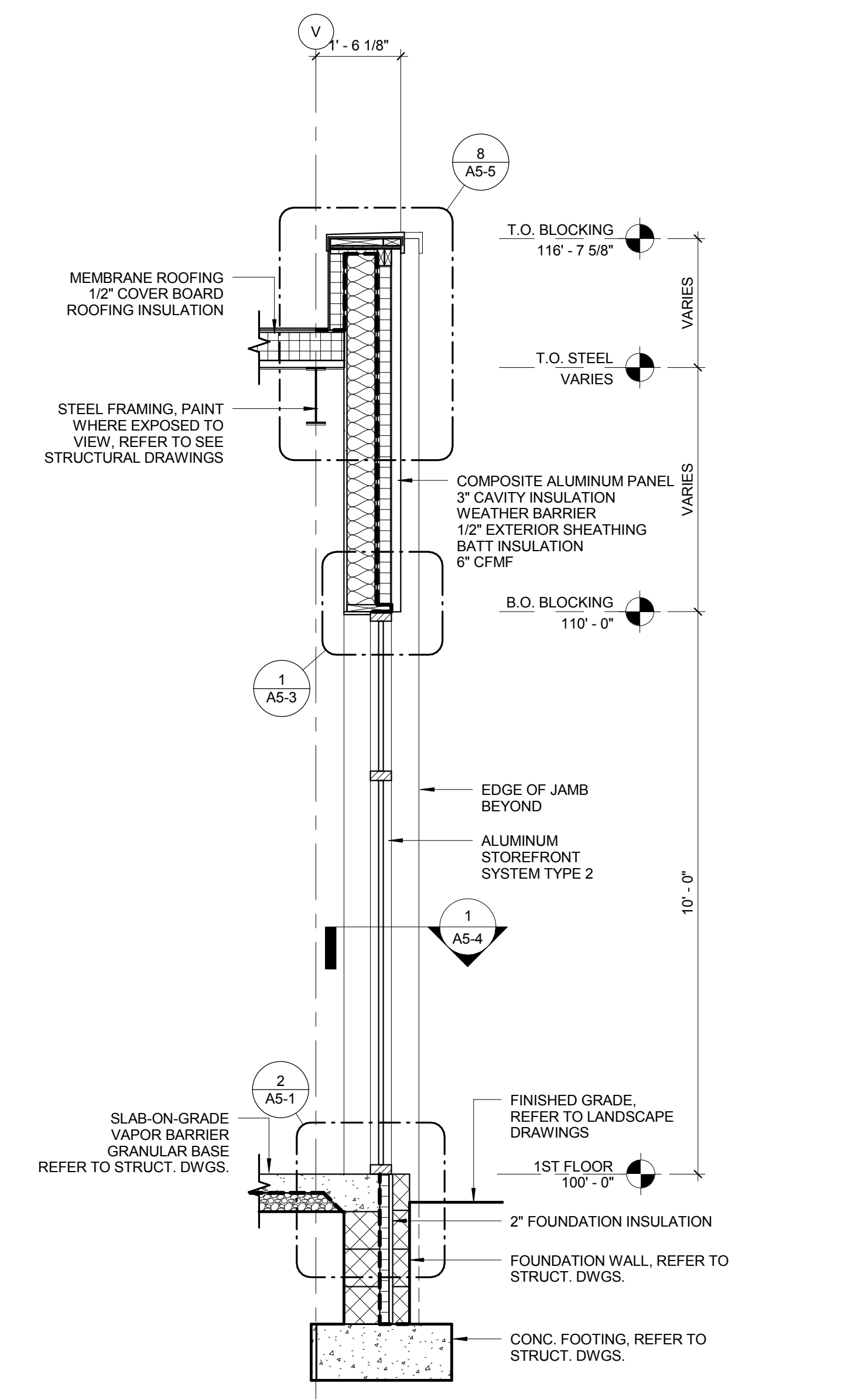
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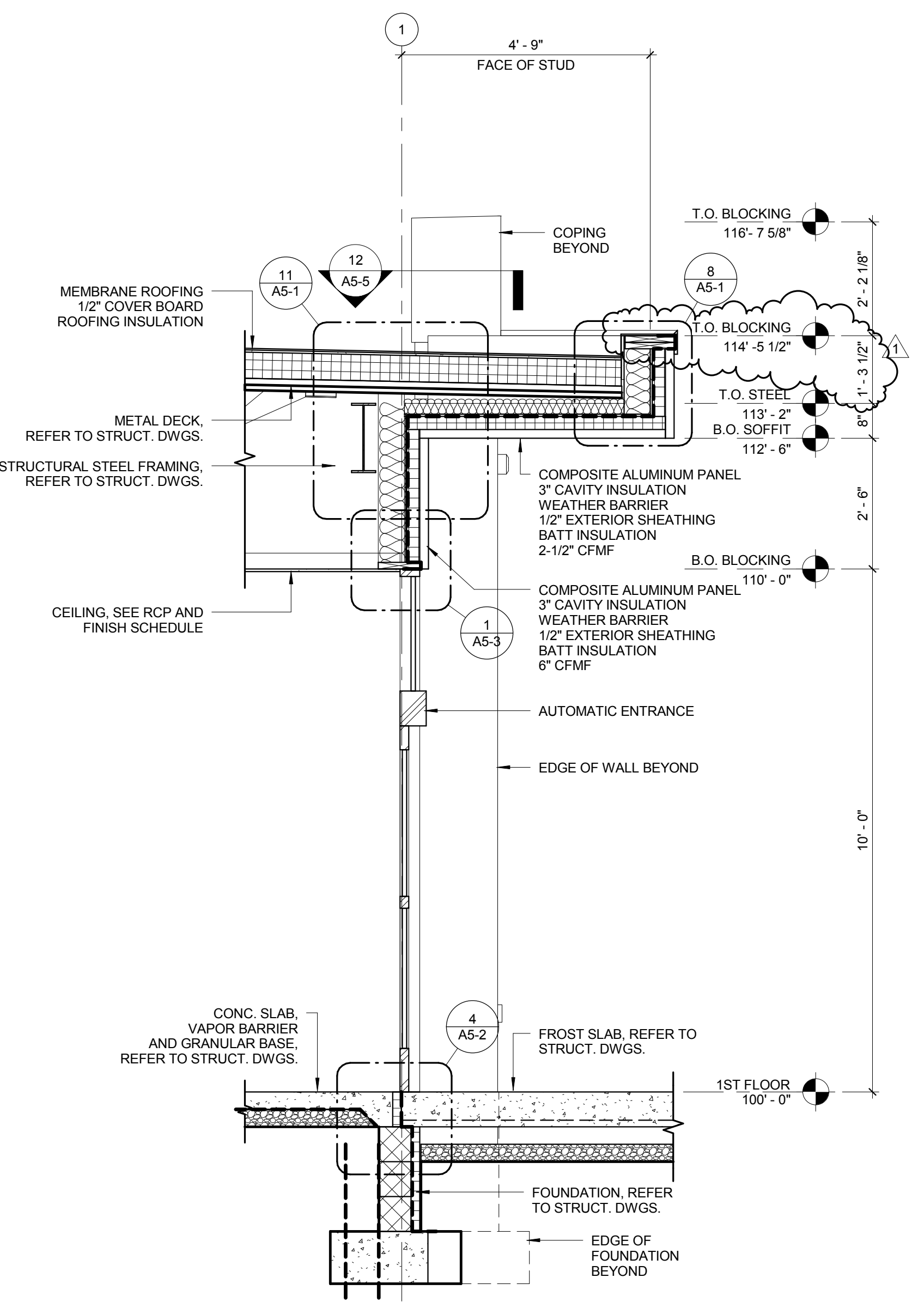
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scale: 1/2" = 1'-0"
date: 06.25.15
revised:

BID DRAWINGS

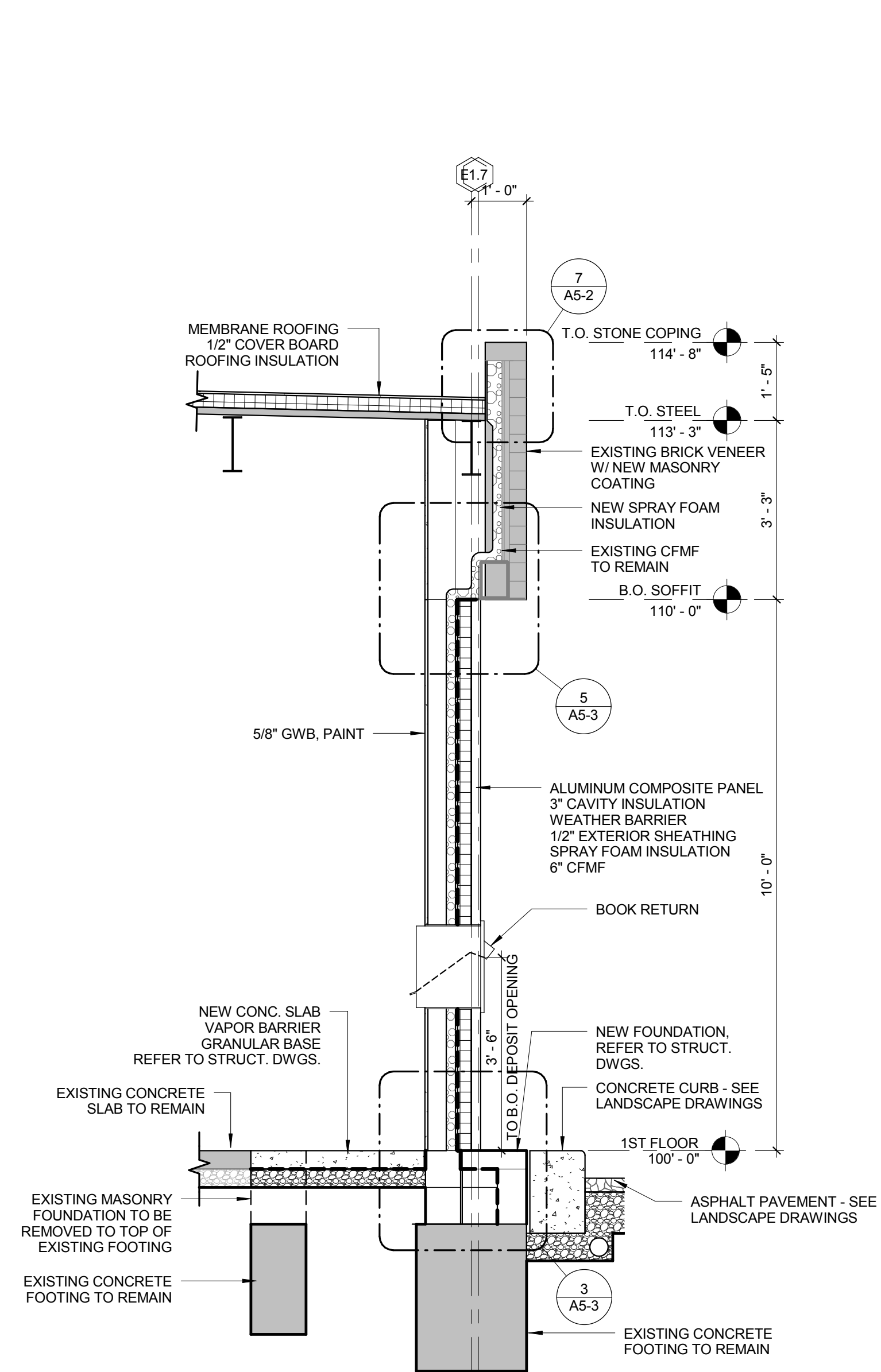
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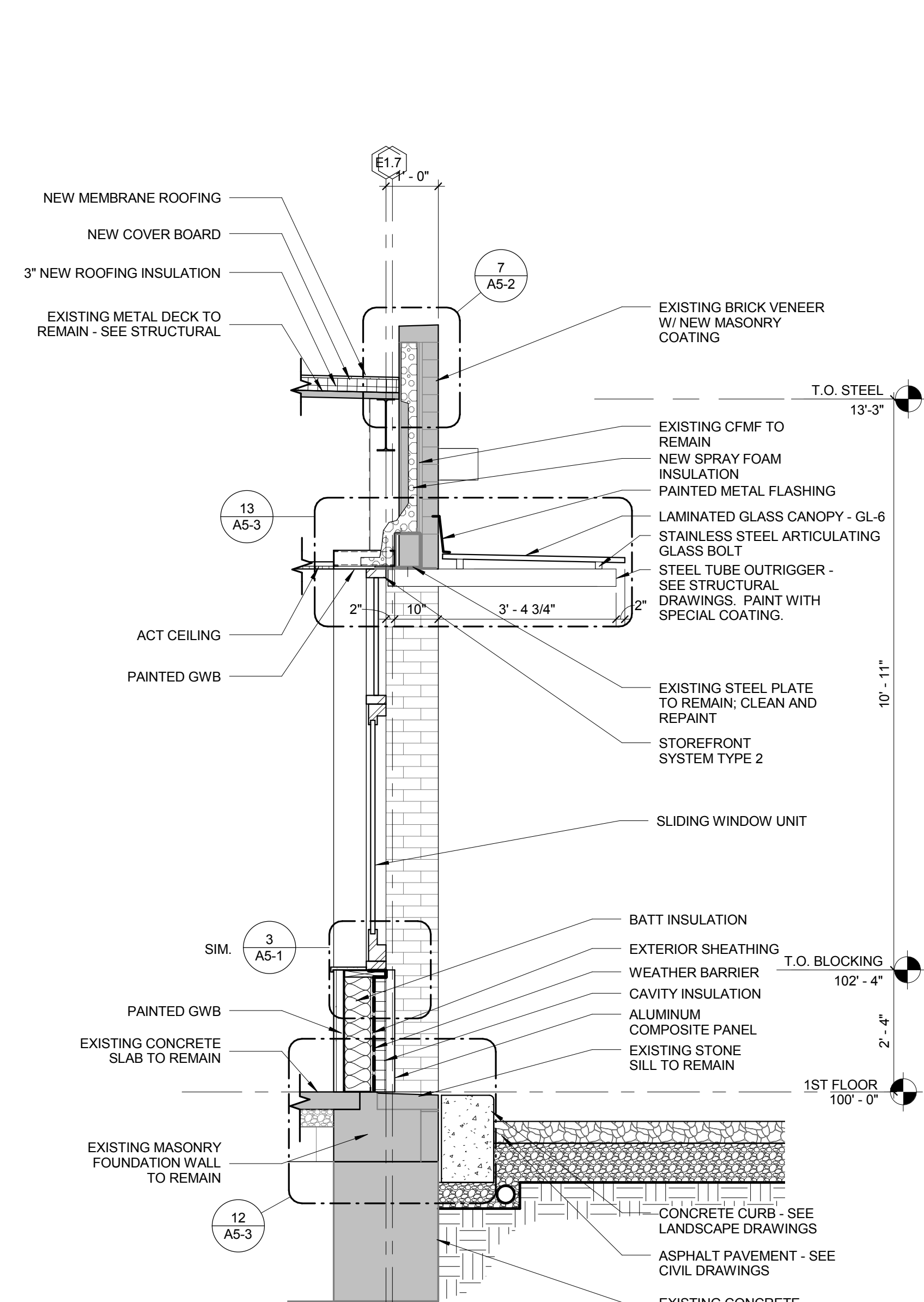
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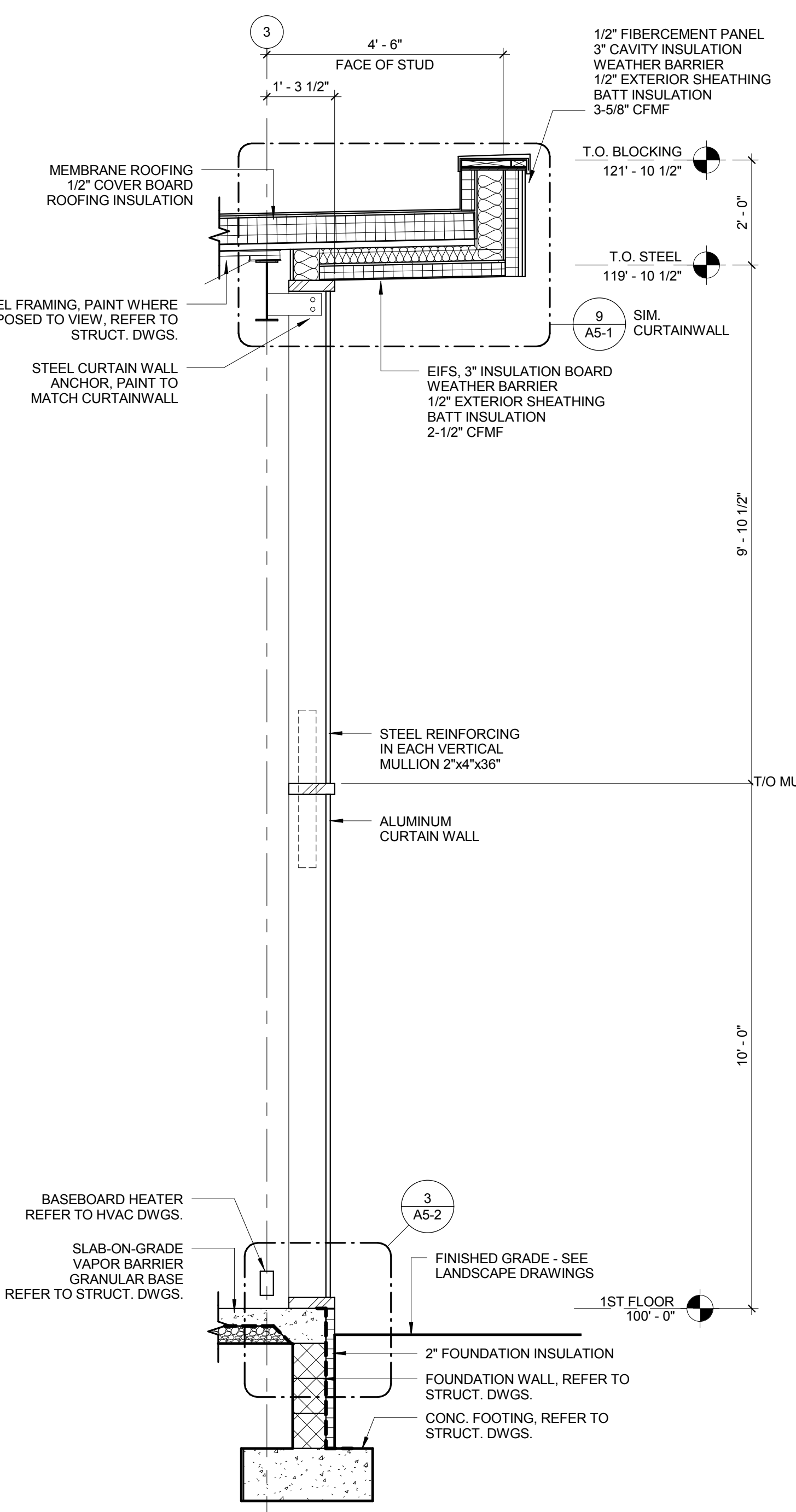
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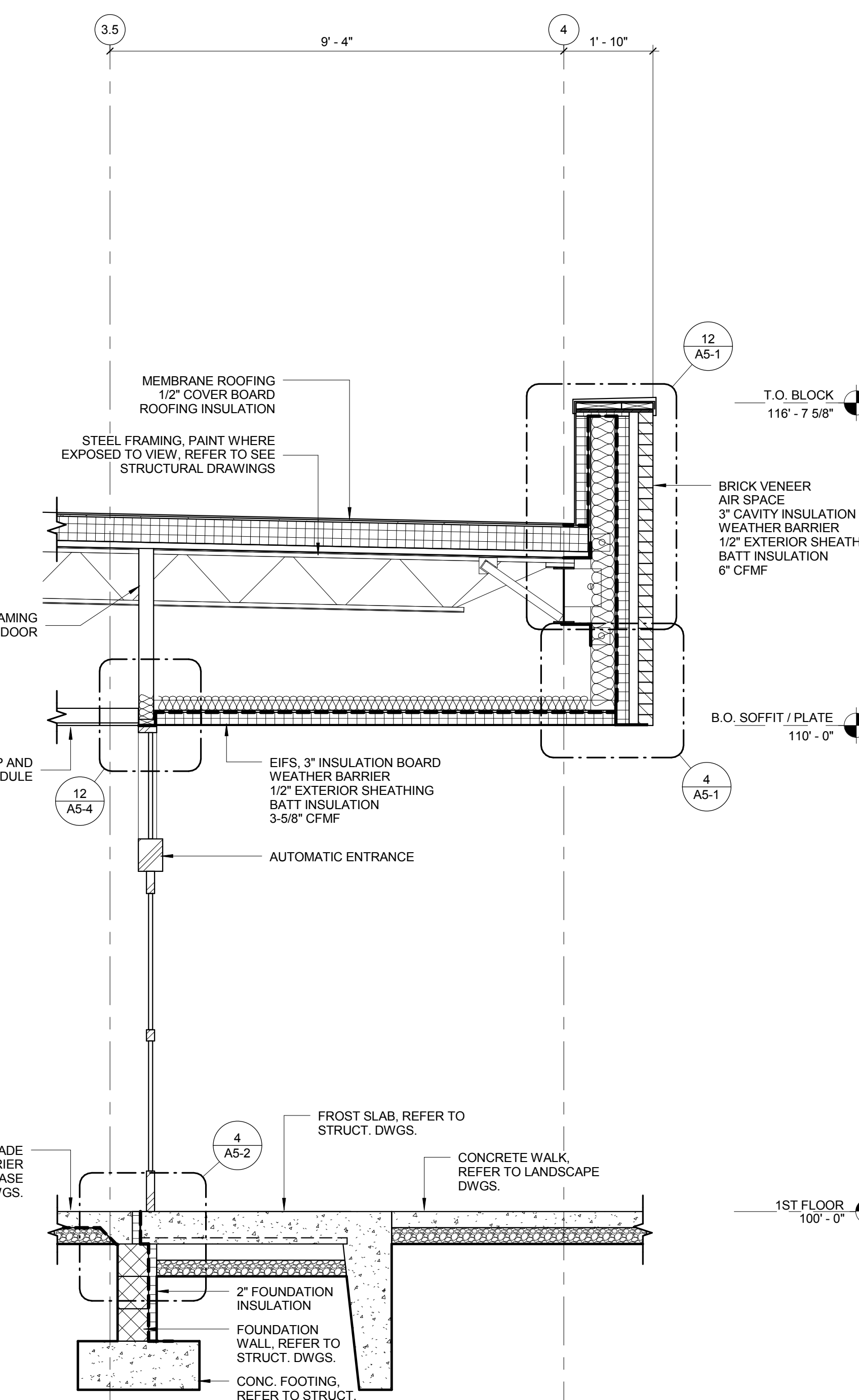
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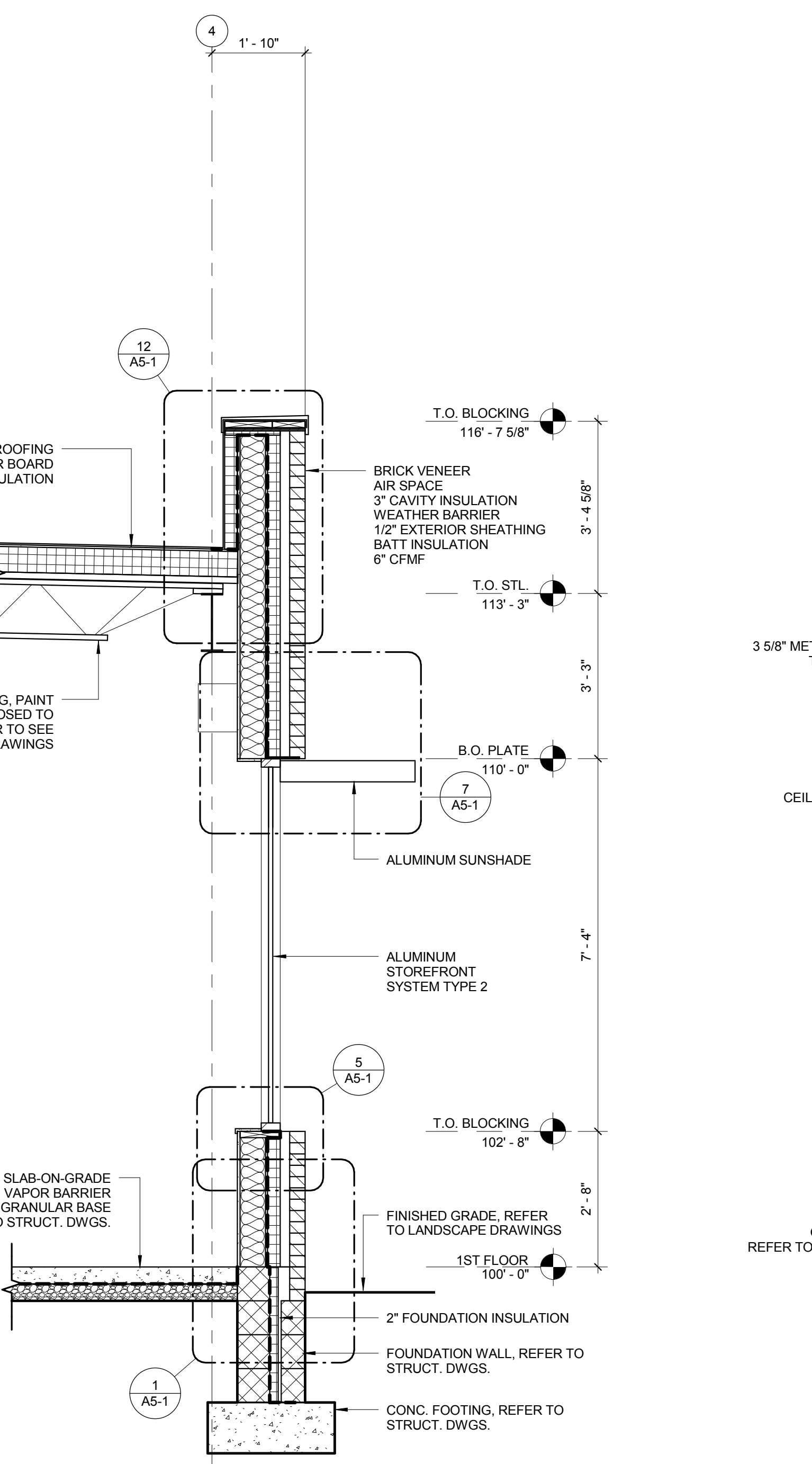
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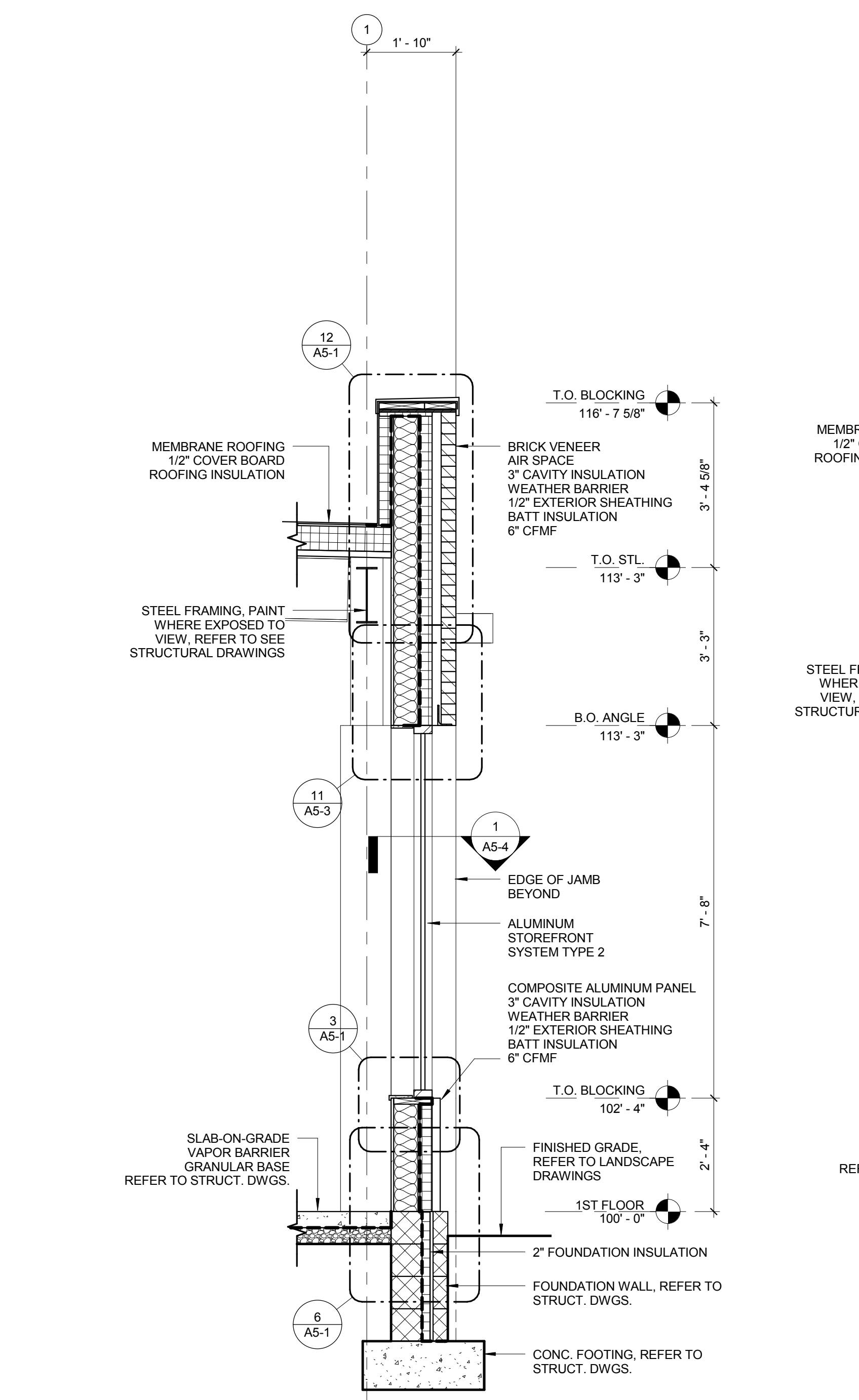
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WALL SECTION 2
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WALL SECTION 3
1/2" = 1'-0" A4-1



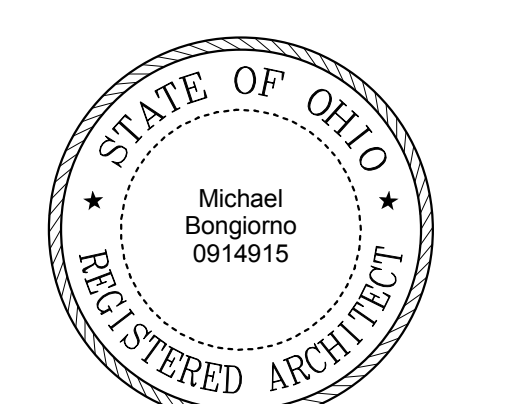
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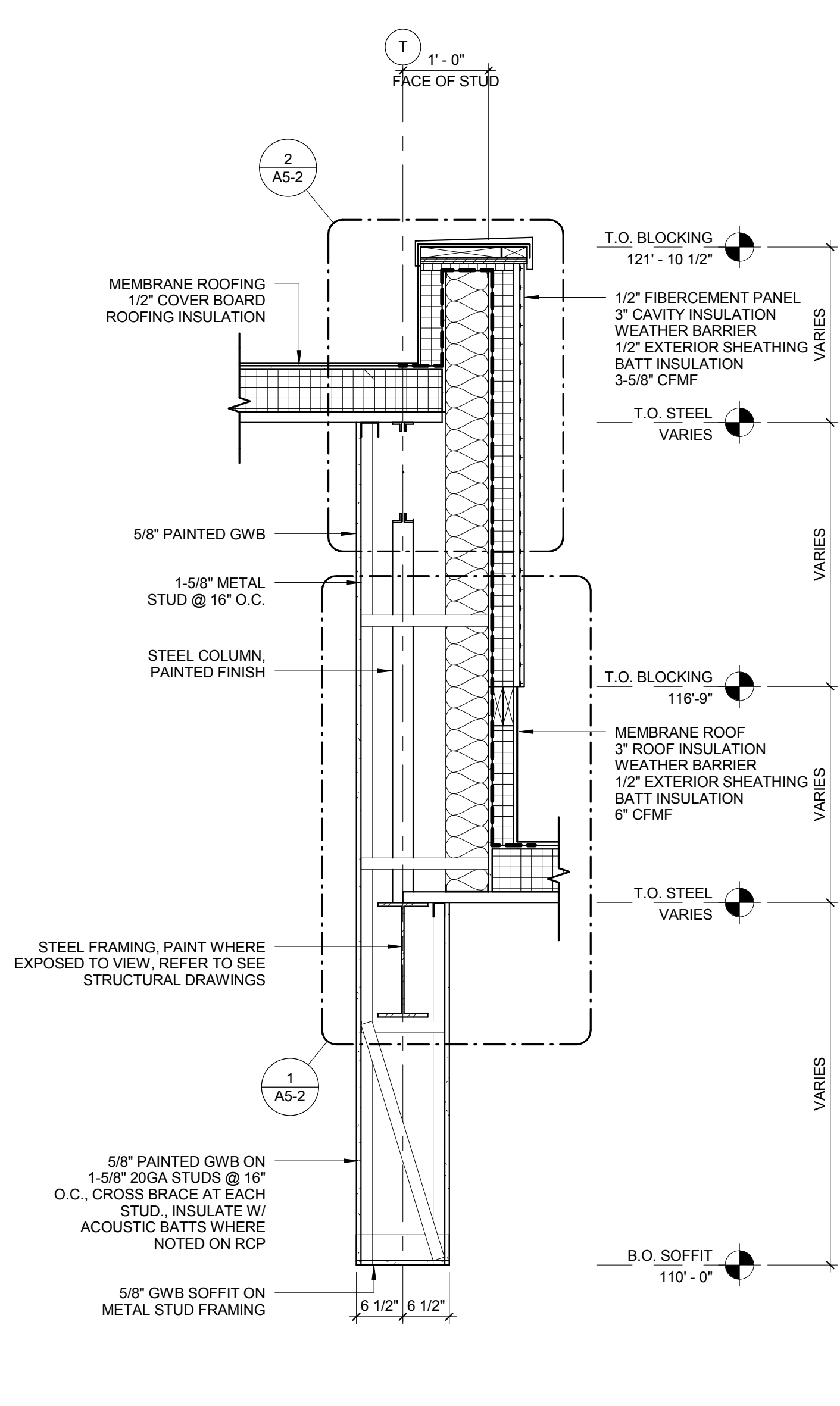
CIVIL AND MEP ENGINEER

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Columbus, Ohio 43215 fax 614 487 8861

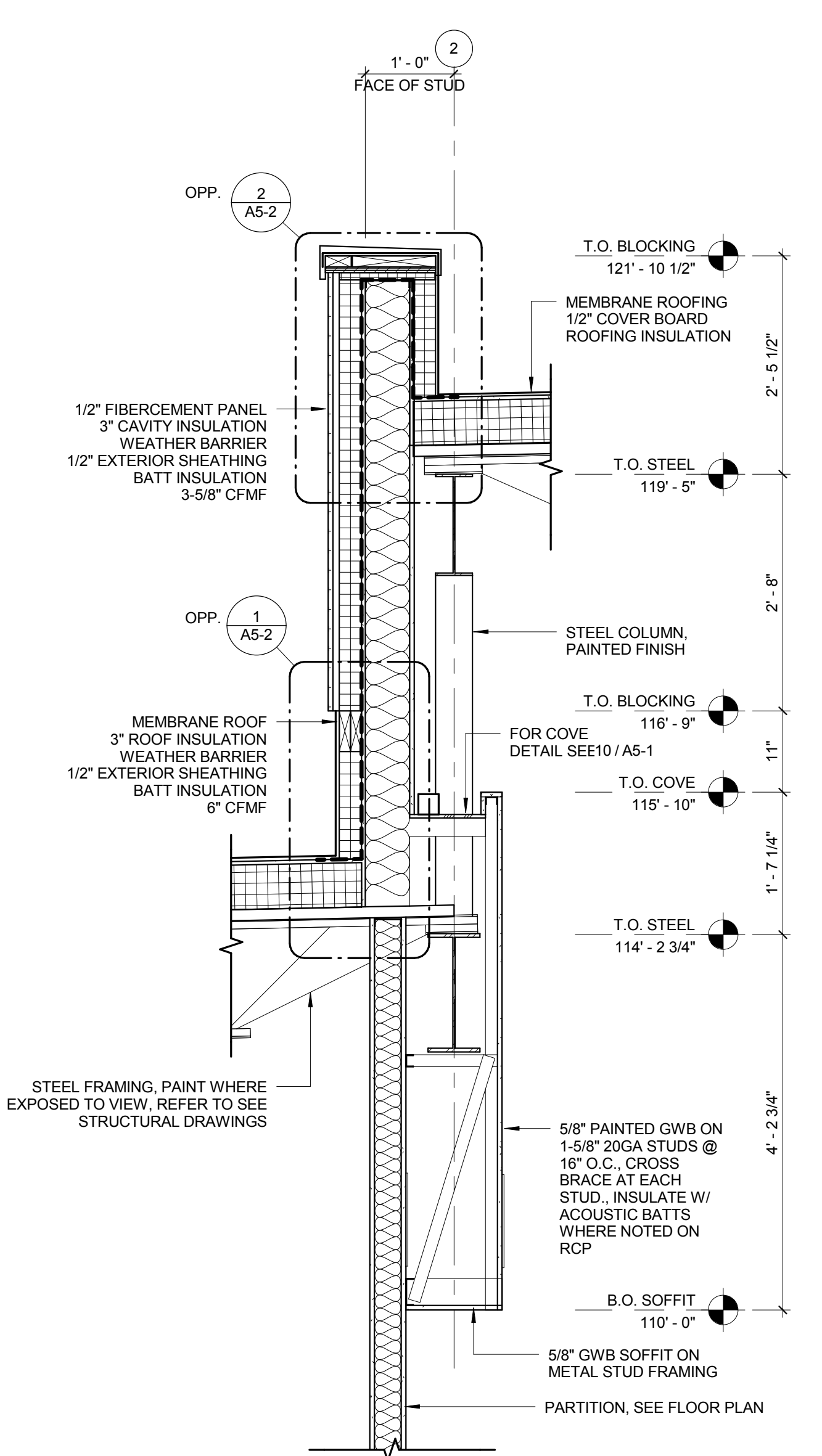
STRUCTURAL ENGINEER

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Columbus, Ohio 43215-1038 fax 614 481 9353

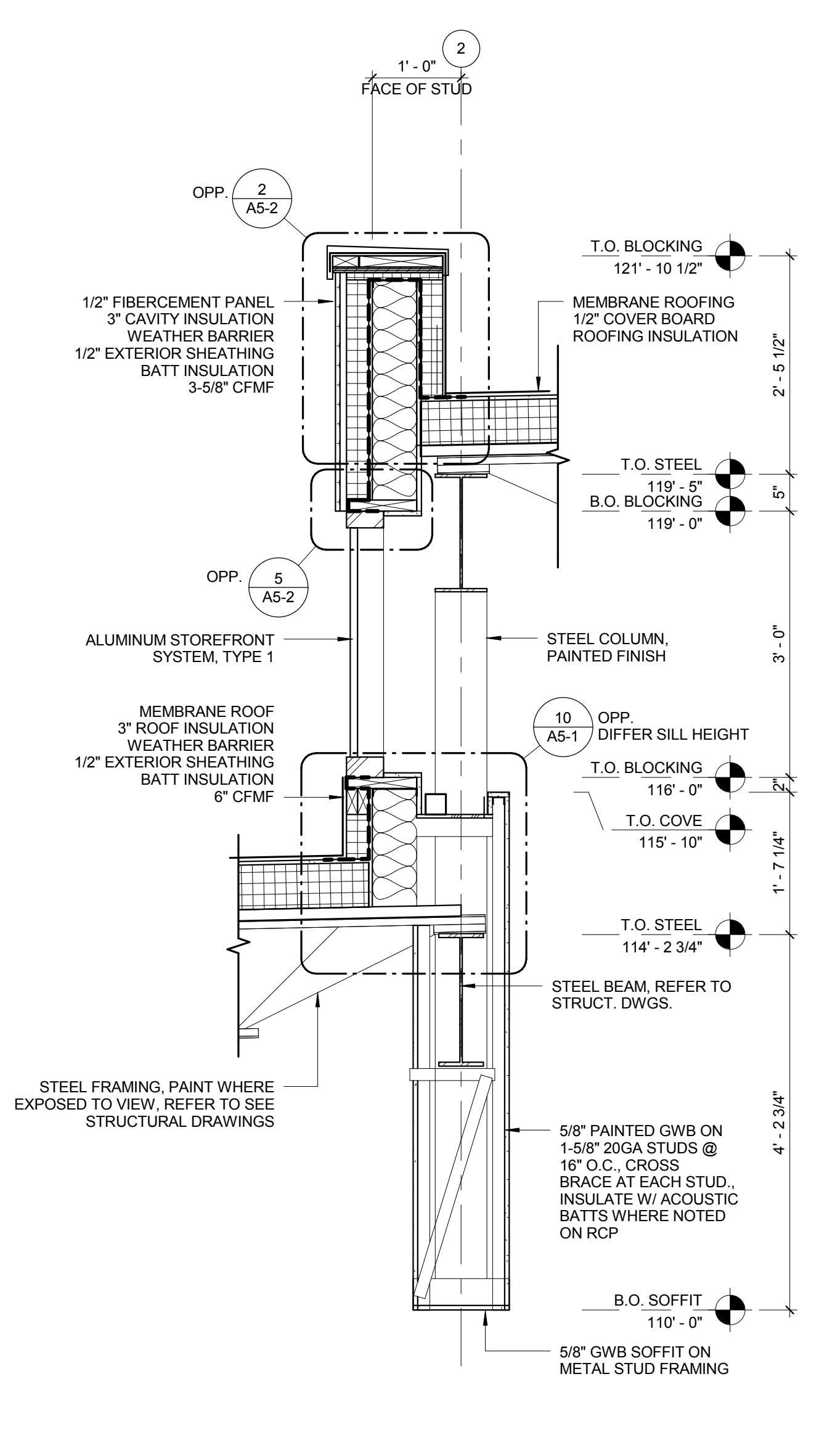
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1 16 Dec 2015 Bulletin 12



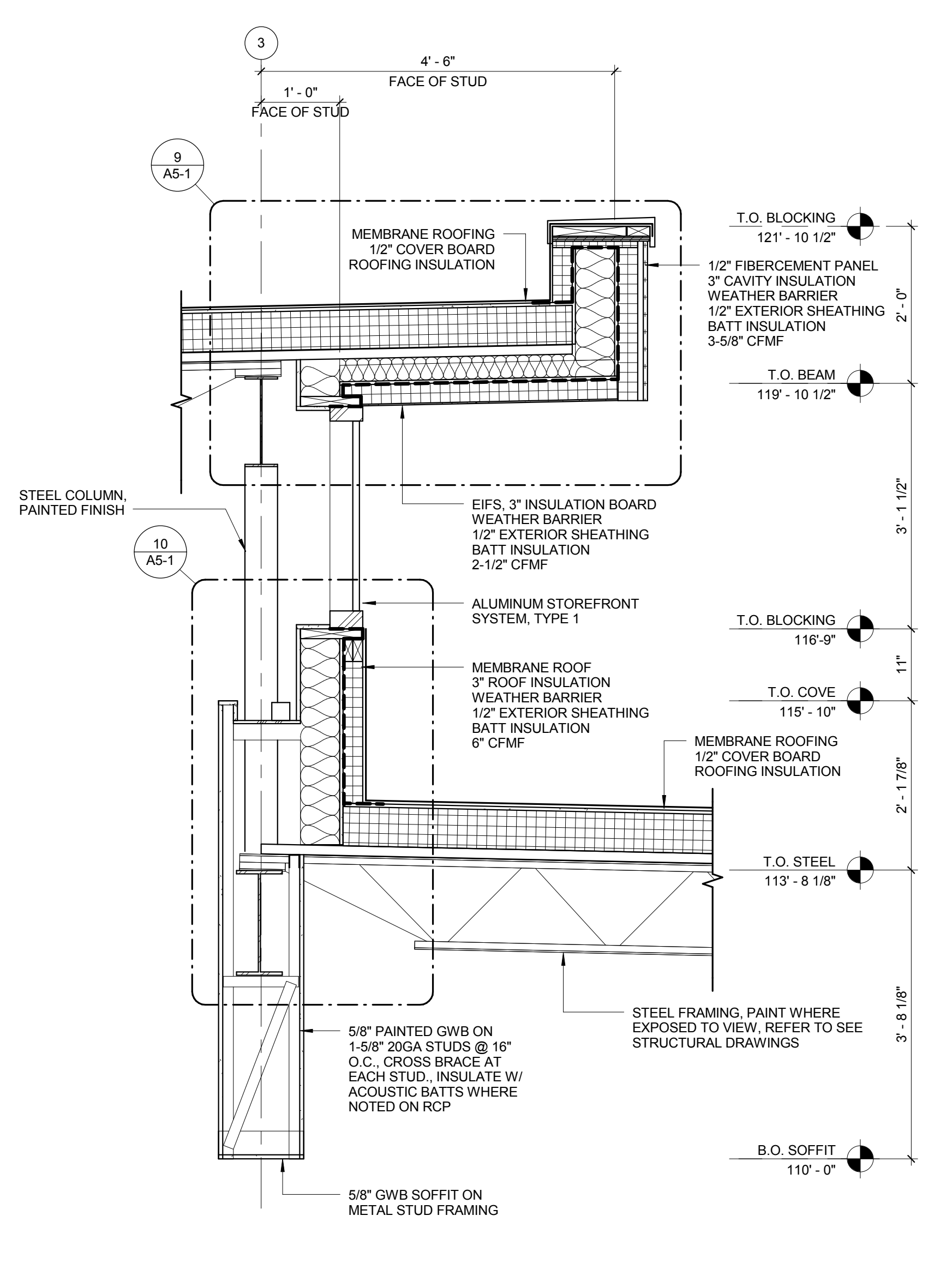
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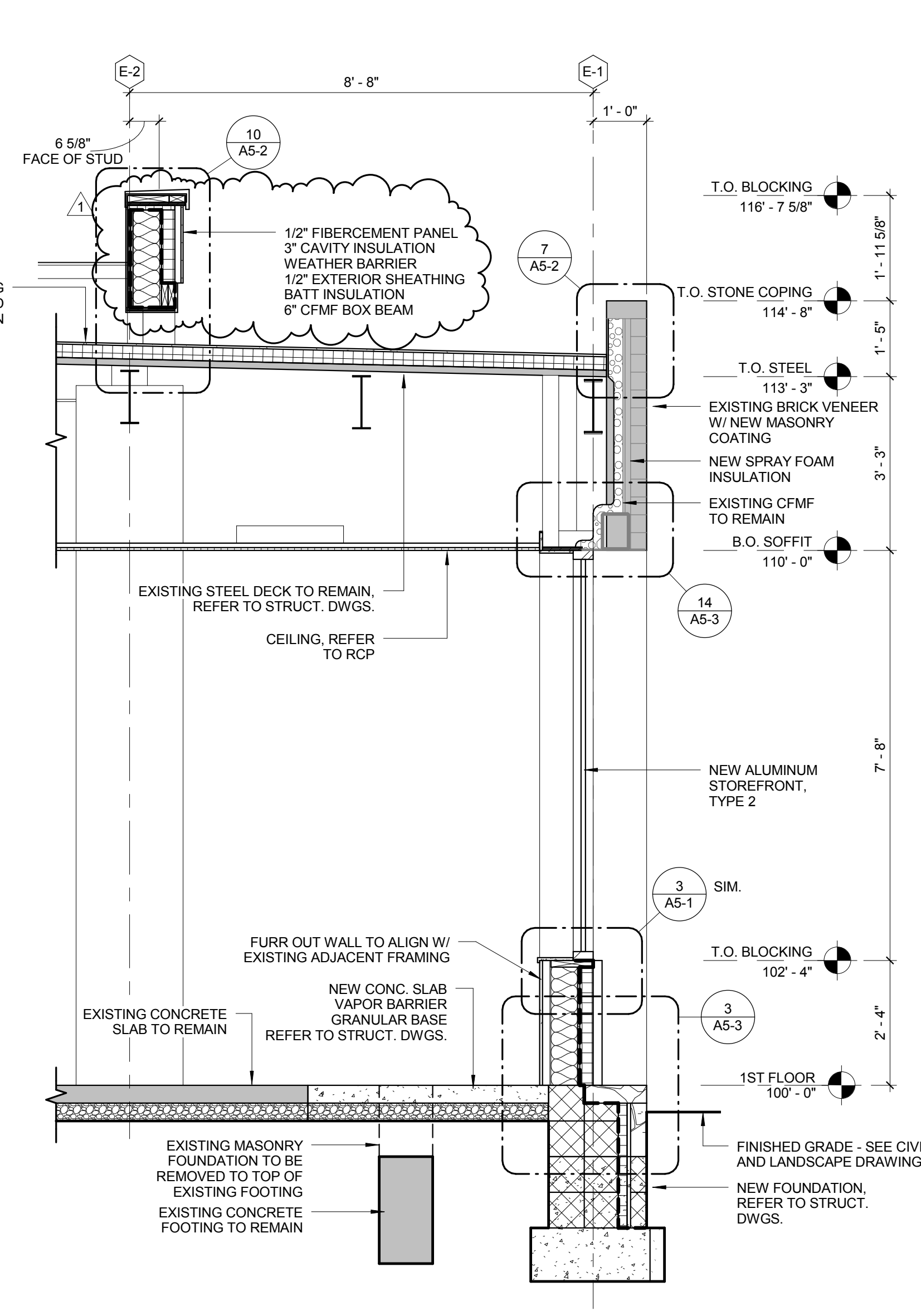
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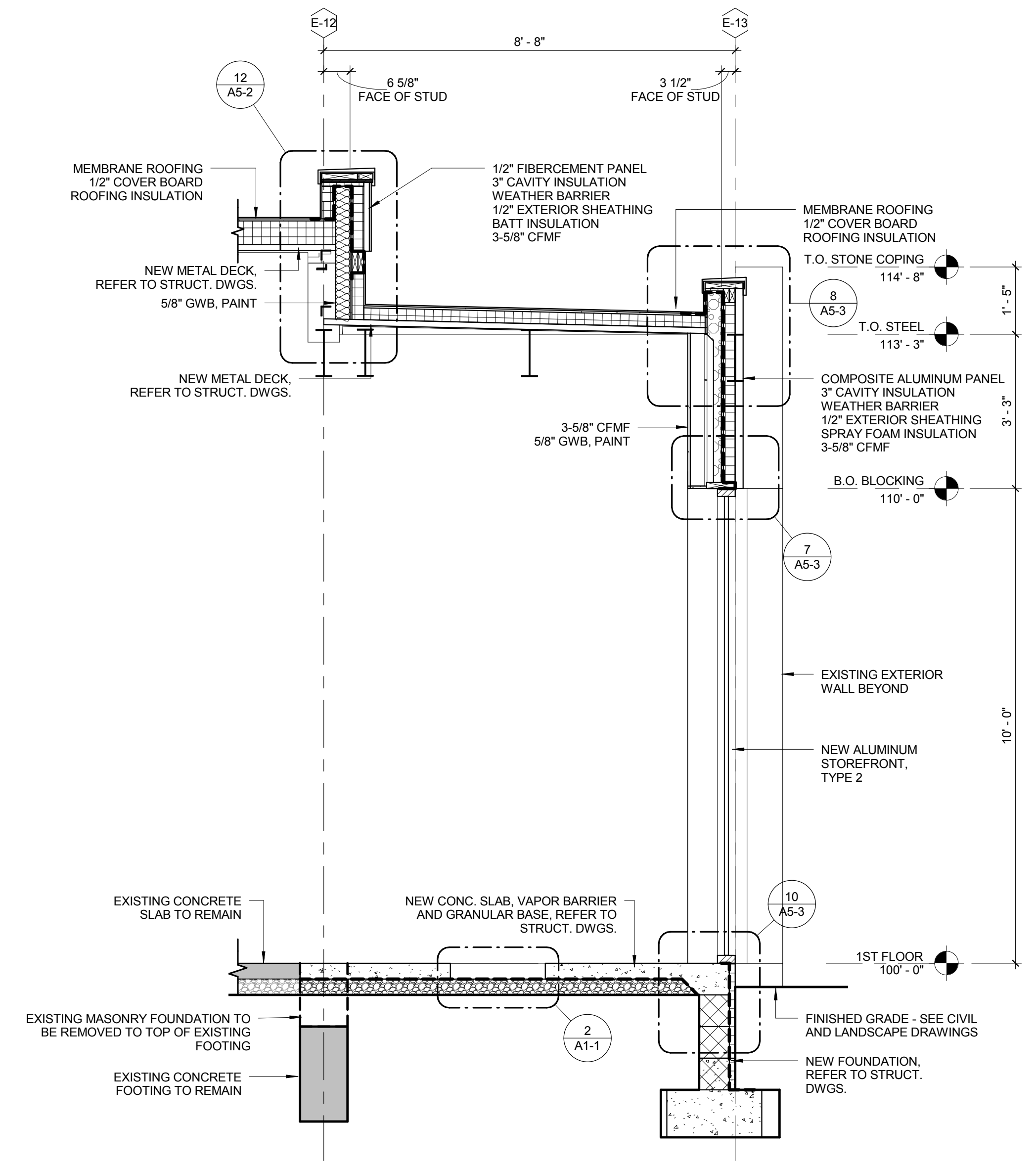
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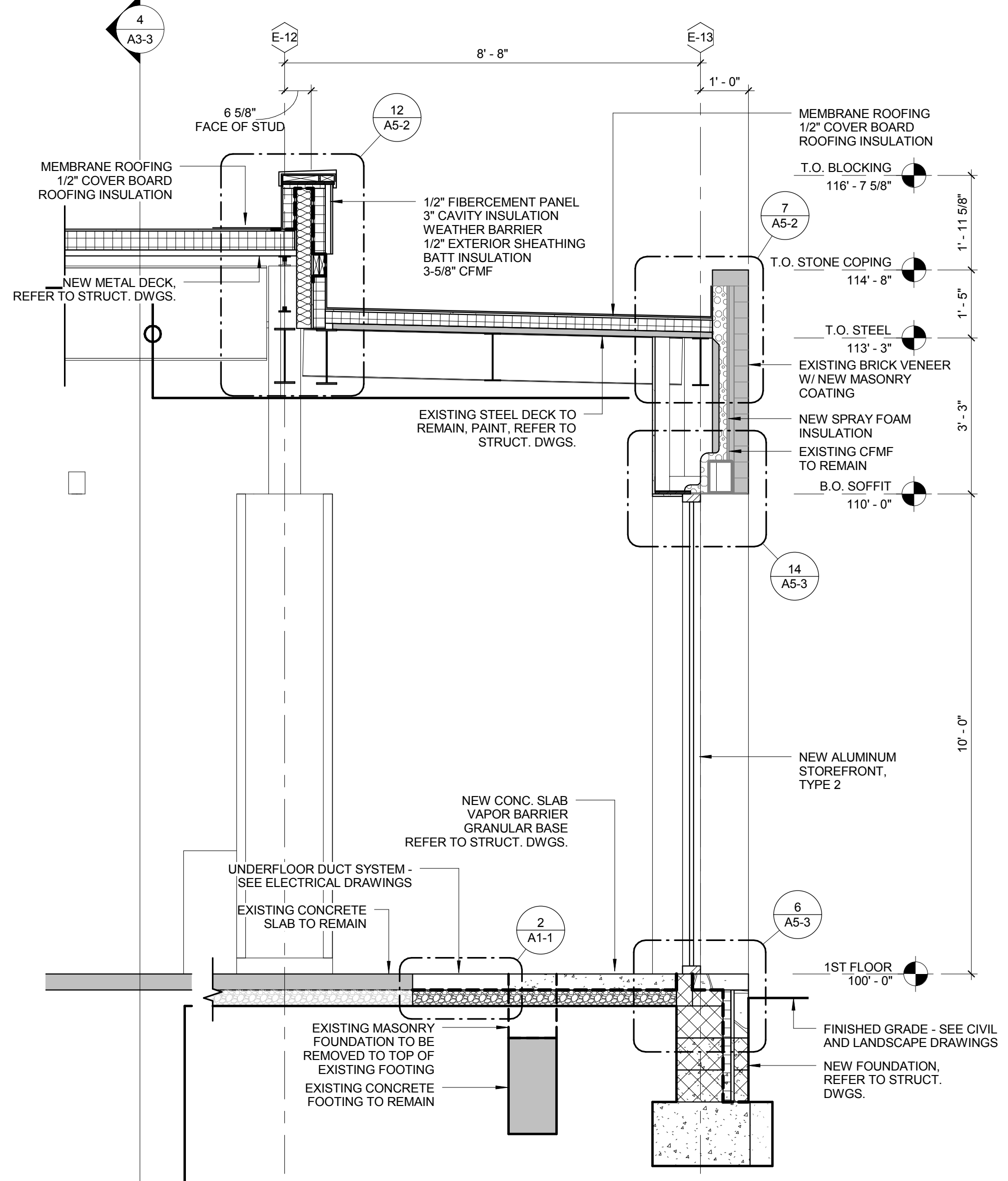
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3/4\"/>



WALL SECTION 3
1/2\"/>



WALL SECTION 2
1/2\"/>



WALL SECTION 1
1/2\"/>

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WALL SECTIONS

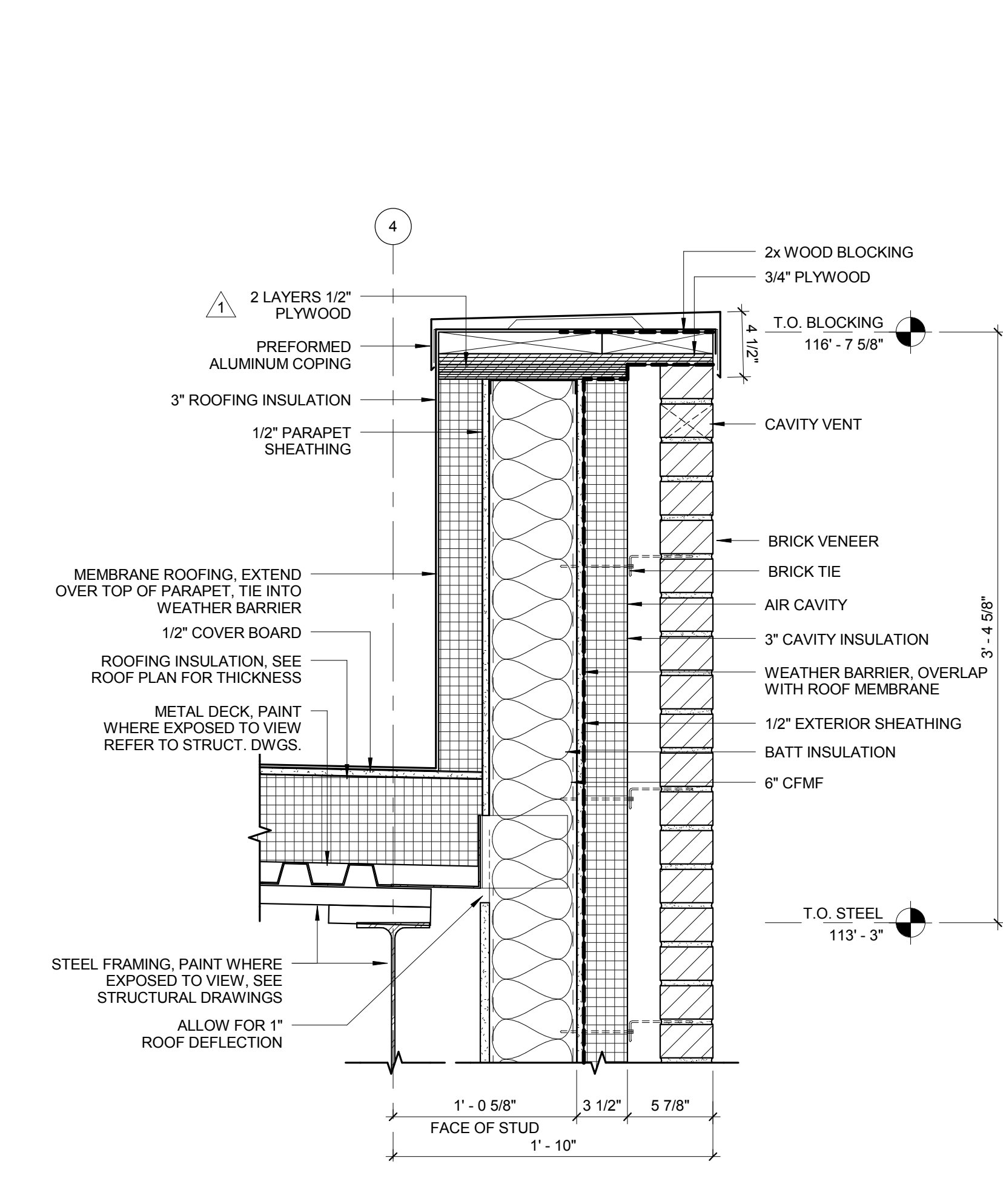
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revised:

BID DRAWINGS

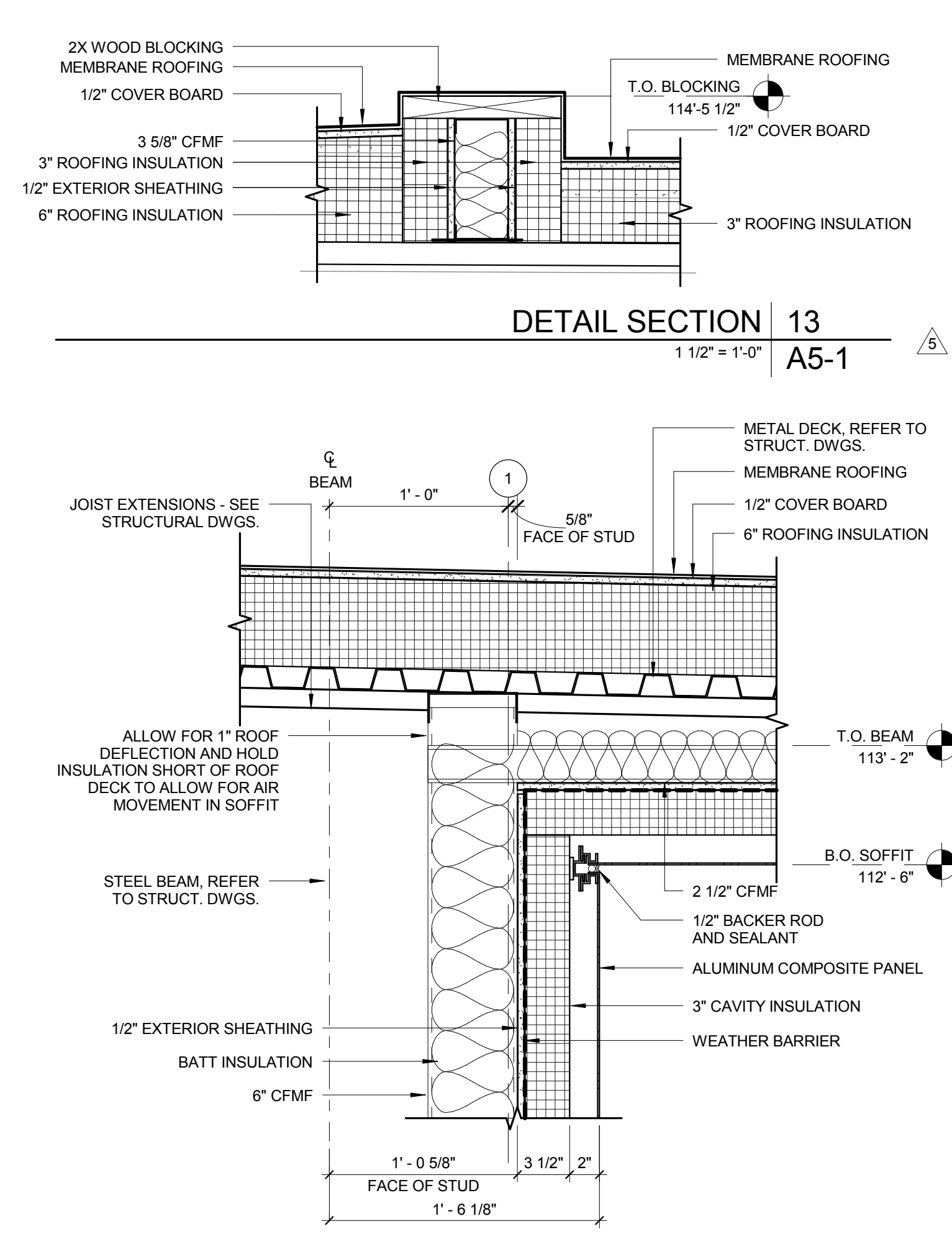
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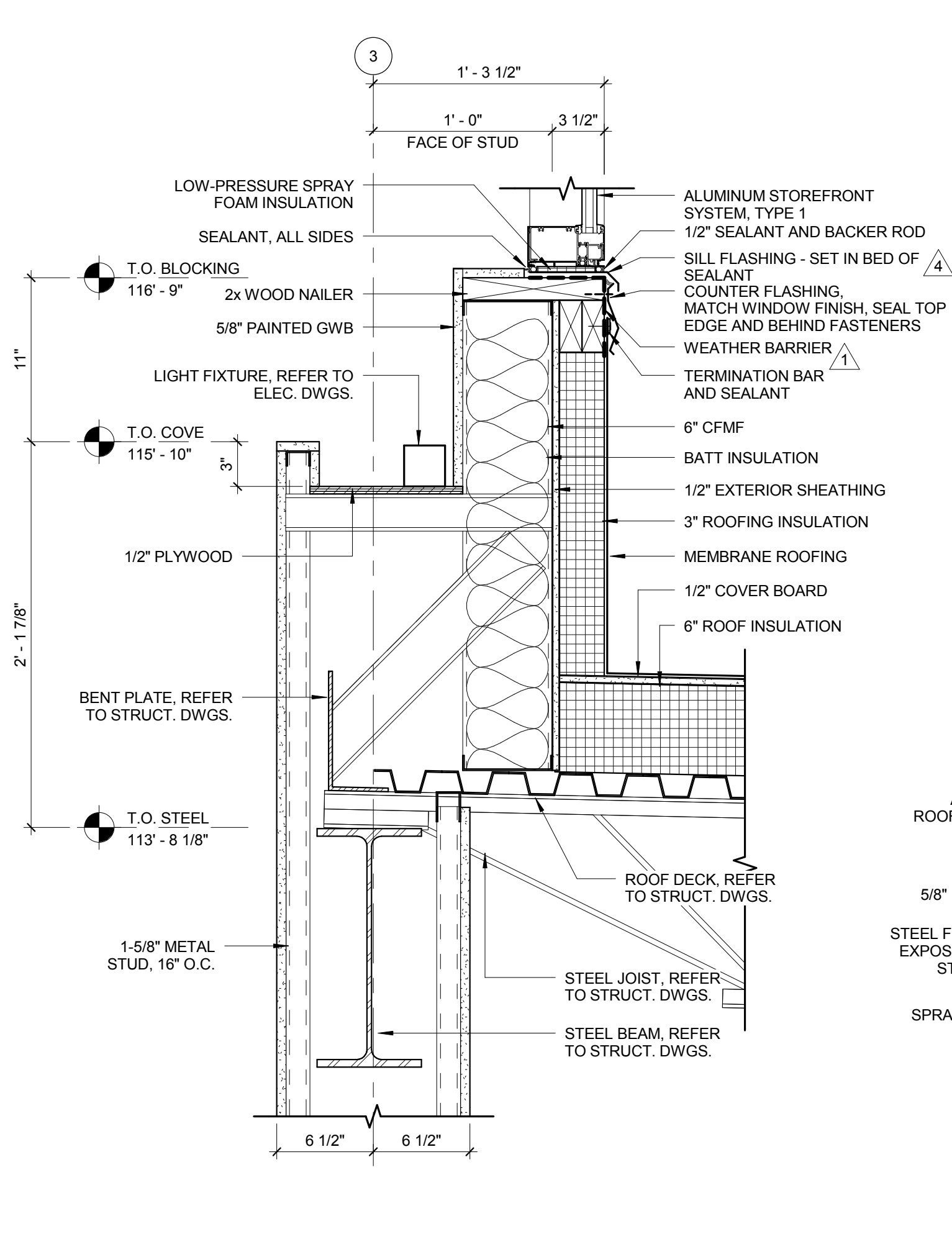


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A5-1

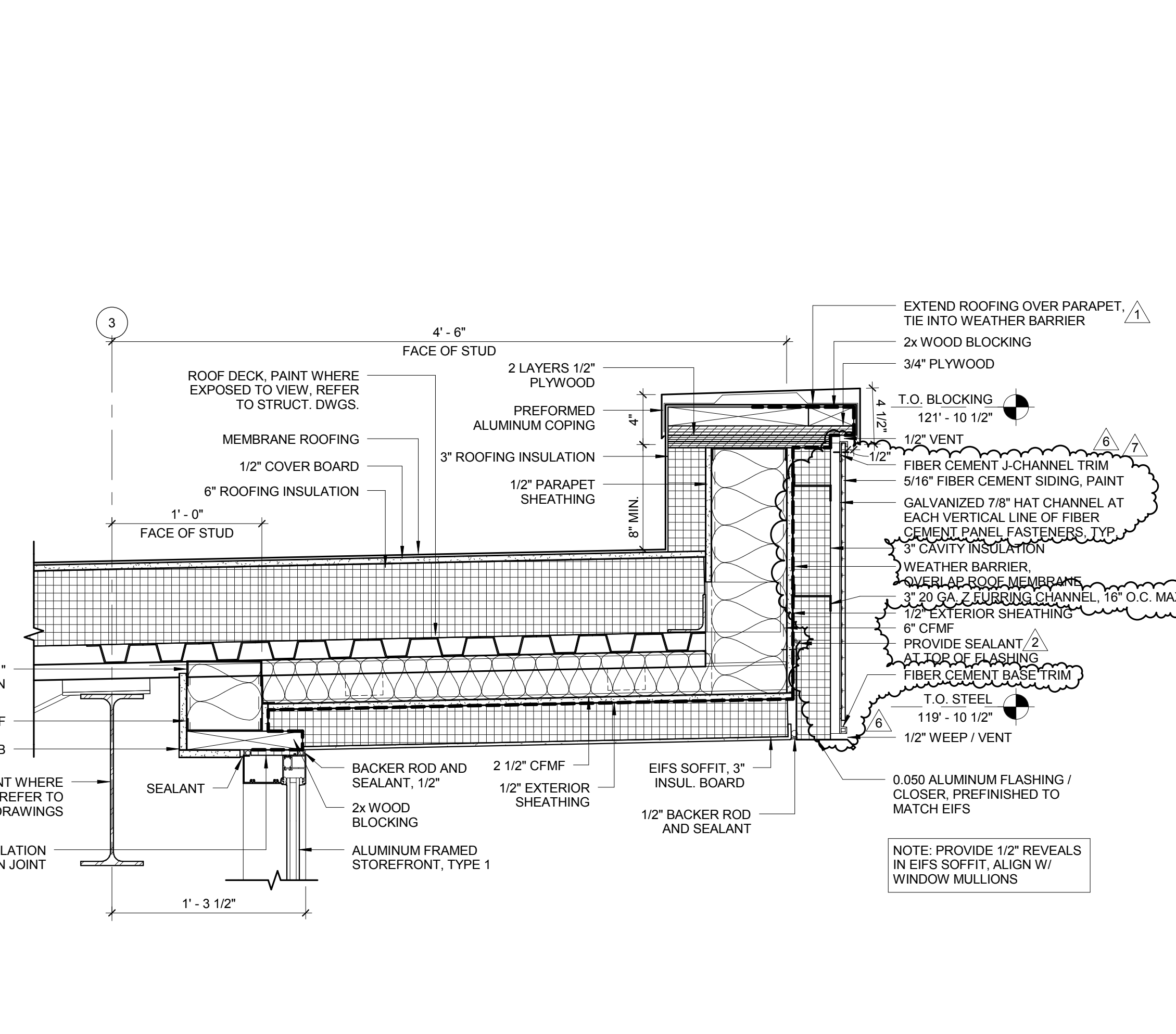


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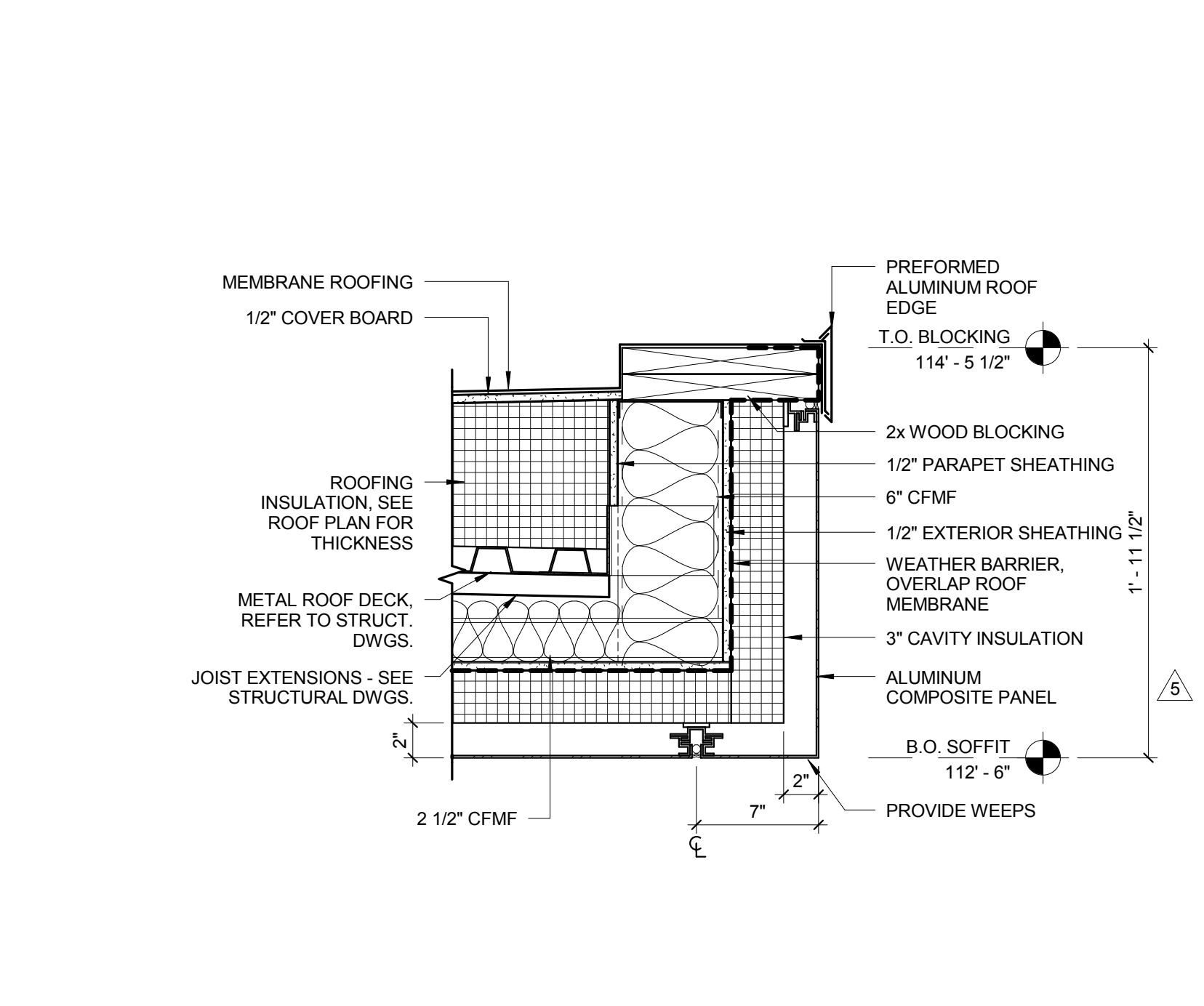
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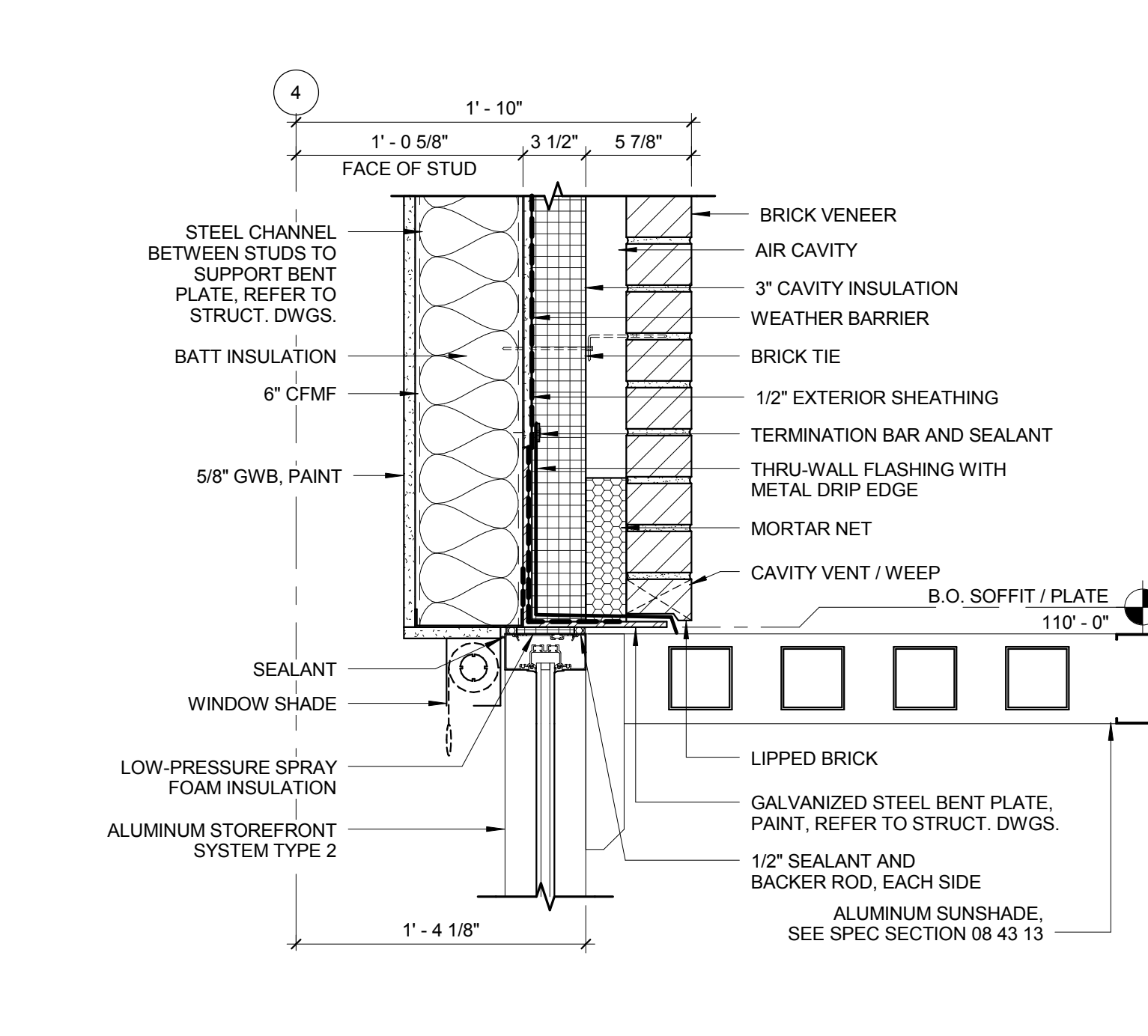
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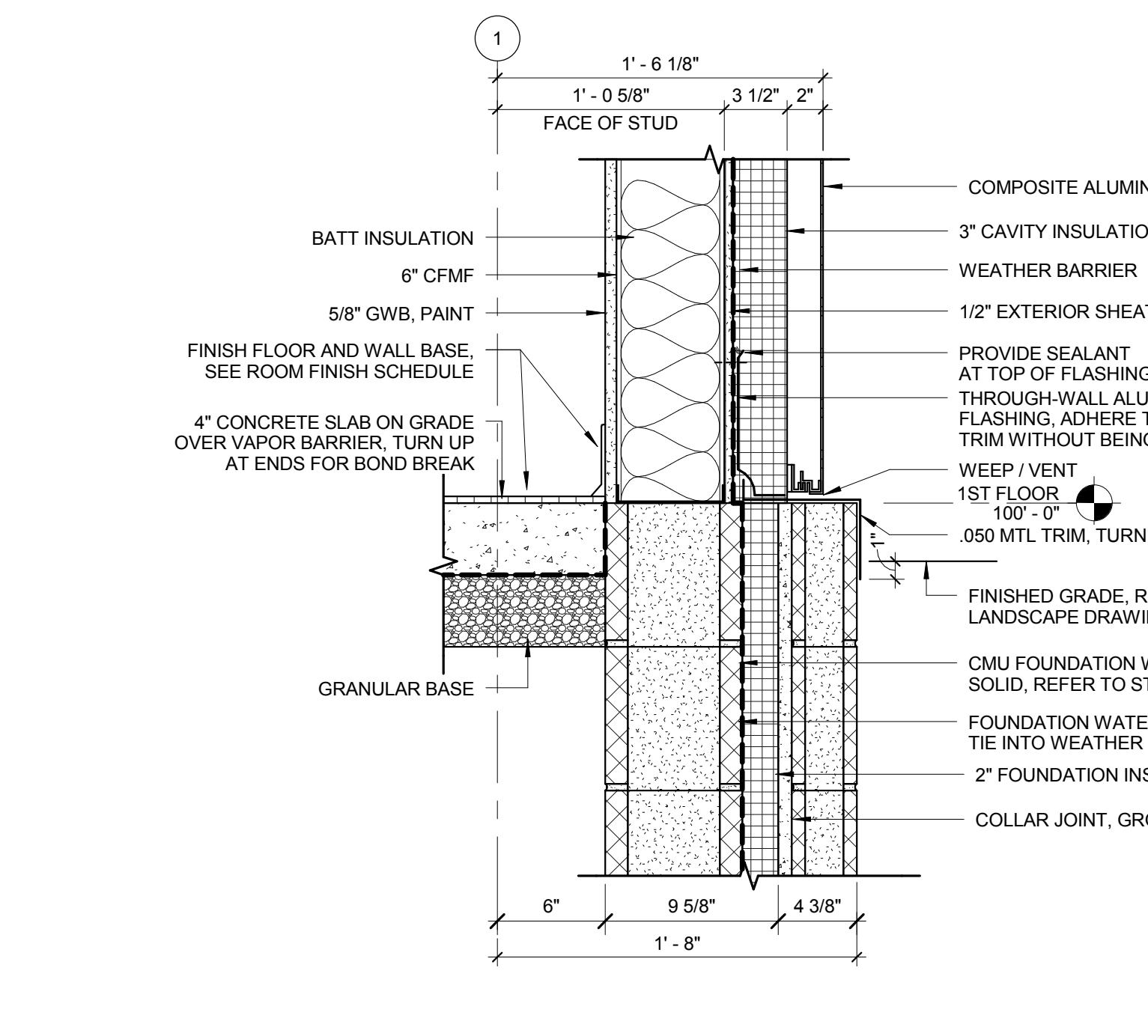
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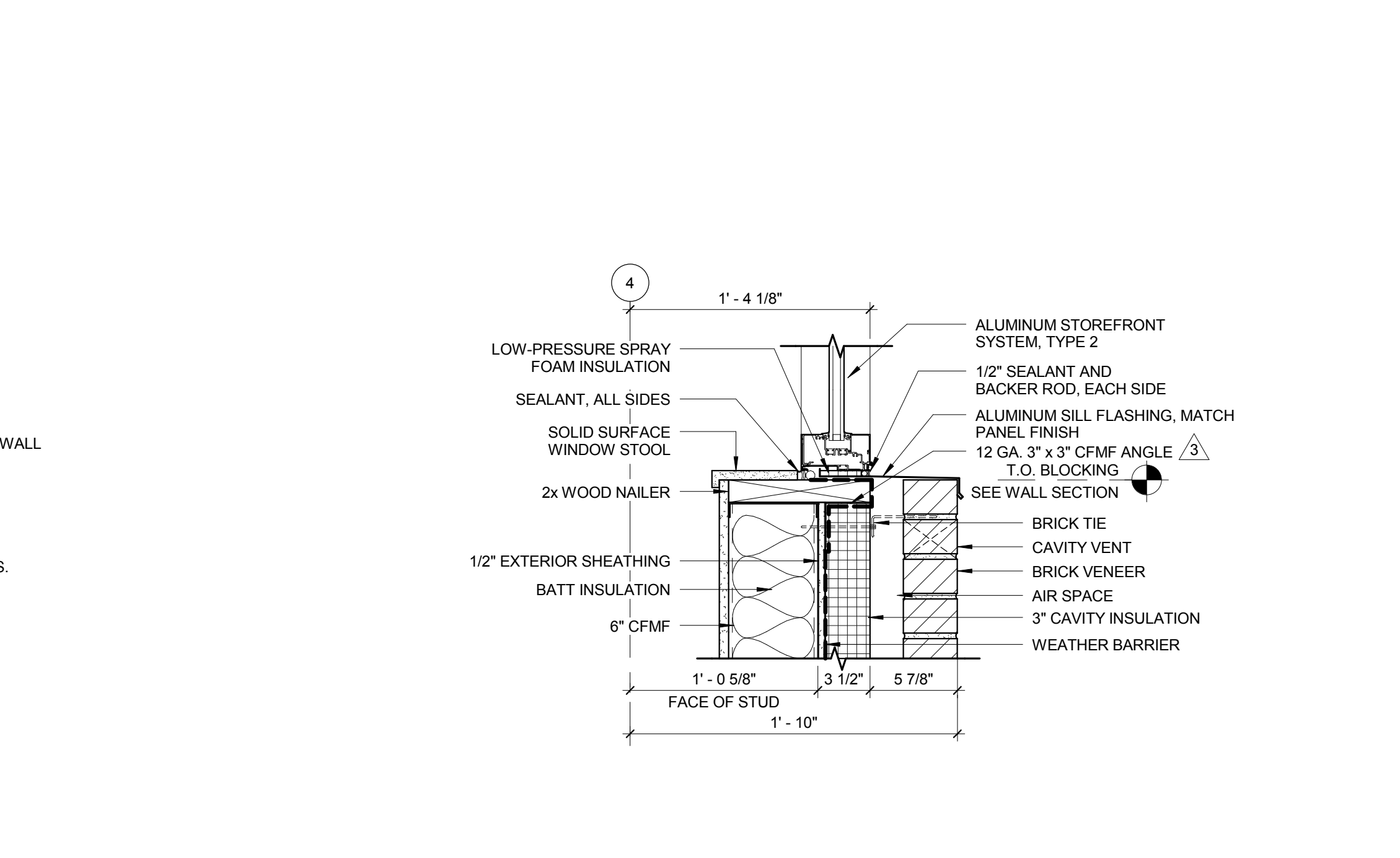
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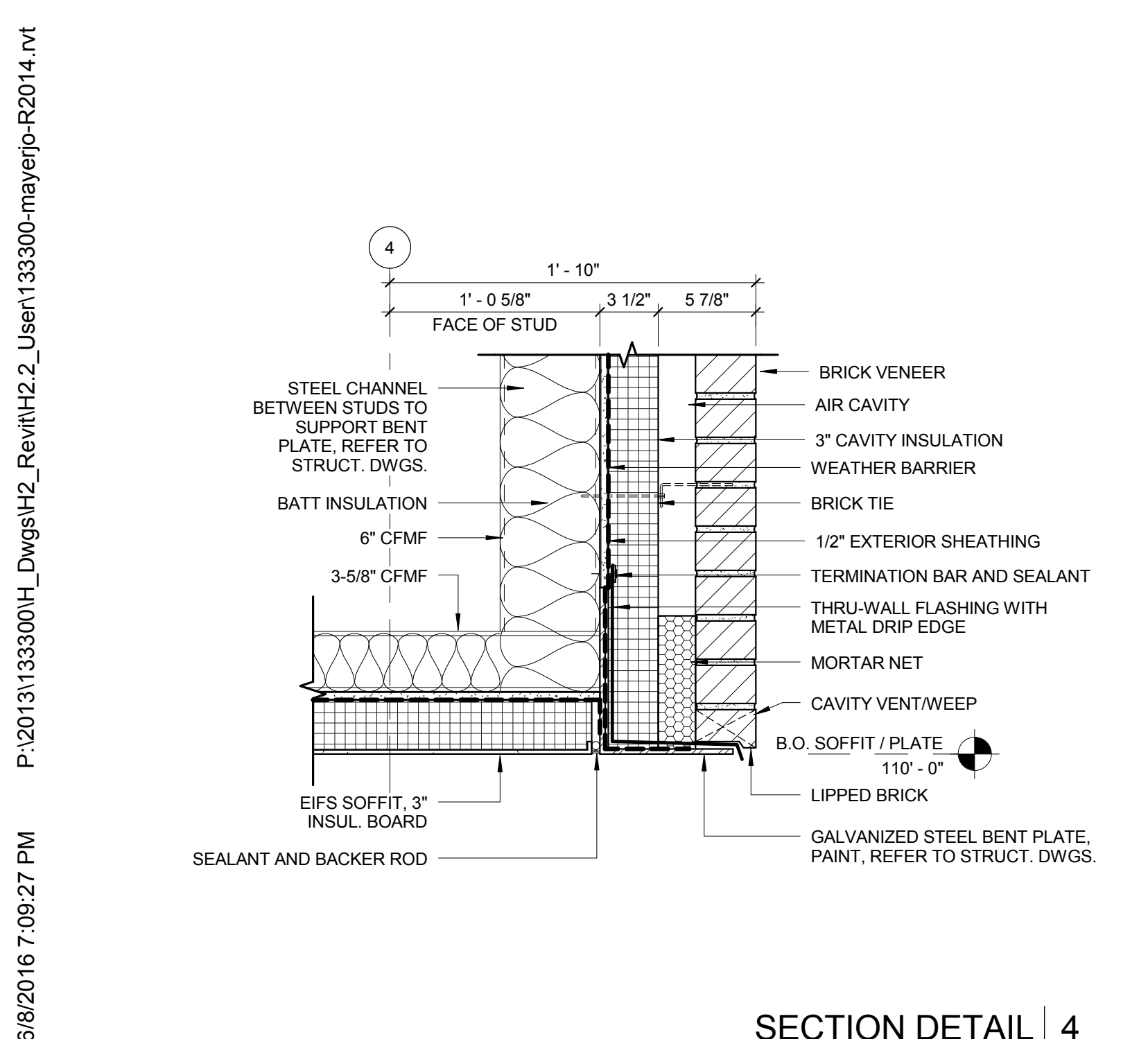
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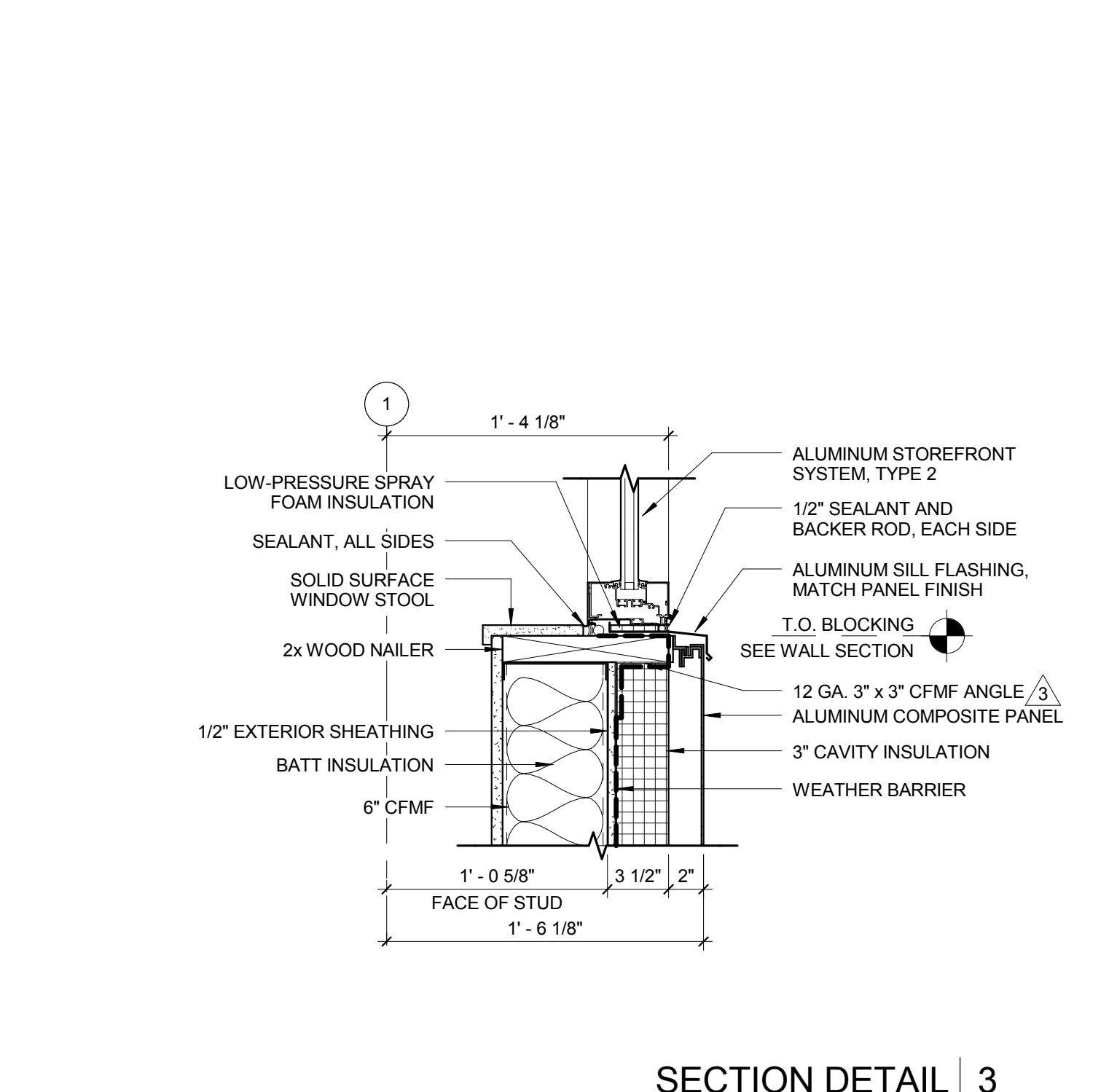
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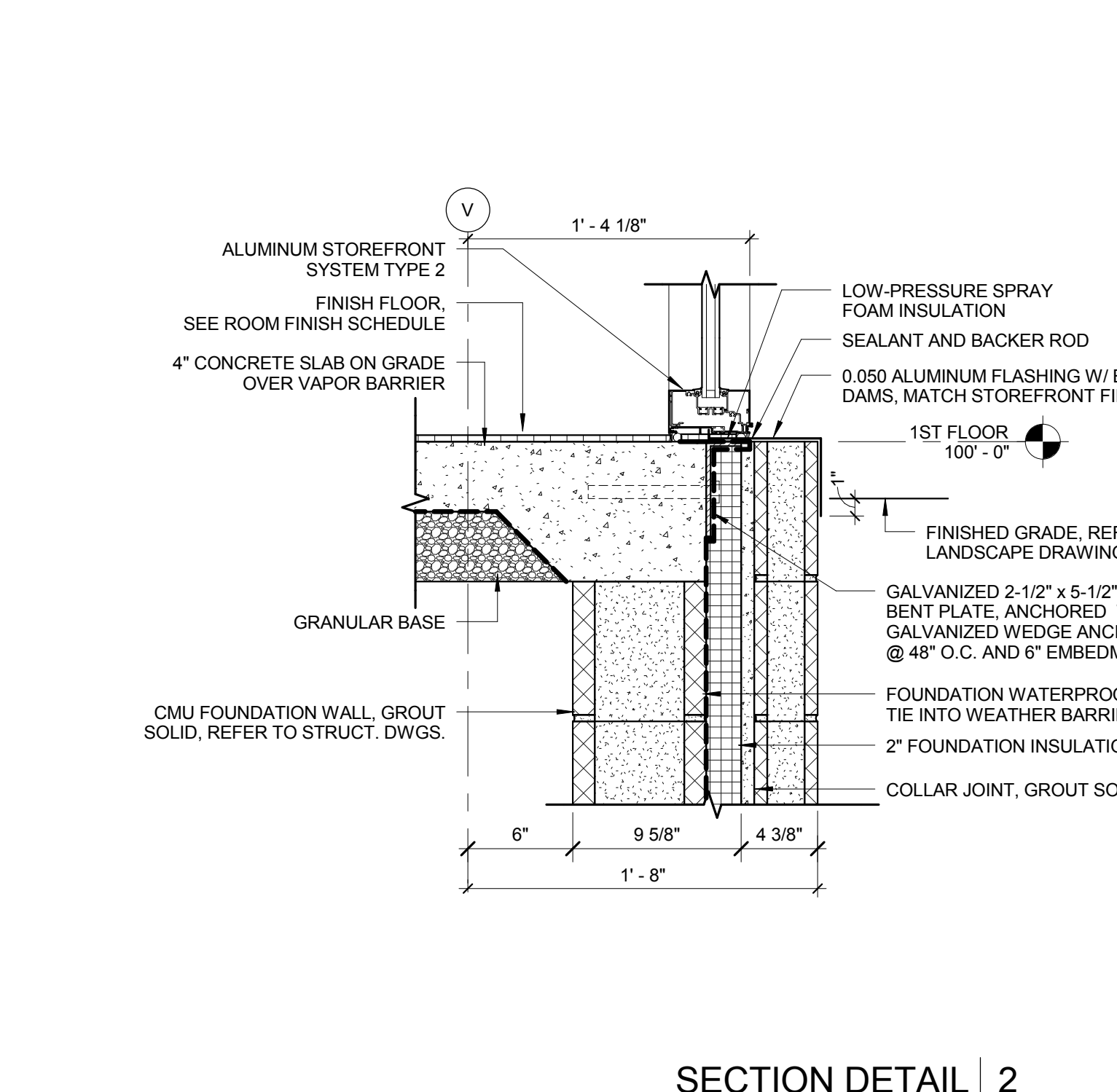
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A5-1



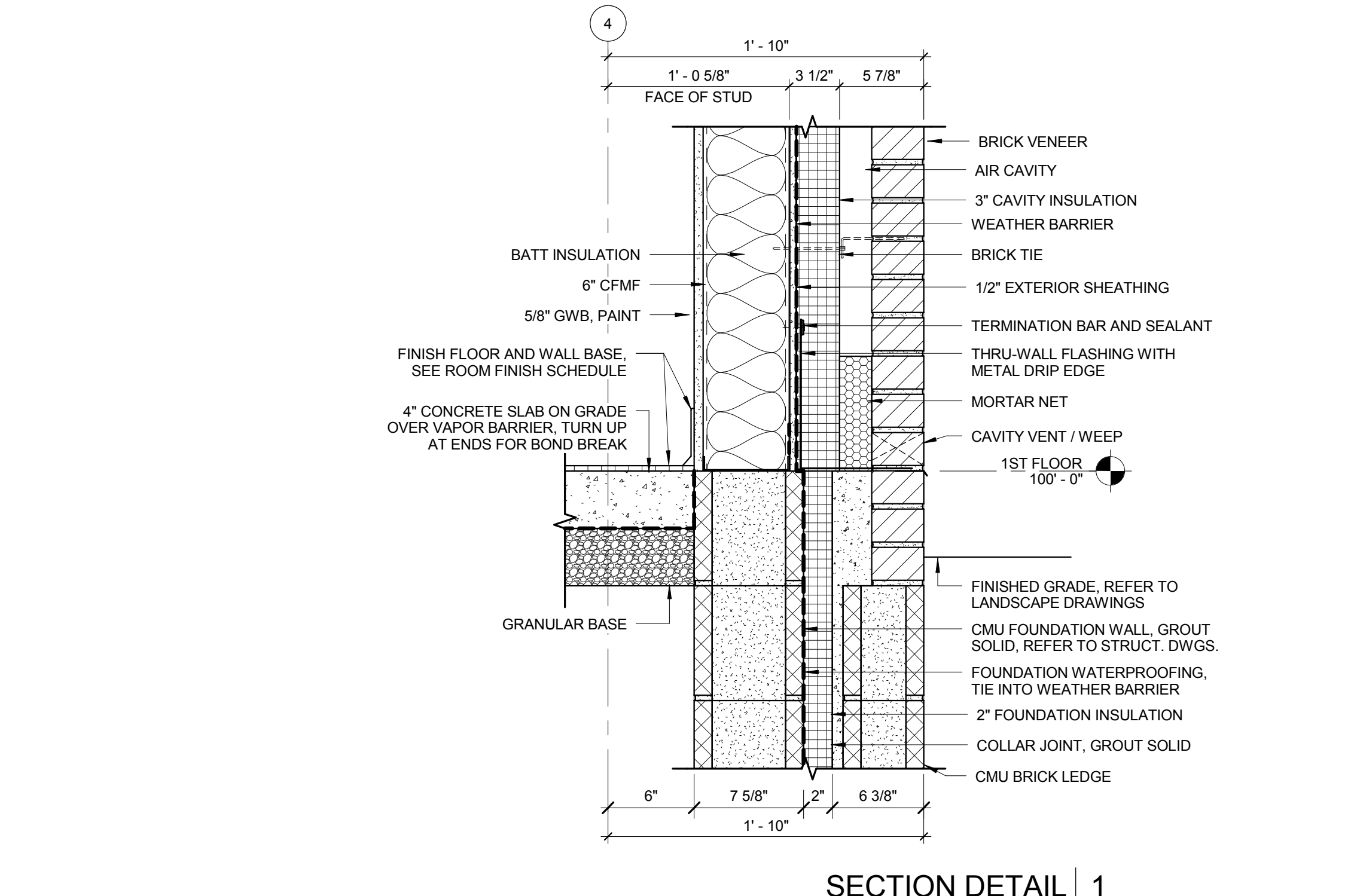
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1 1/2" = 1'-0"
A5-1



SECTION DETAIL 3
1 1/2" = 1'-0"
A5-1



SECTION DETAIL 2
1 1/2" = 1'-0"
A5-1



SECTION DETAIL 1
1 1/2" = 1'-0"
A5-1

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09/14/15

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CIVIL AND MEP ENGINEER
Korba
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Columbus, OH 43215 fax 614 487 8861

STRUCTURAL ENGINEER
SMSH
1166 Dublin Rd., Ste 200 phone 614 481 9800
Columbus, OH 43215-1038 fax 614 481 9303

#	DATE	DESCRIPTION
1	08 Jan 2016	Bulletin 017
2	18 Jan 2016	Bulletin 014 REVISED
3	17 Feb 2016	Bulletin 028
4	23 Feb 2016	Bulletin 30
5	10 Mar 2016	Bulletin 035
6	11 May 2016	Bulletin 044
7	31 May 2016	Bulletin 044 REVISED

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Scale: 1 1/2" = 1'-0"
date: 04.21.2016
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A5-1



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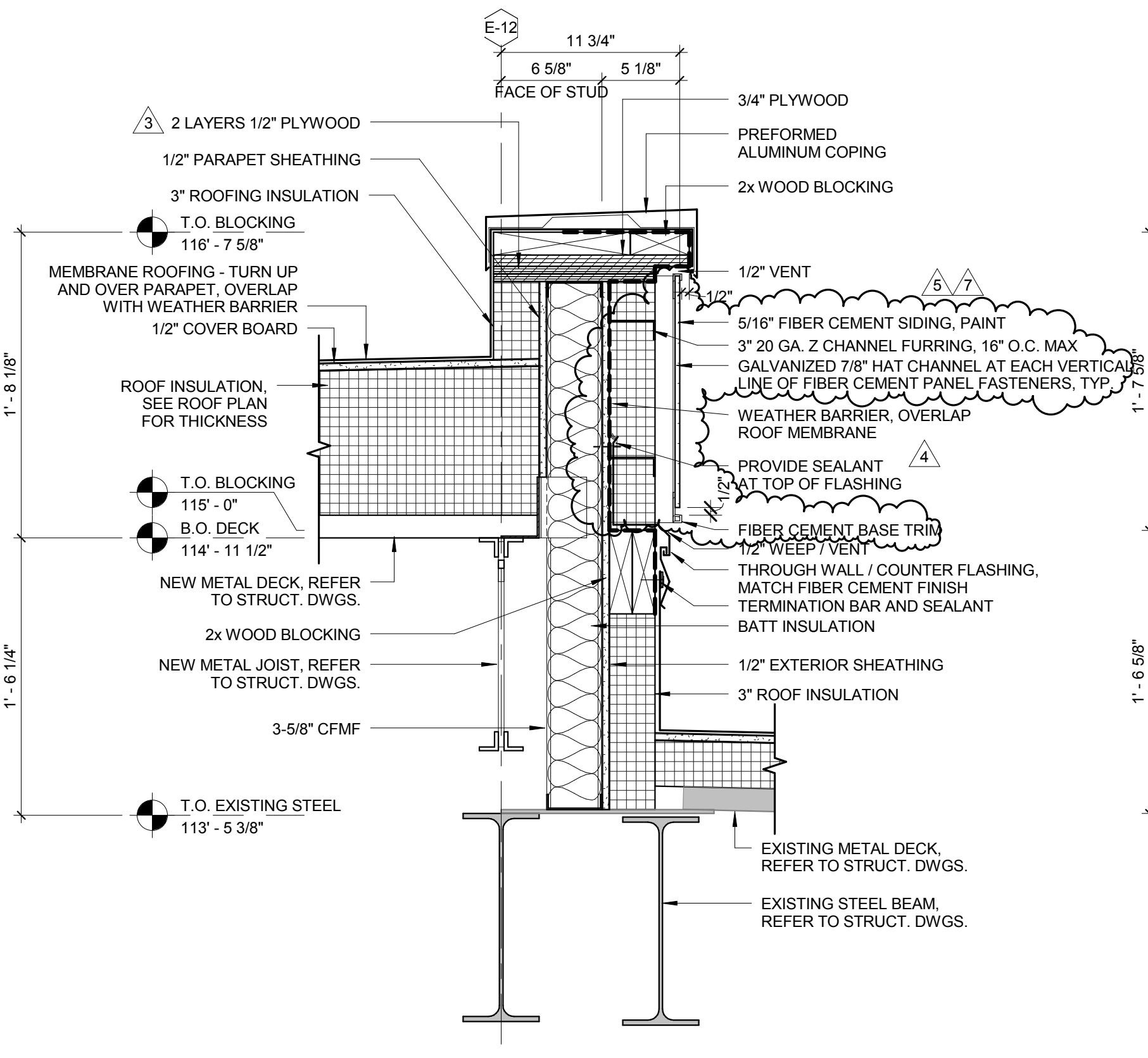
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2	08 Jan 2016	Bulletin 016
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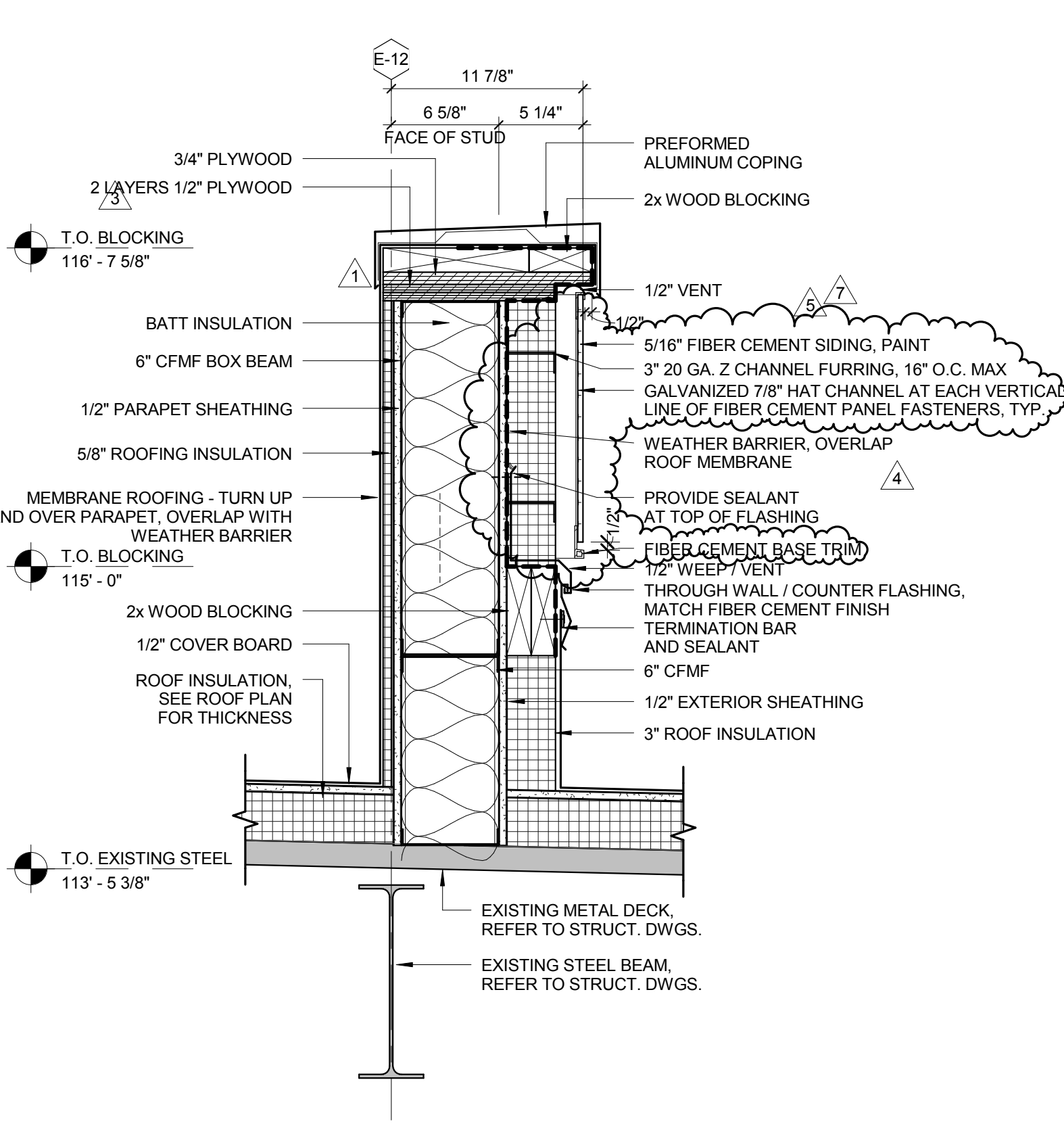
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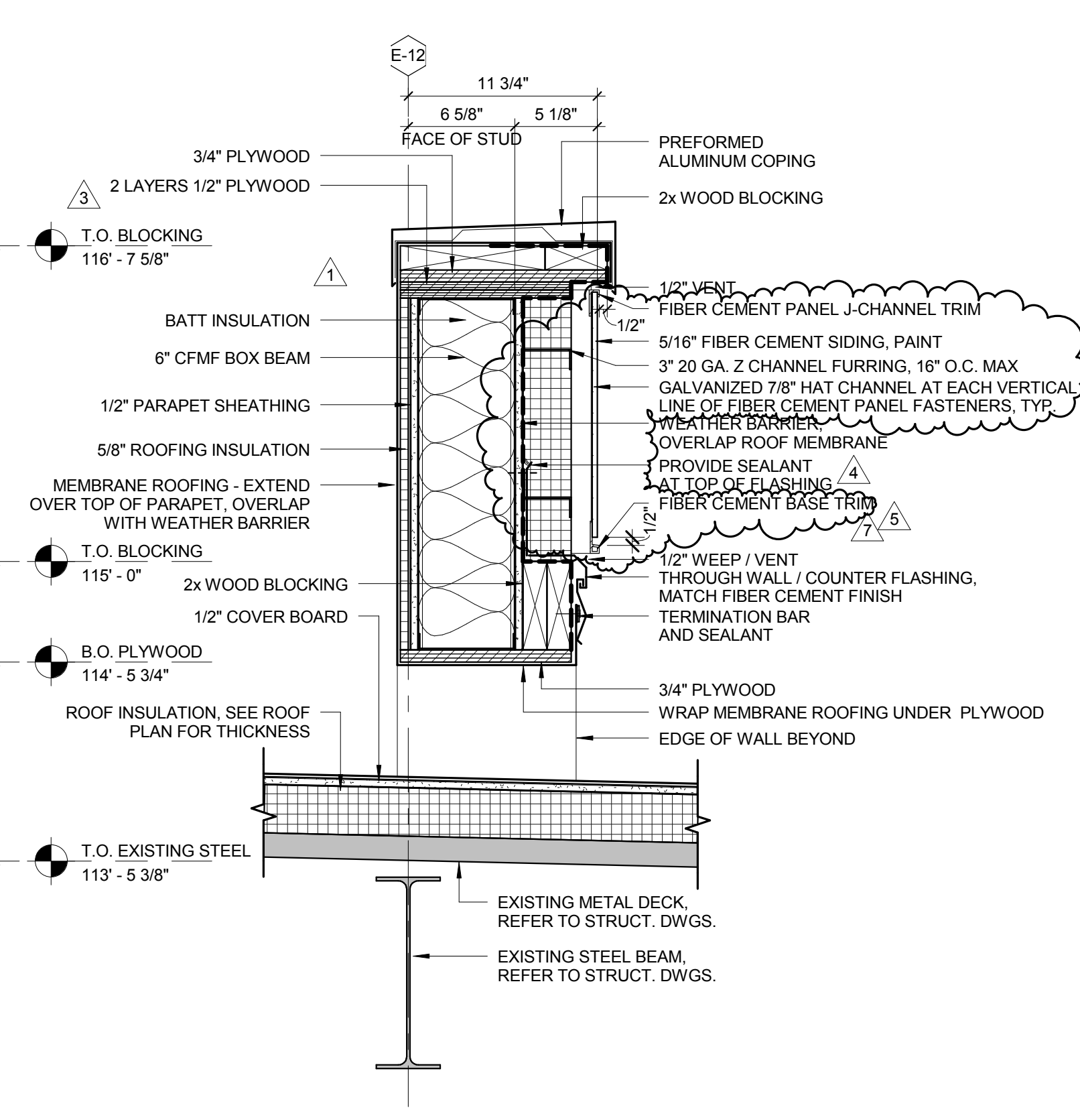
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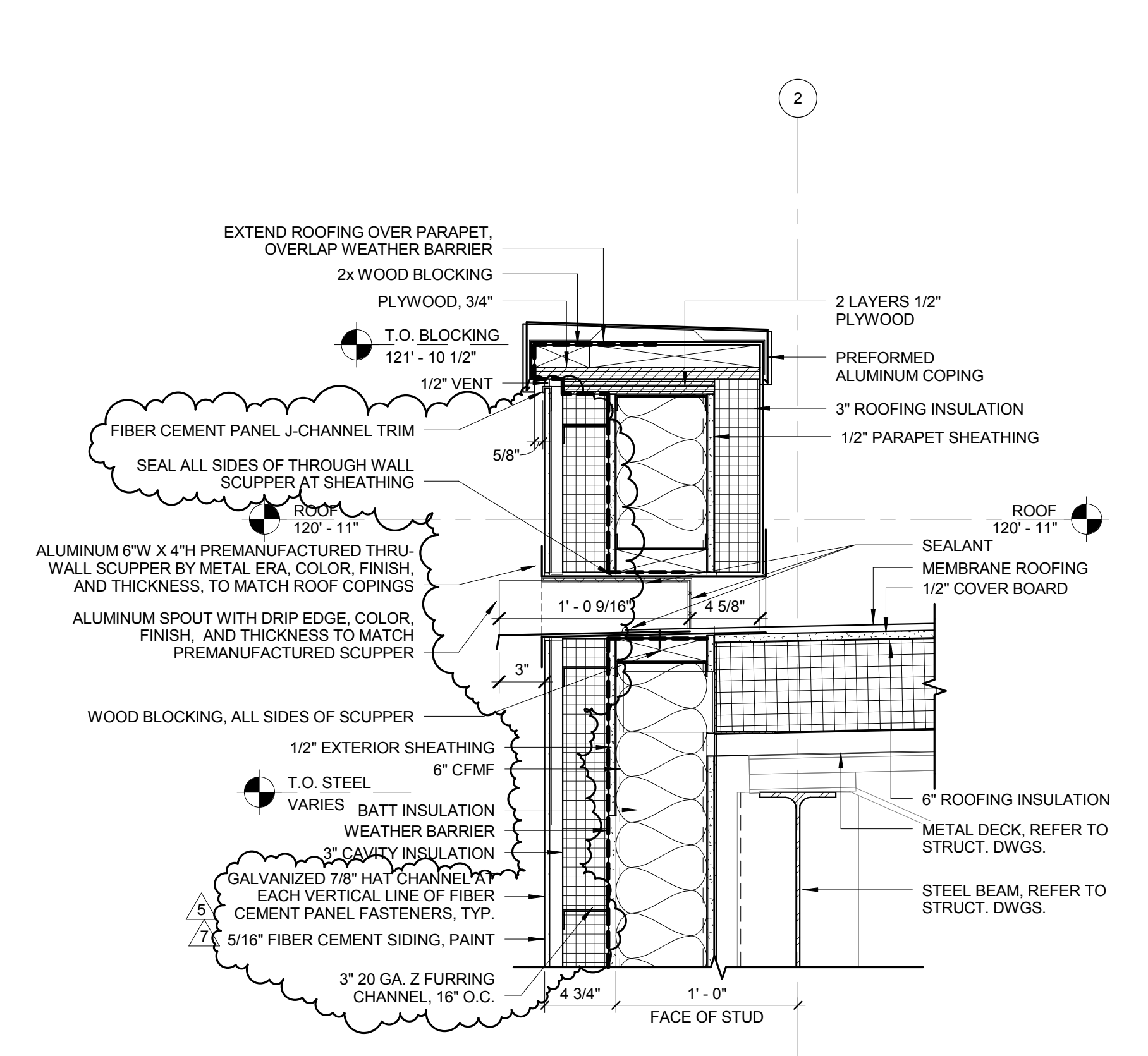
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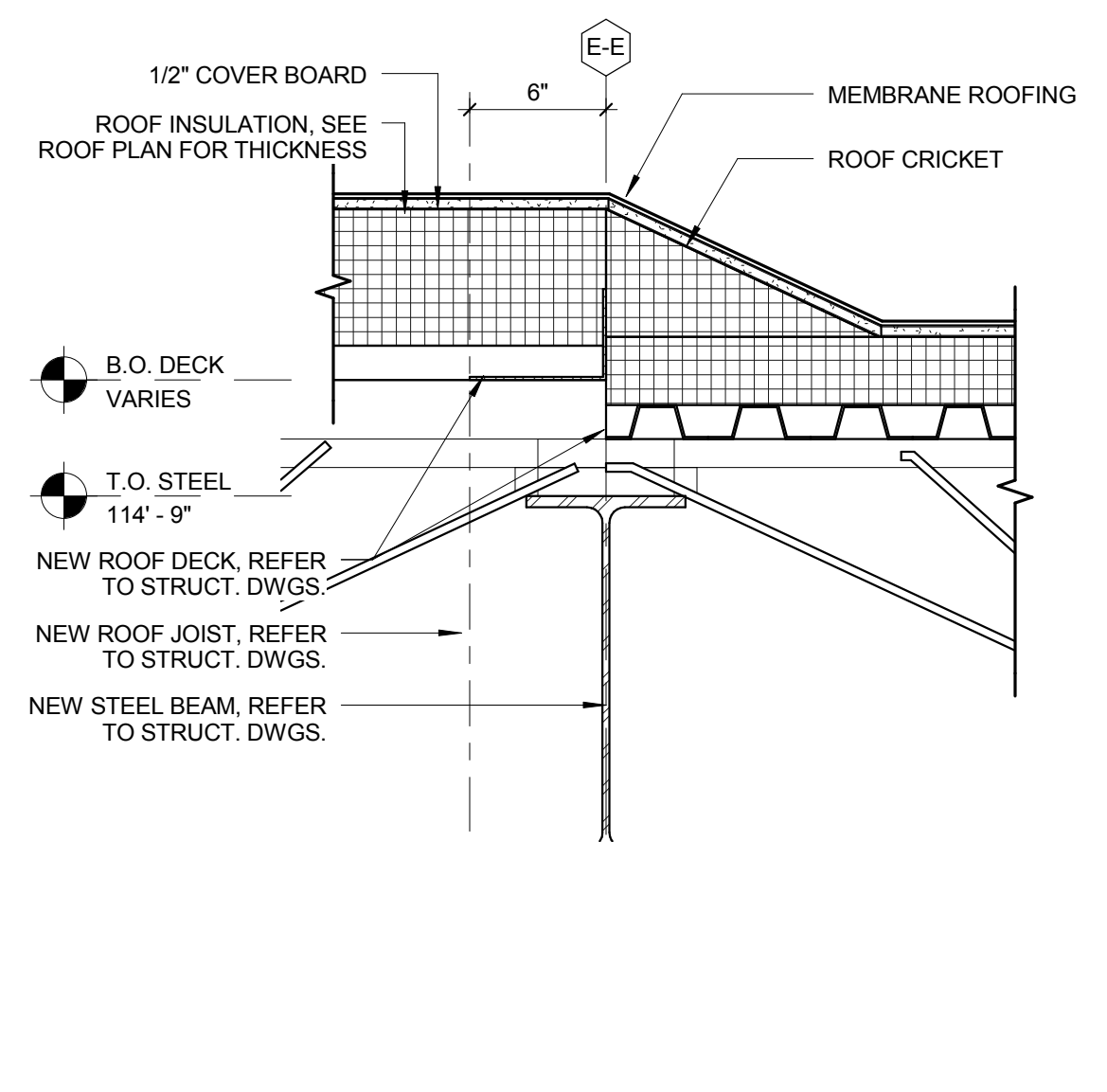
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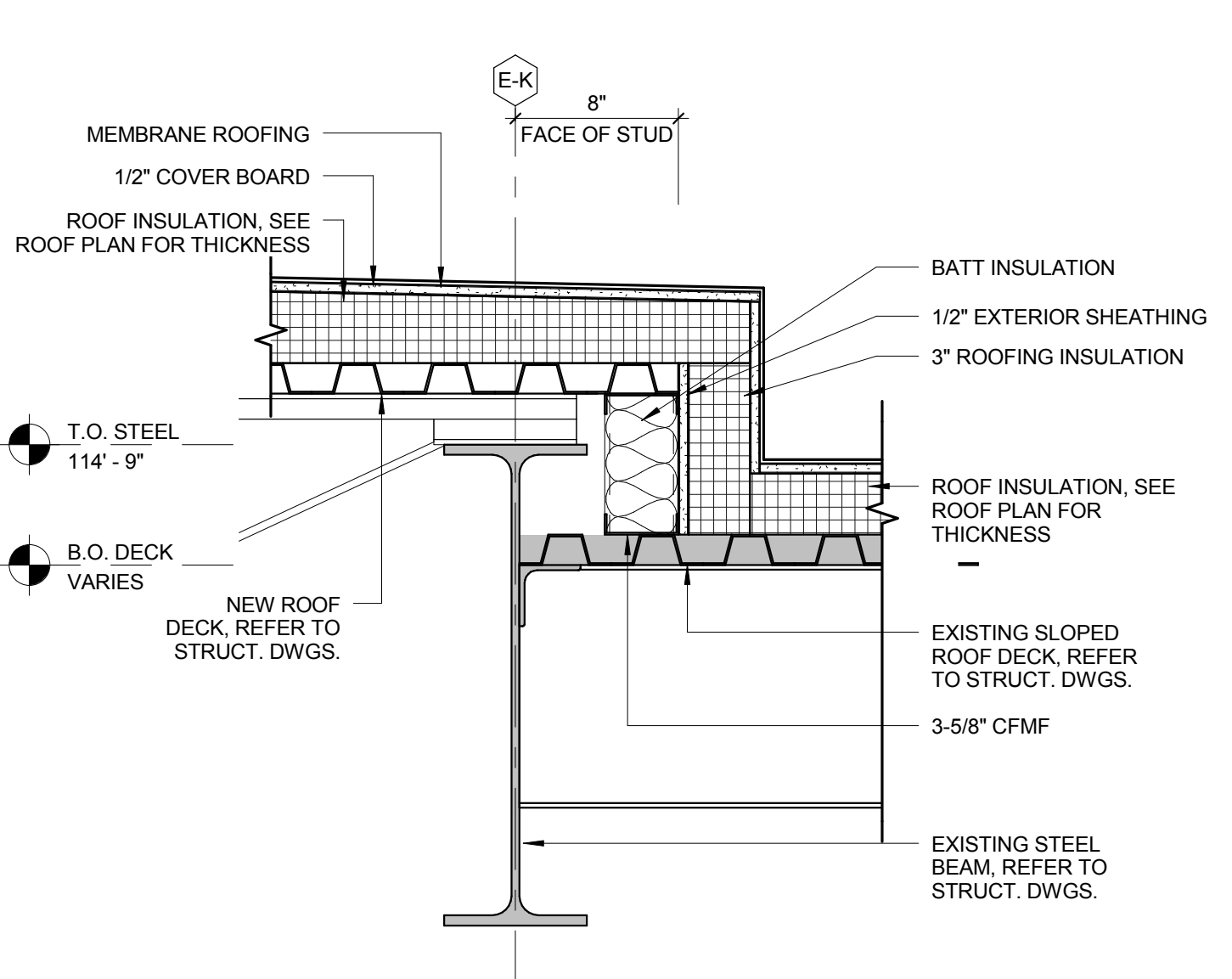
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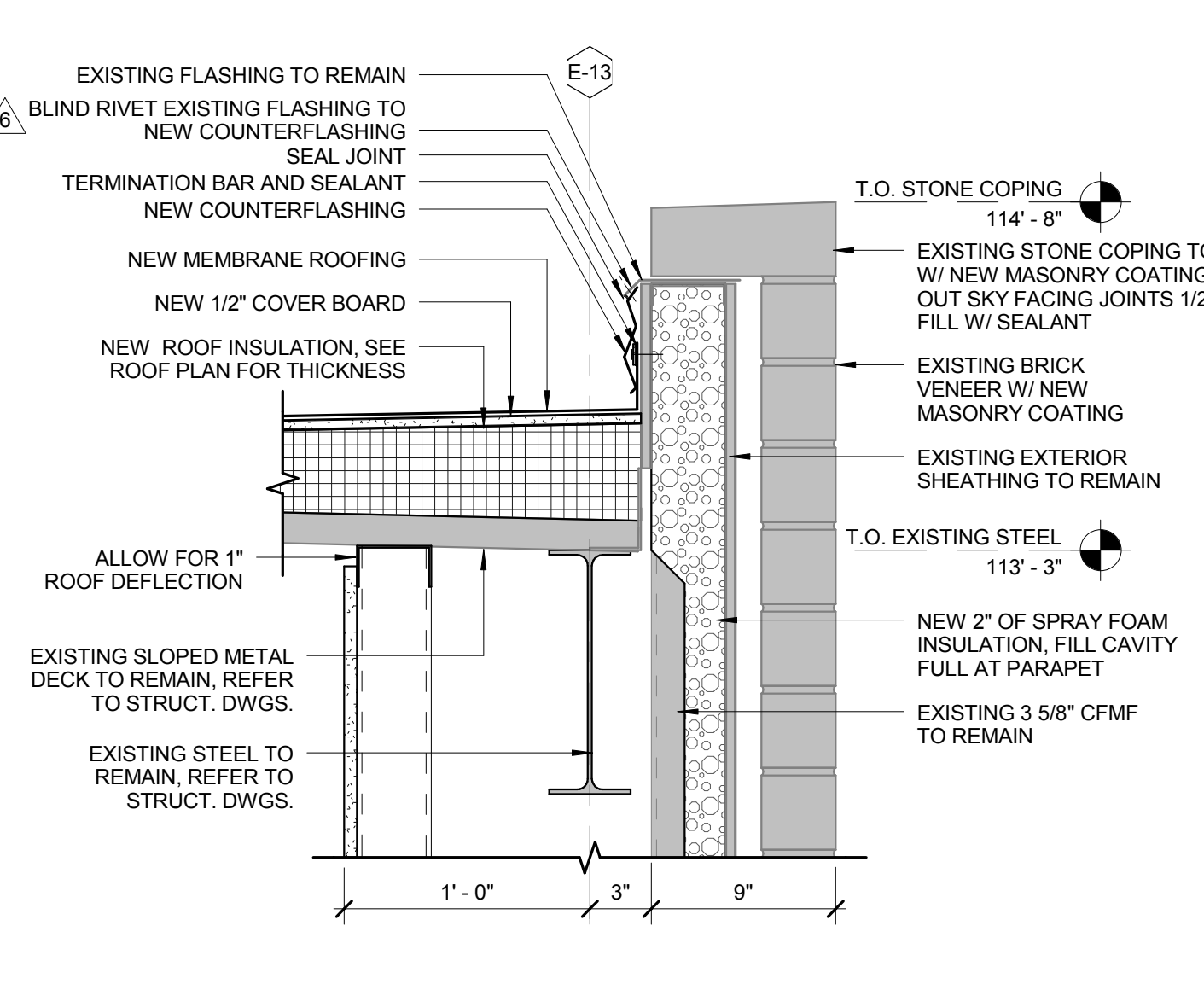
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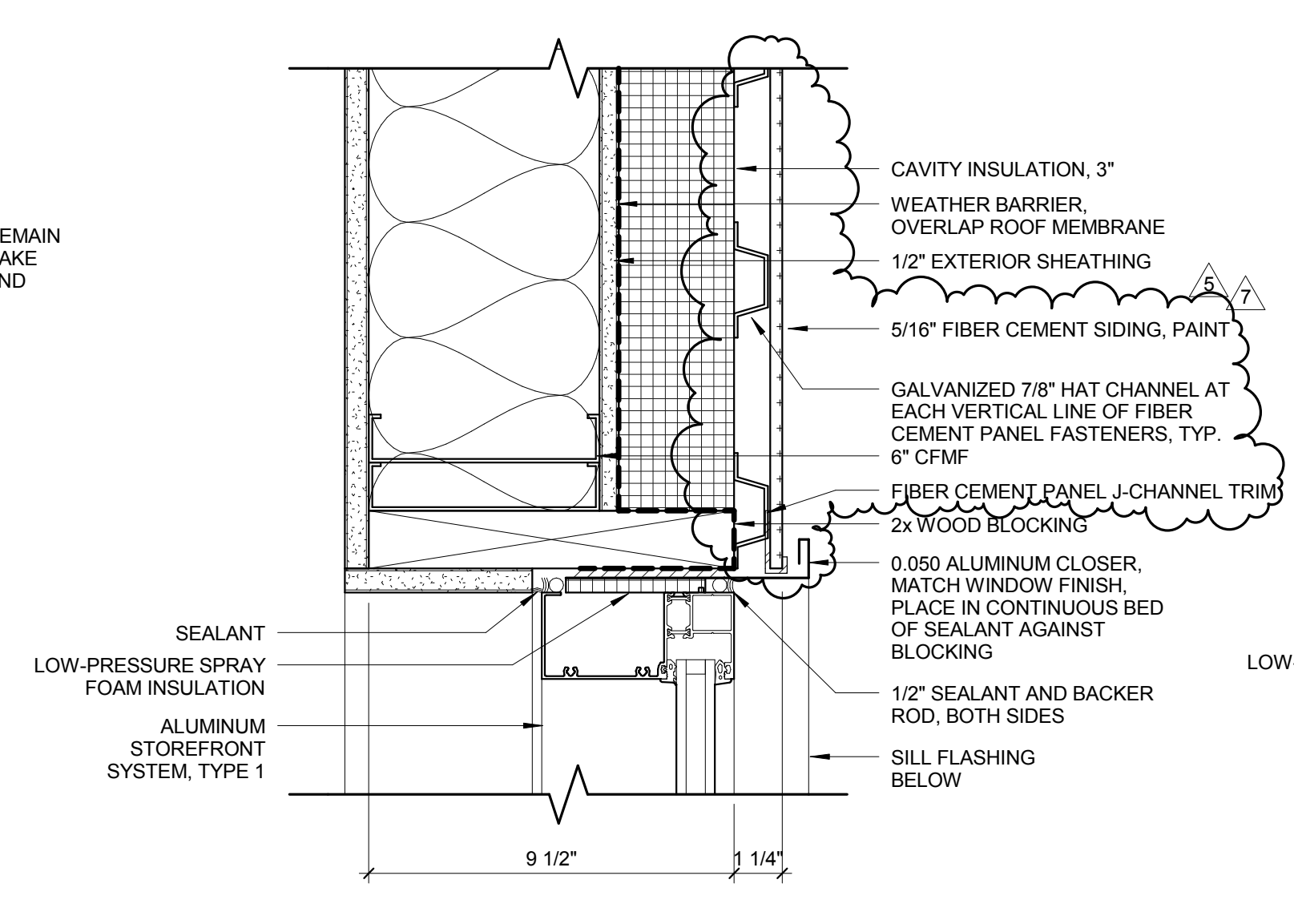
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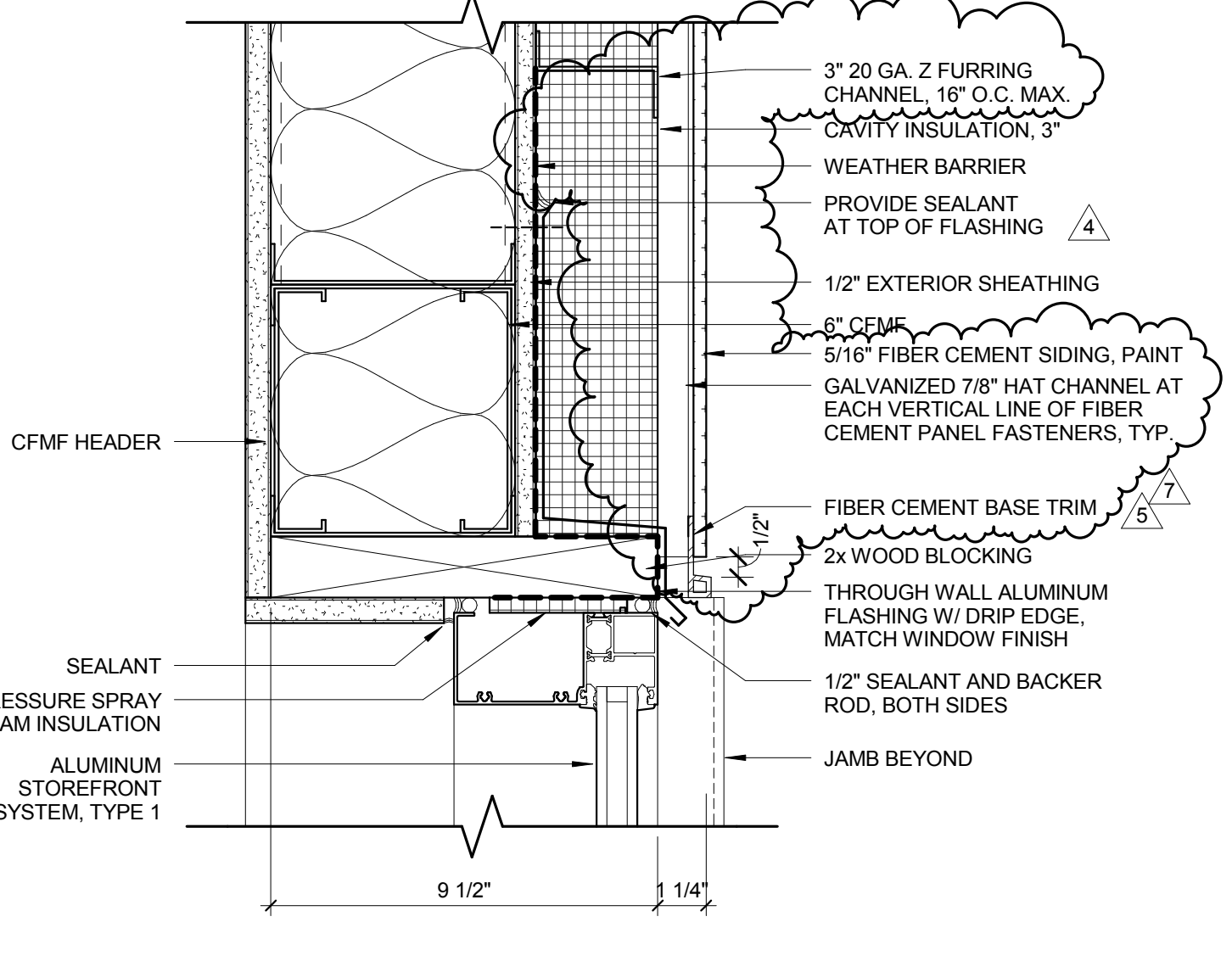
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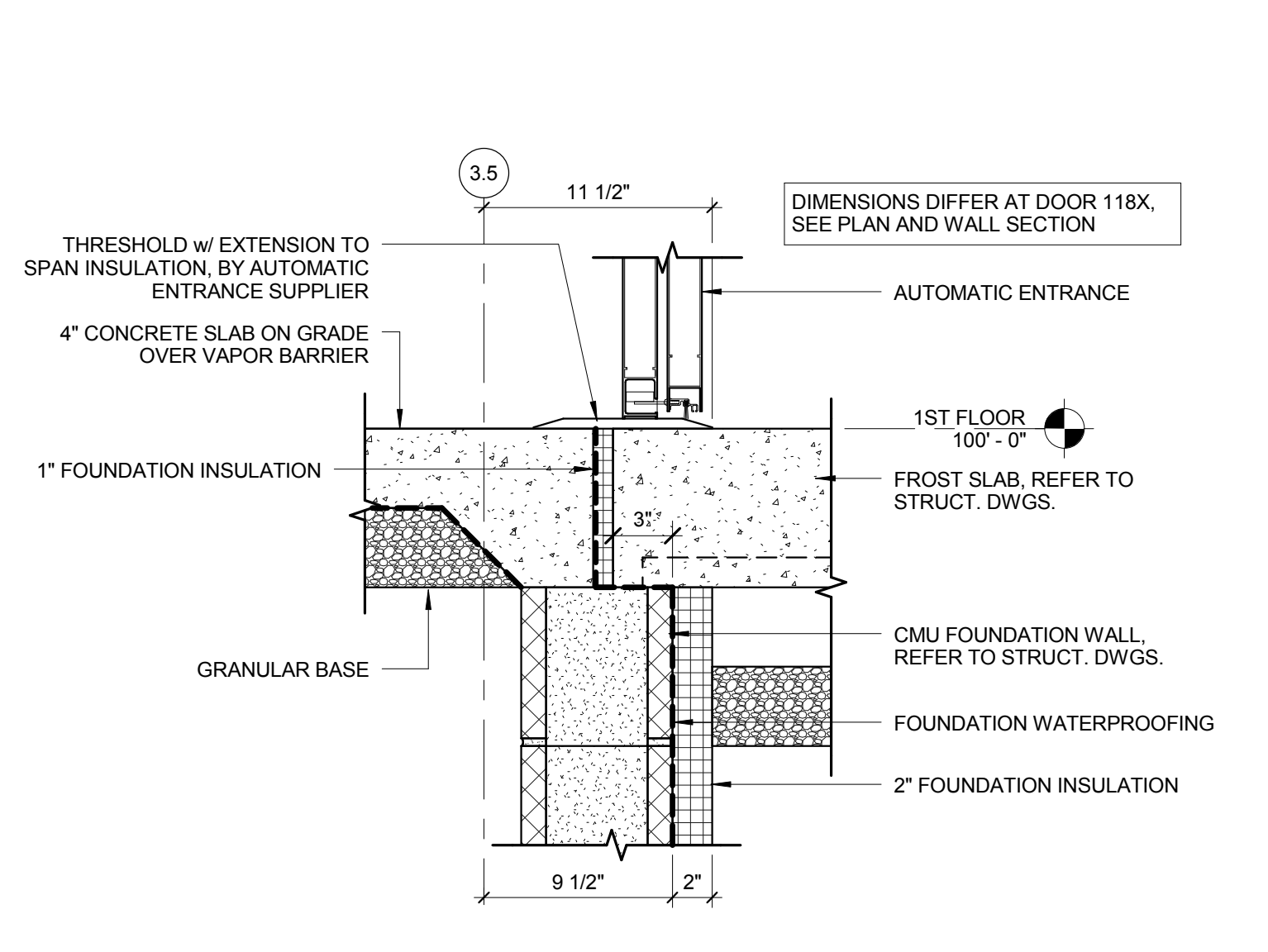
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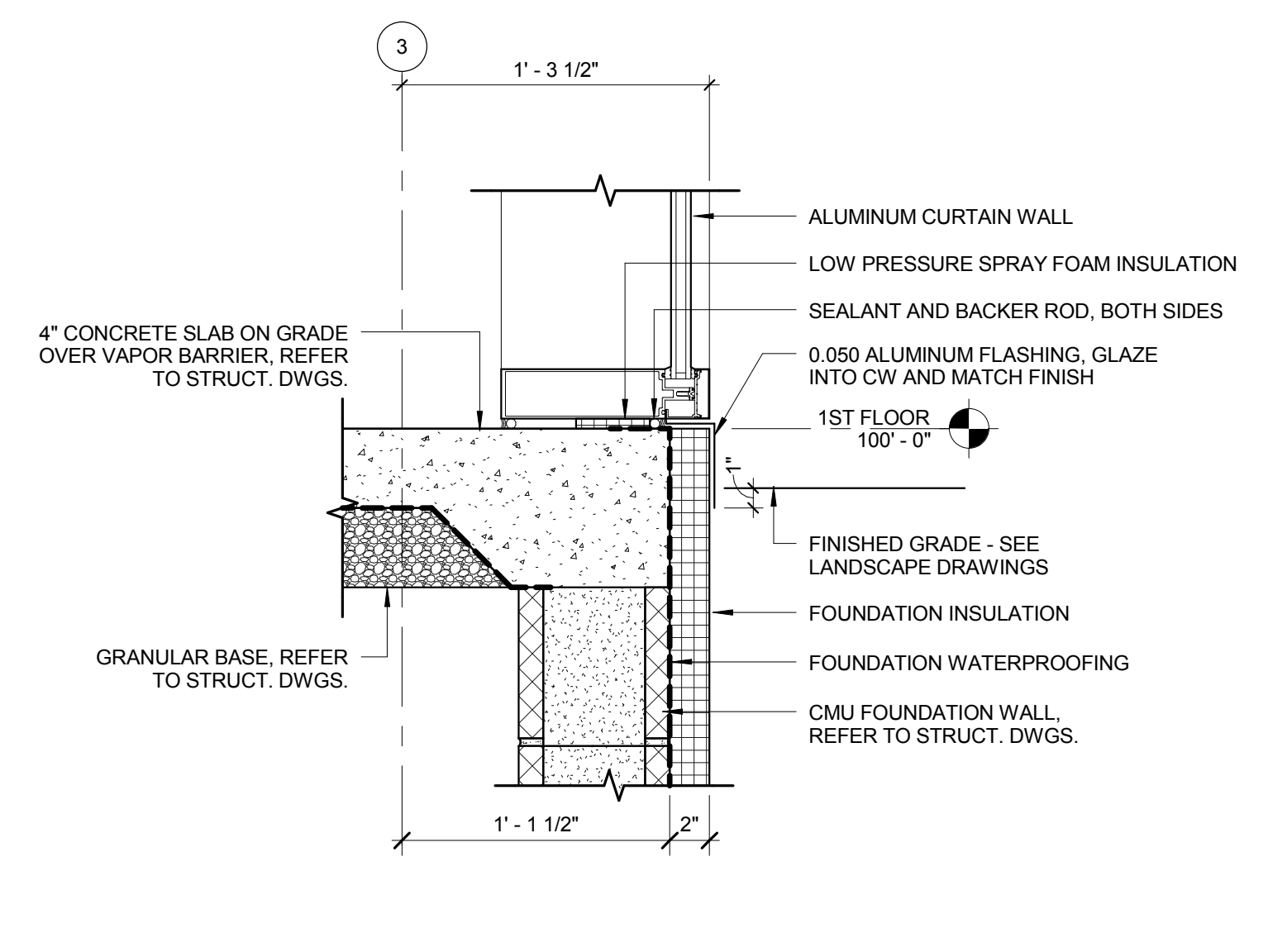
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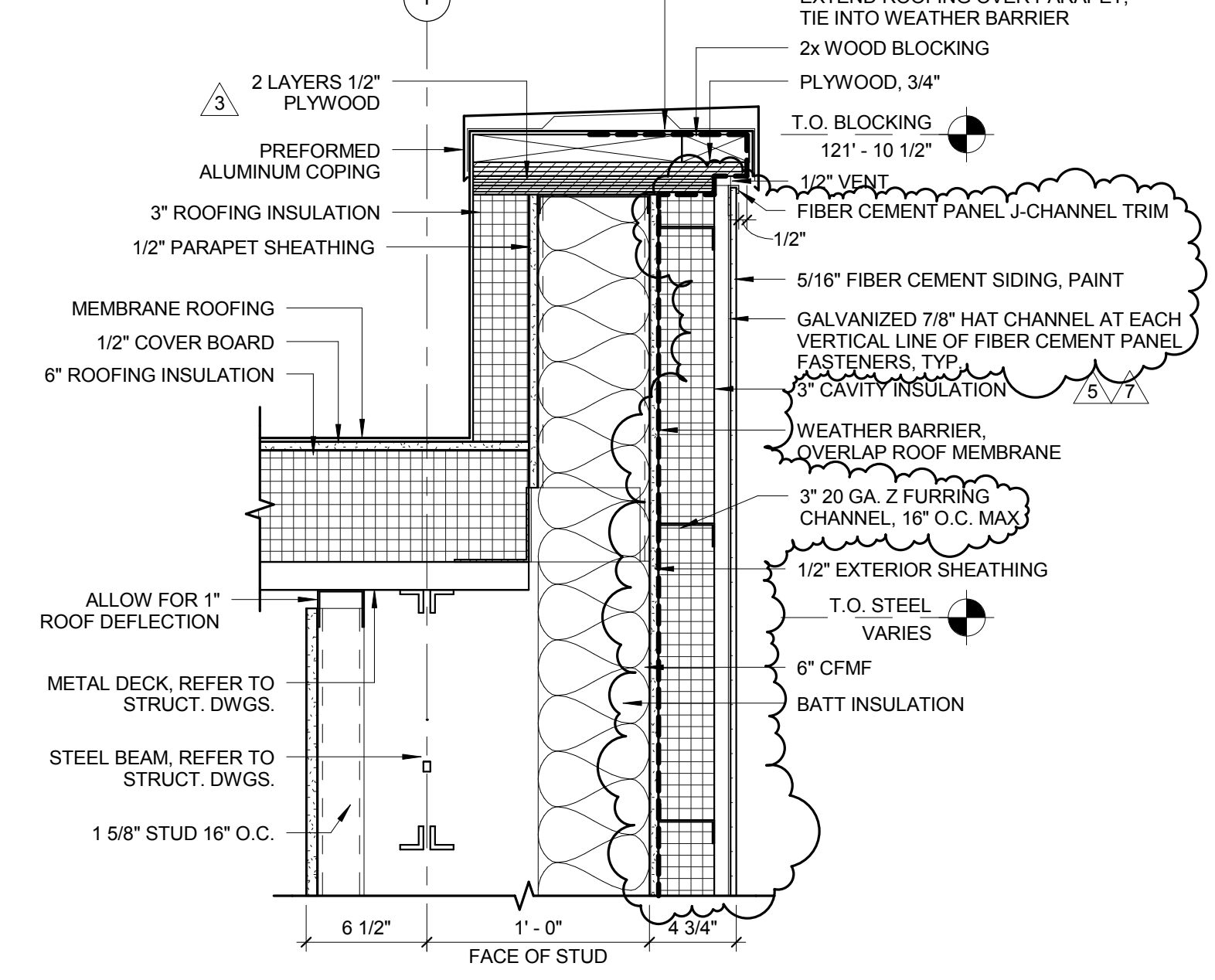
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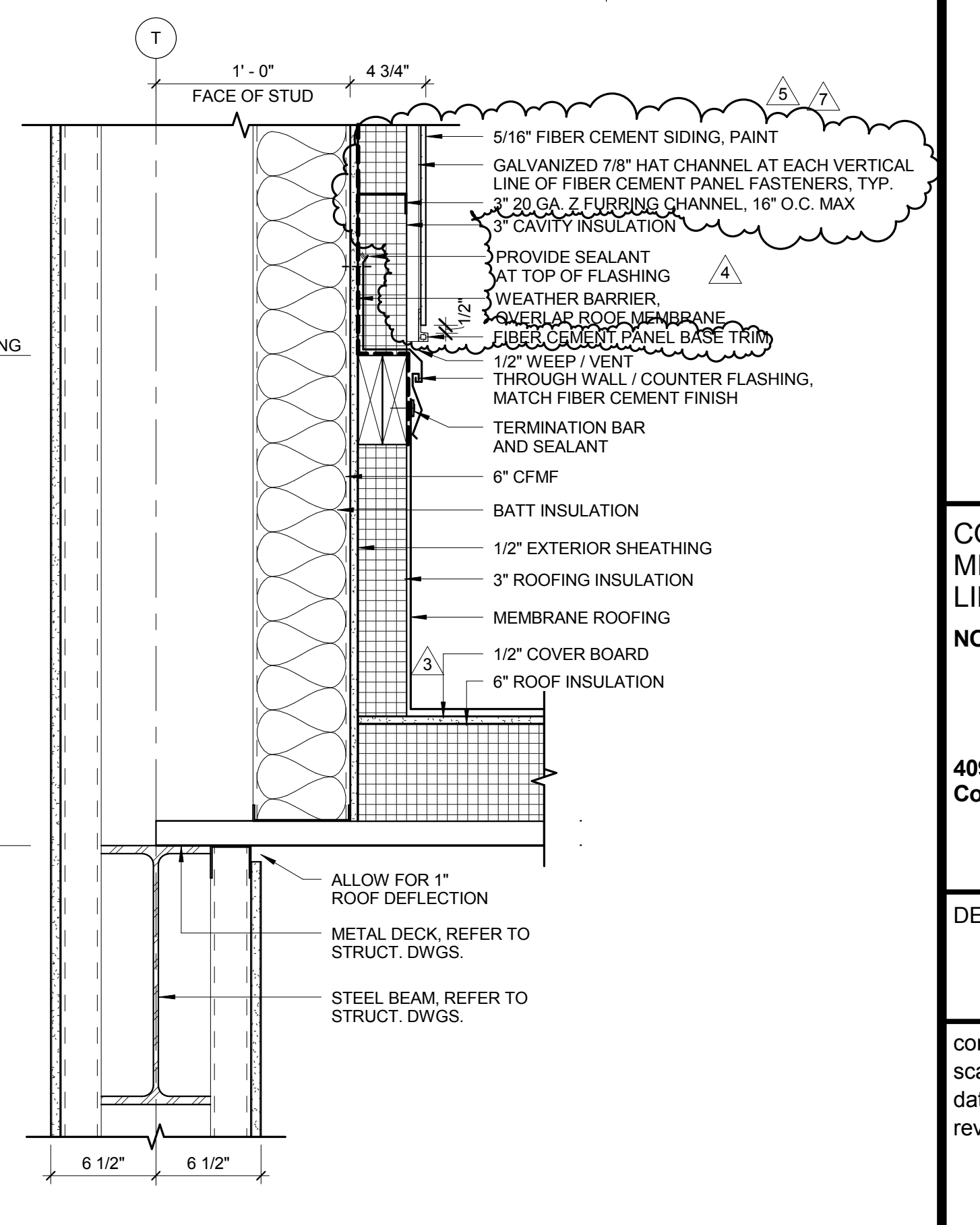
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1 1/2" = 1'-0"
A5-2



SECTION DETAIL 3
1 1/2" = 1'-0"
A5-2



SECTION DETAIL 2
1 1/2" = 1'-0"
A5-2



SECTION DETAIL 1
1 1/2" = 1'-0"
A5-2

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A5-2



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462 South Ludlow Alley Columbus, OH 43215 phone 614 621 2709 fax 614 621 3004

CIVIL AND MEP ENGINEER
Kongta
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#	DATE	DESCRIPTION
1	17 Dec 2015	Bulletin 013
2	04 Jan 2016	FC 008
3	06 Jan 2016	Bulletin 015
4	08 Jan 2016	Bulletin 017
5	14 Jan 2016	Bulletin 015 REVISD
6	18 Jan 2016	Bulletin 014 REVISD
7	20 Jan 2016	Bulletin 019
8	19 Jan 2016	FC 010
9	19 Jan 2016	FC 011
10	11 May 2016	Bulletin 044
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12	Date 60	Bulletin 047

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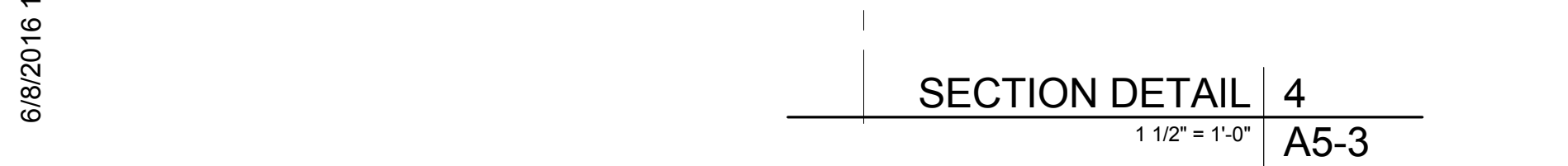
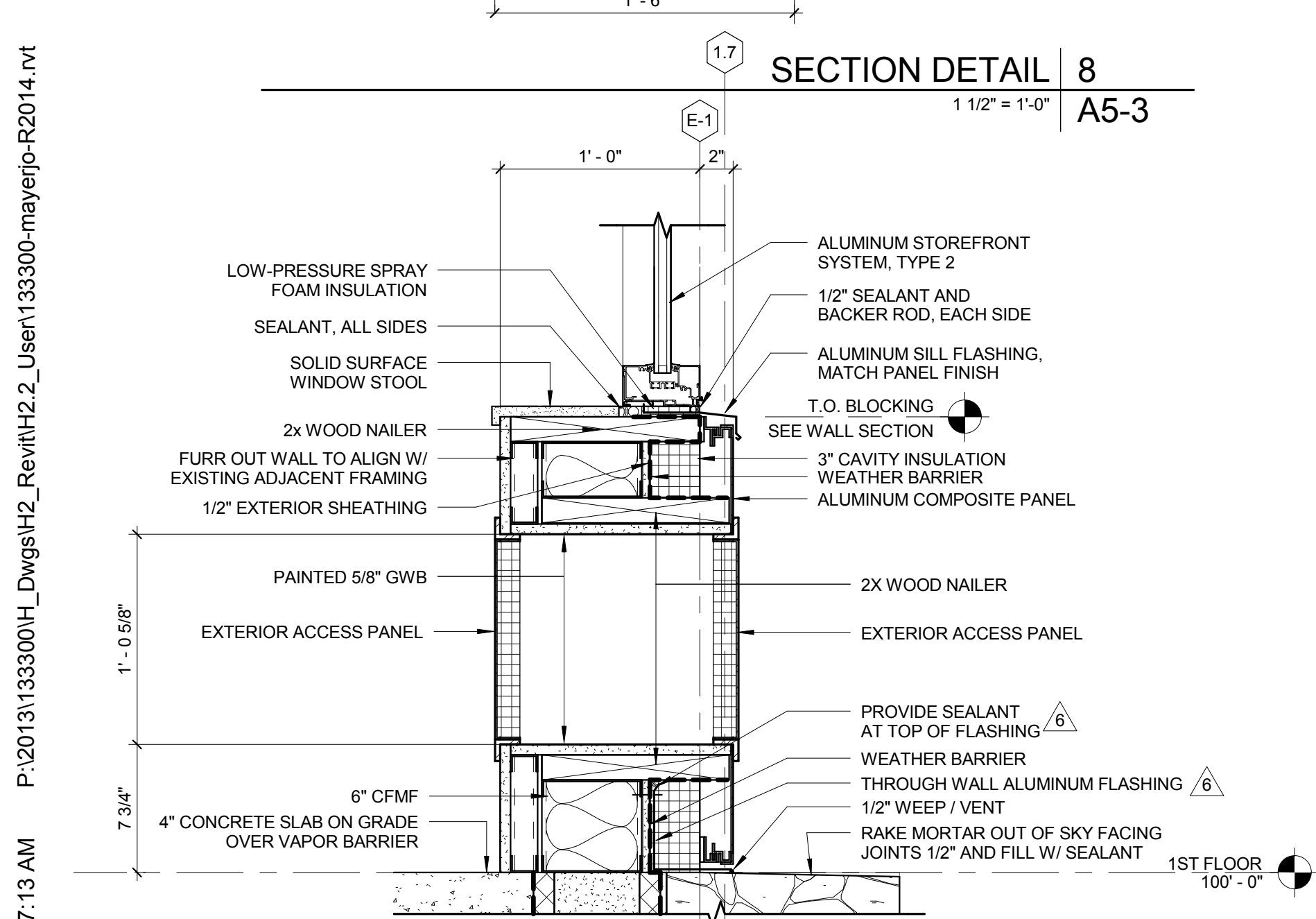
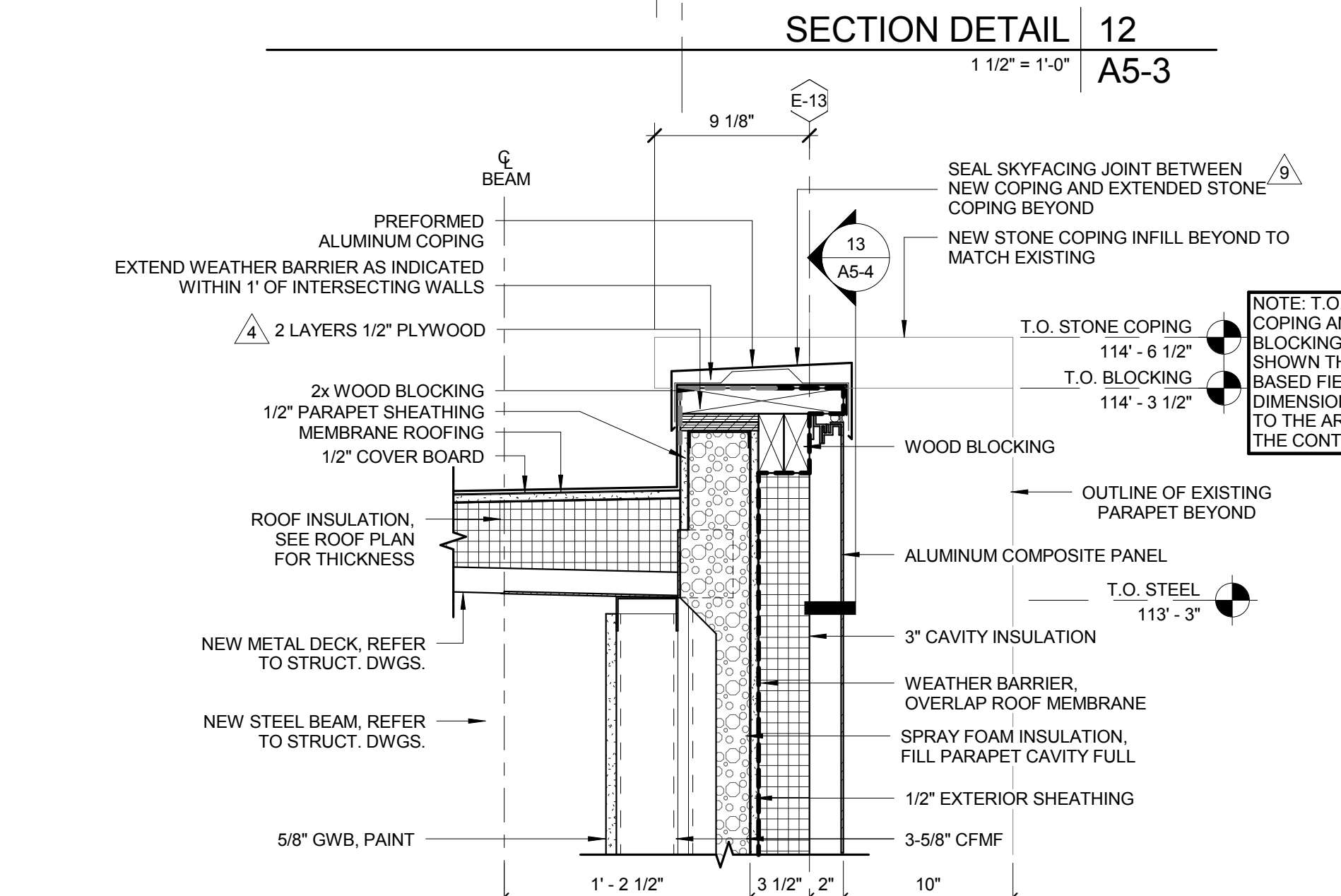
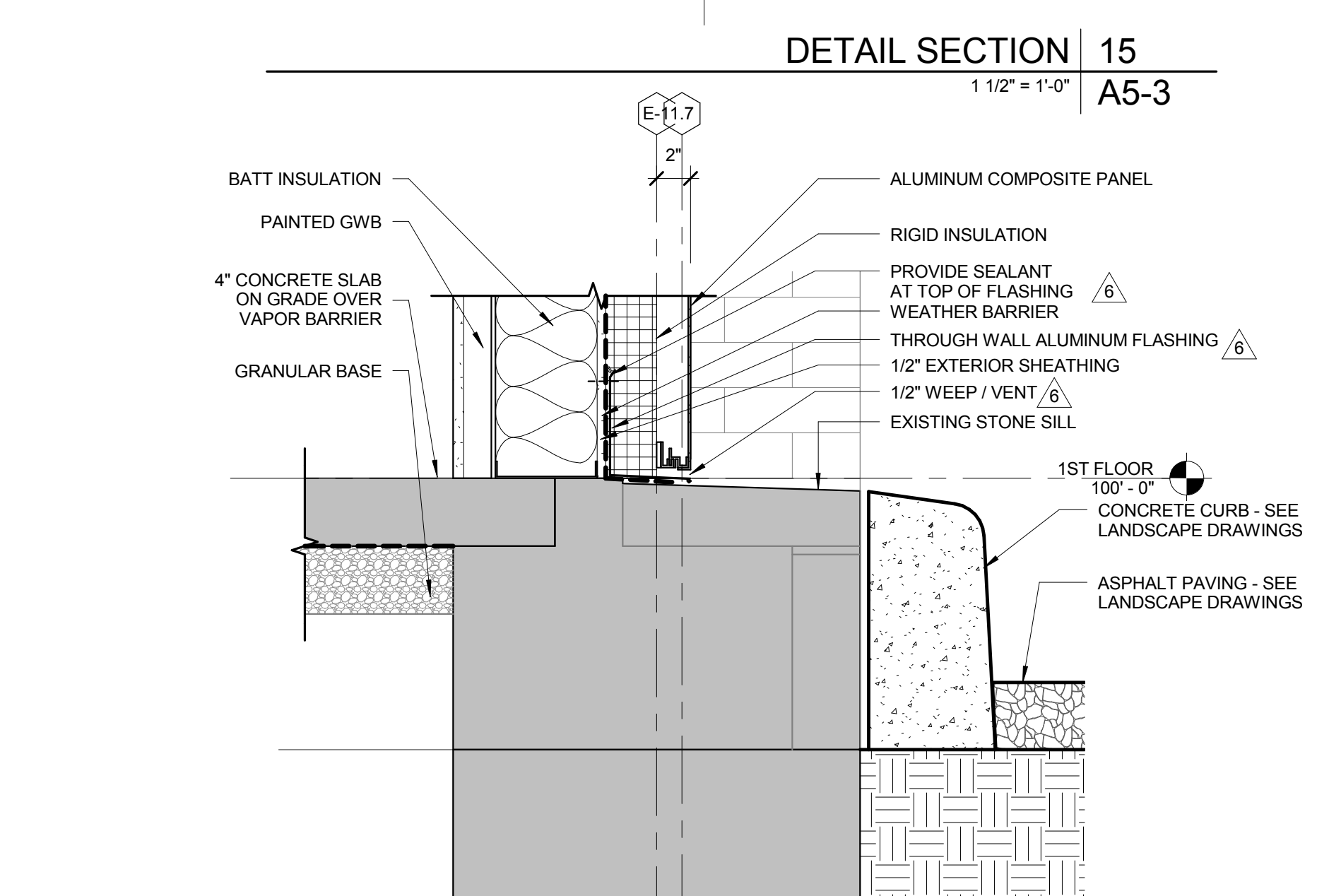
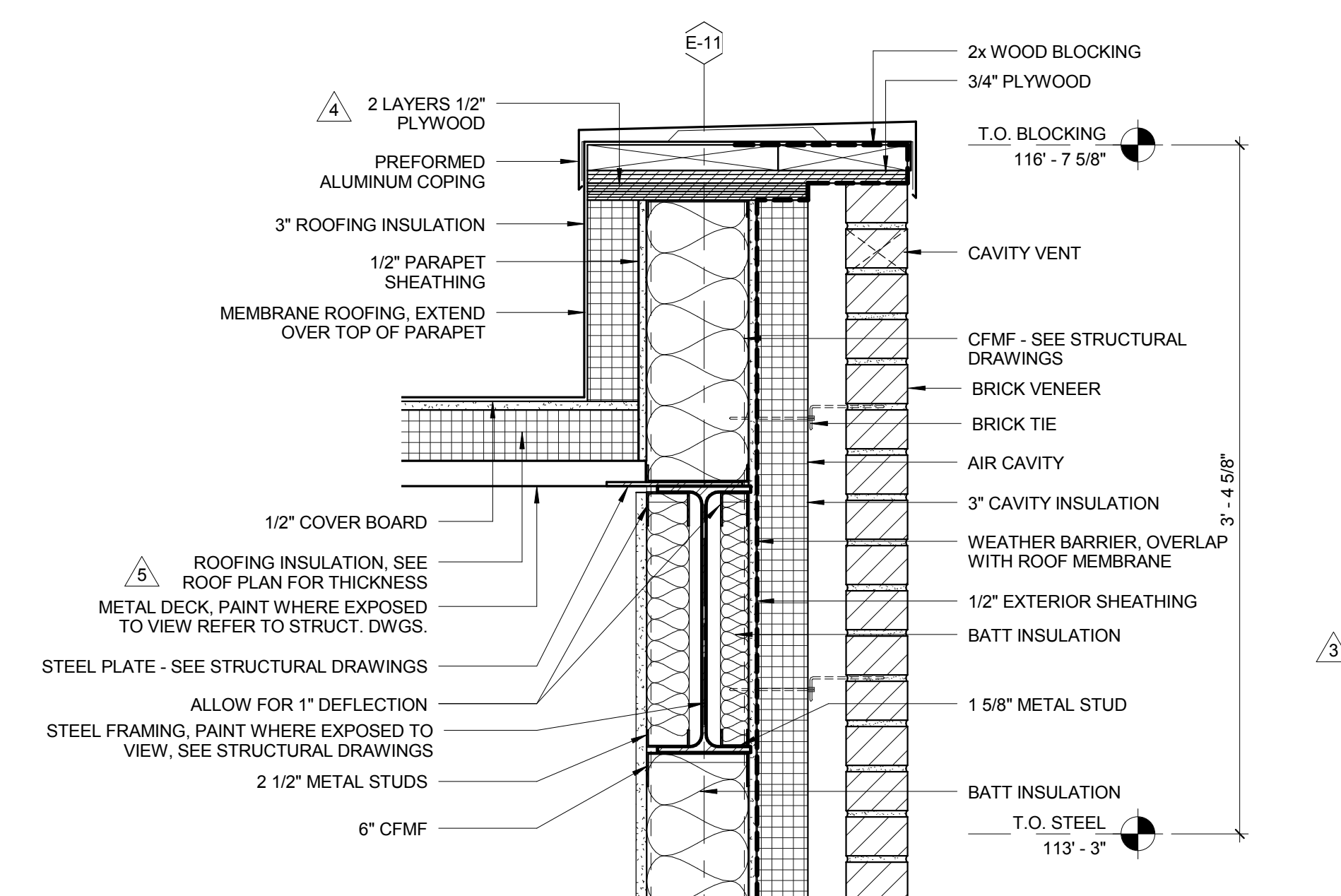
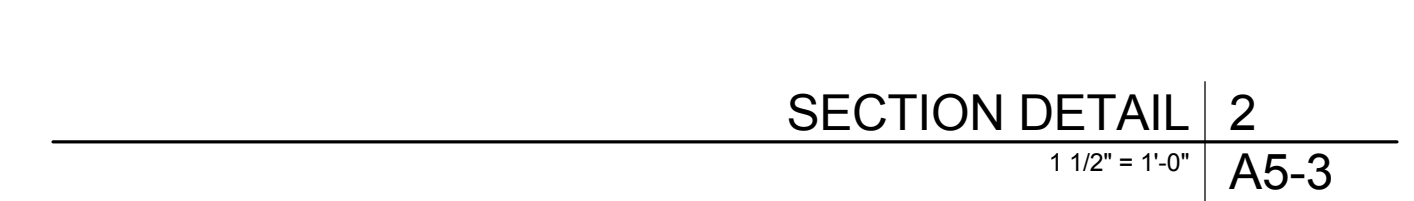
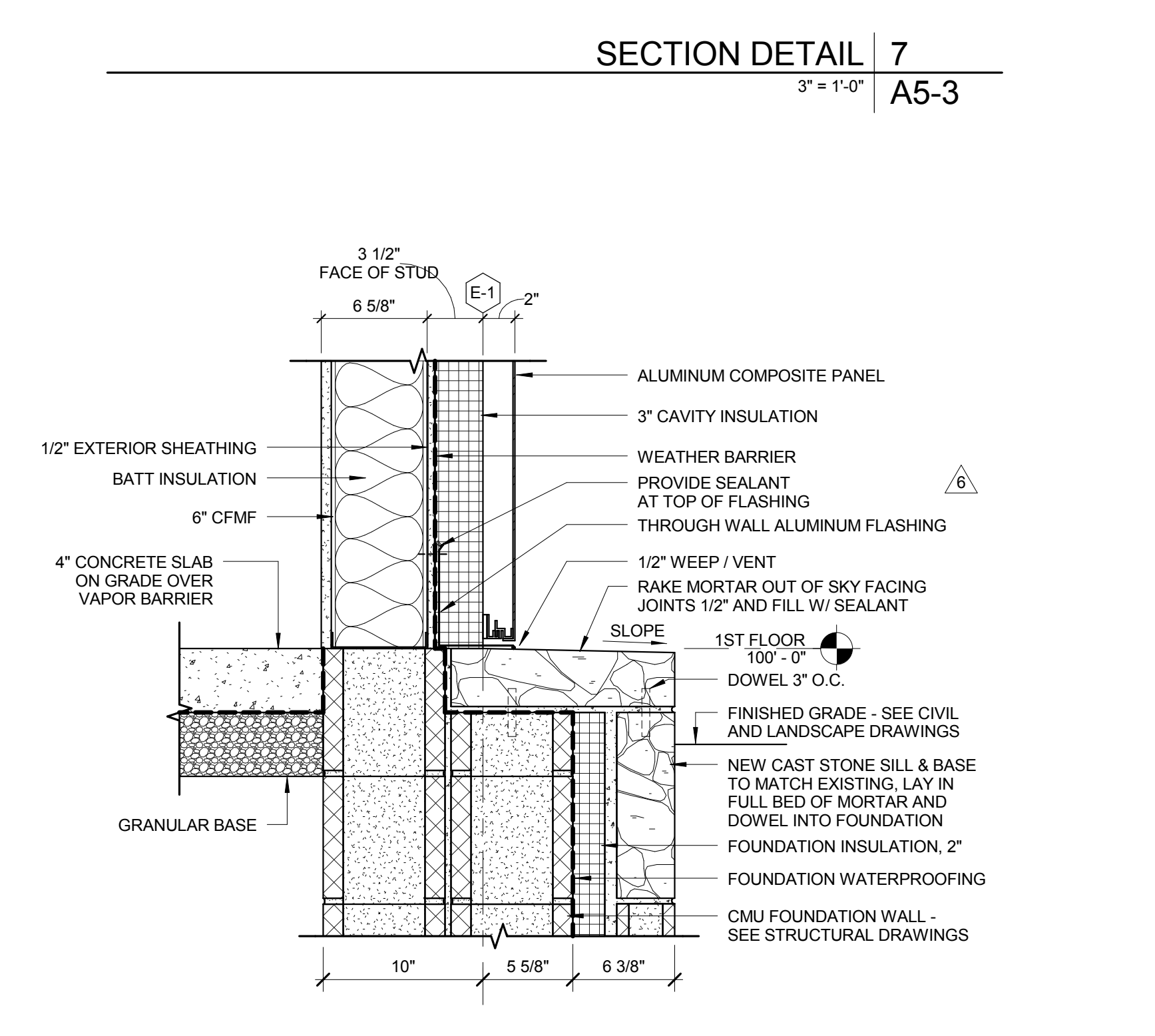
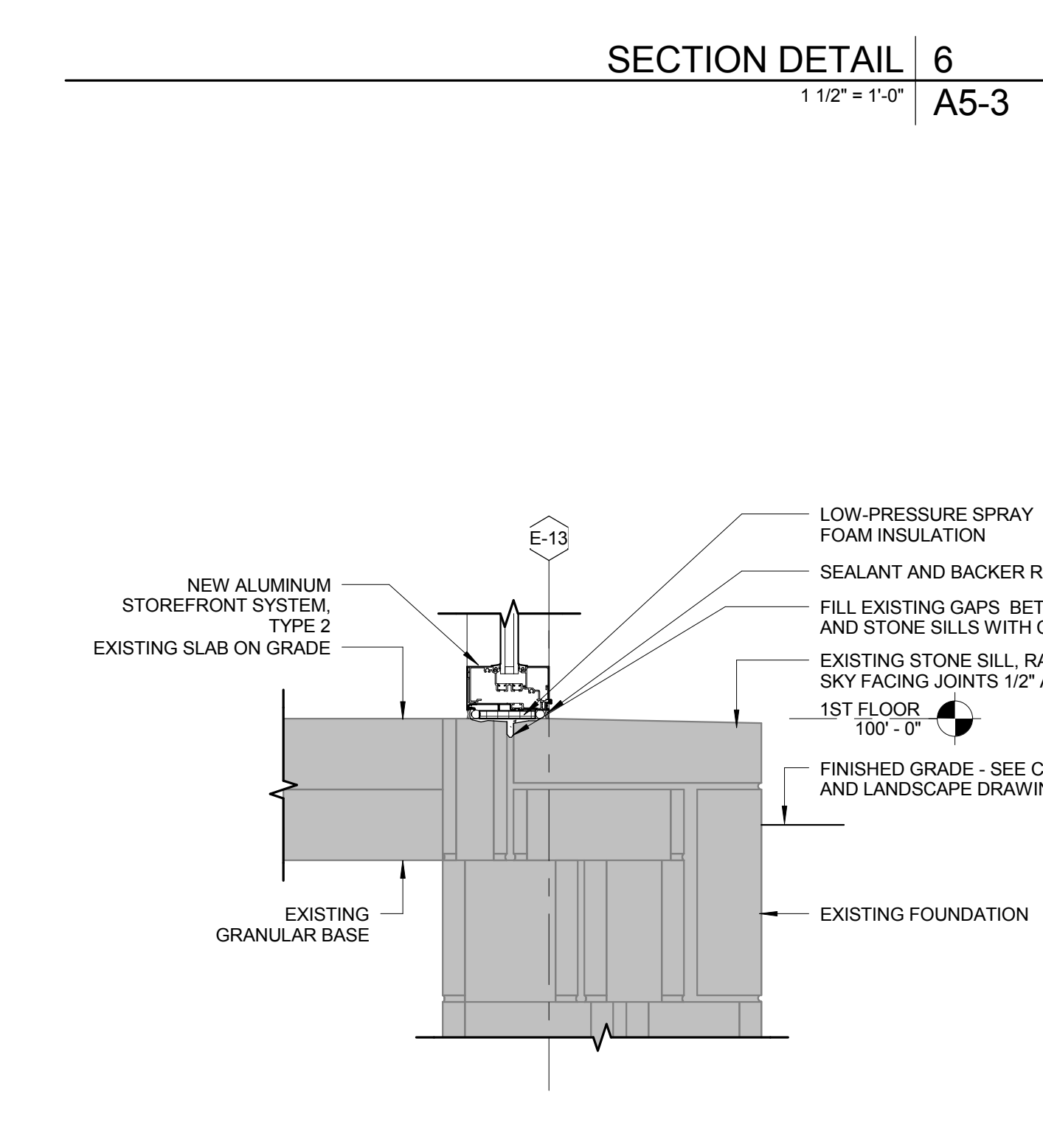
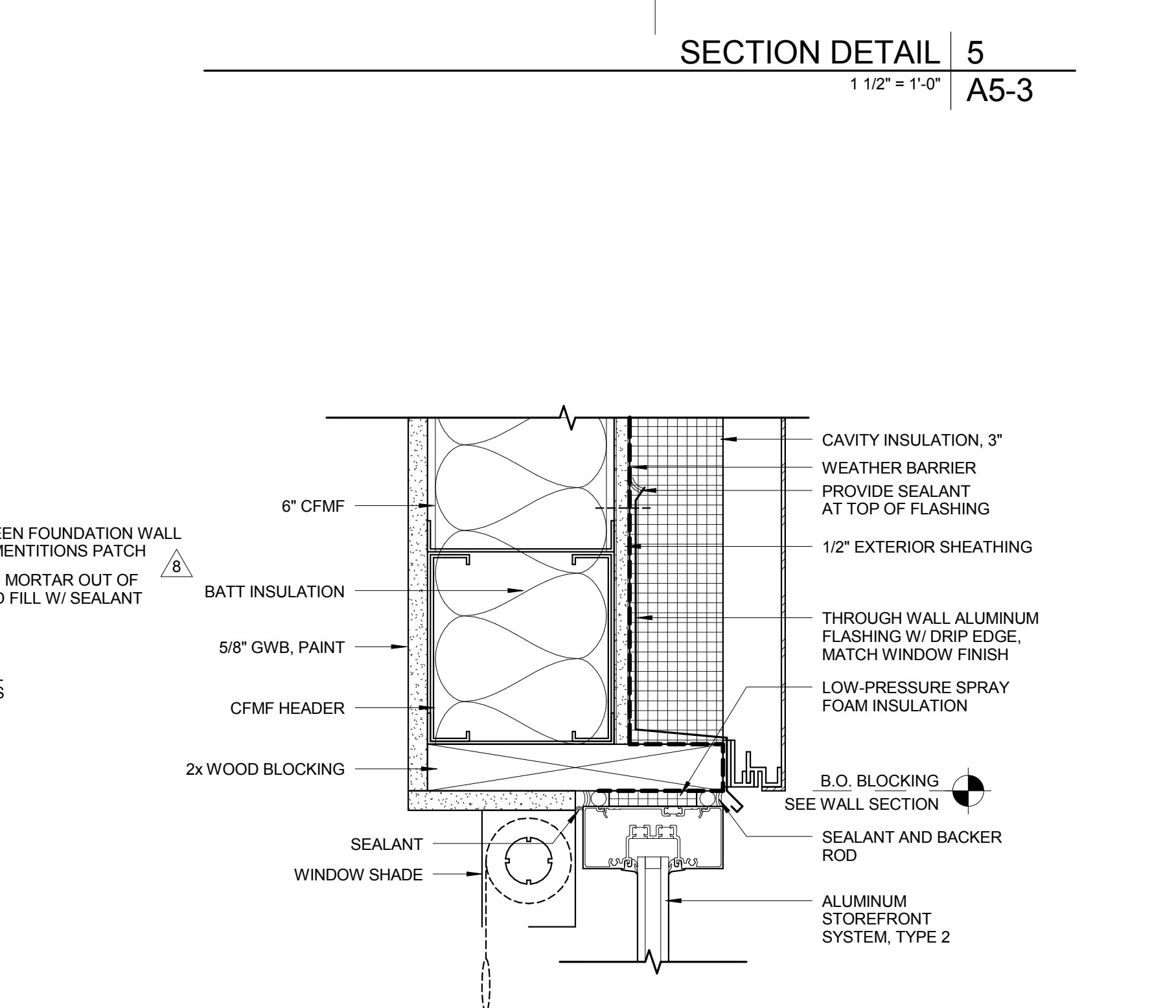
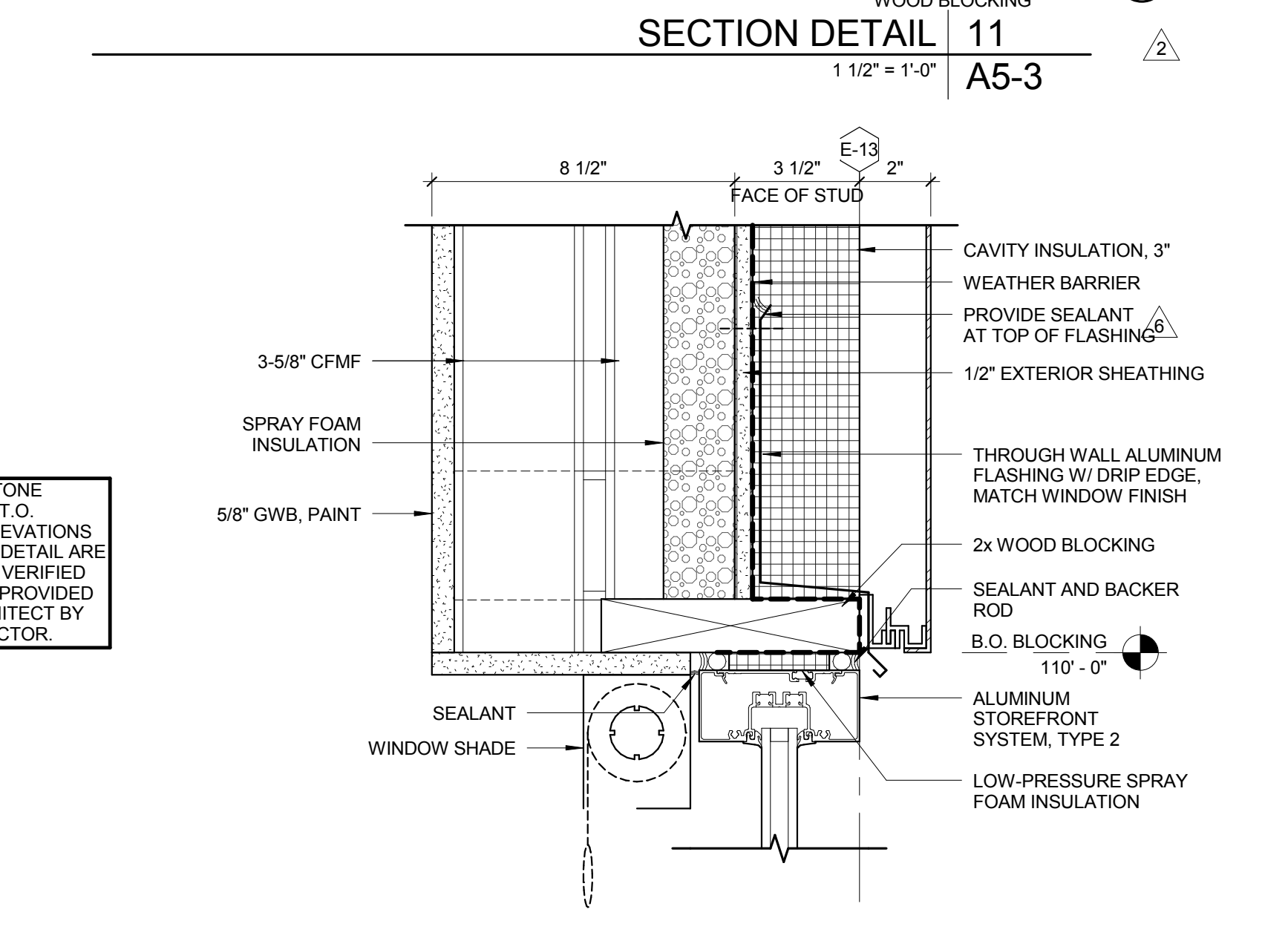
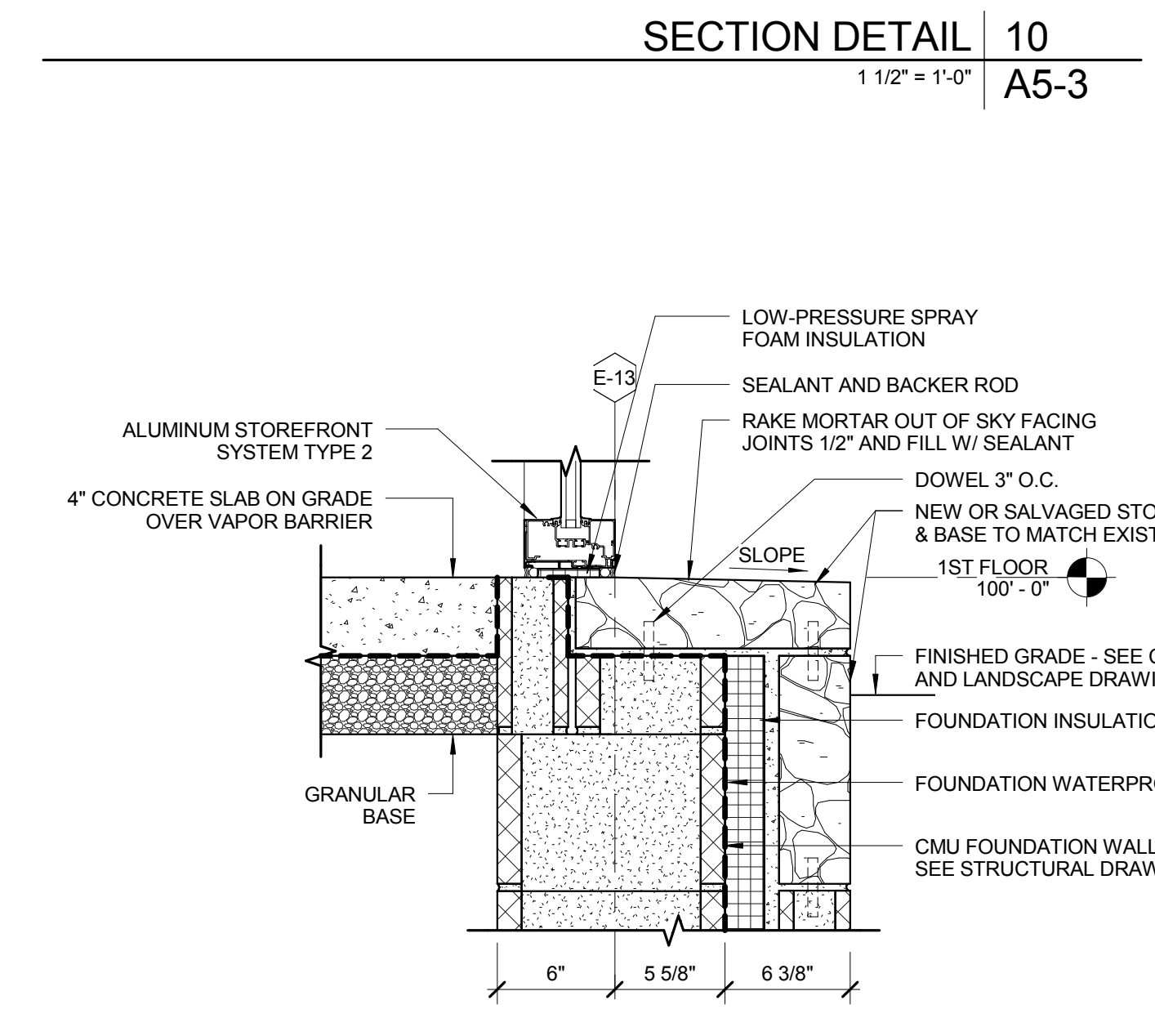
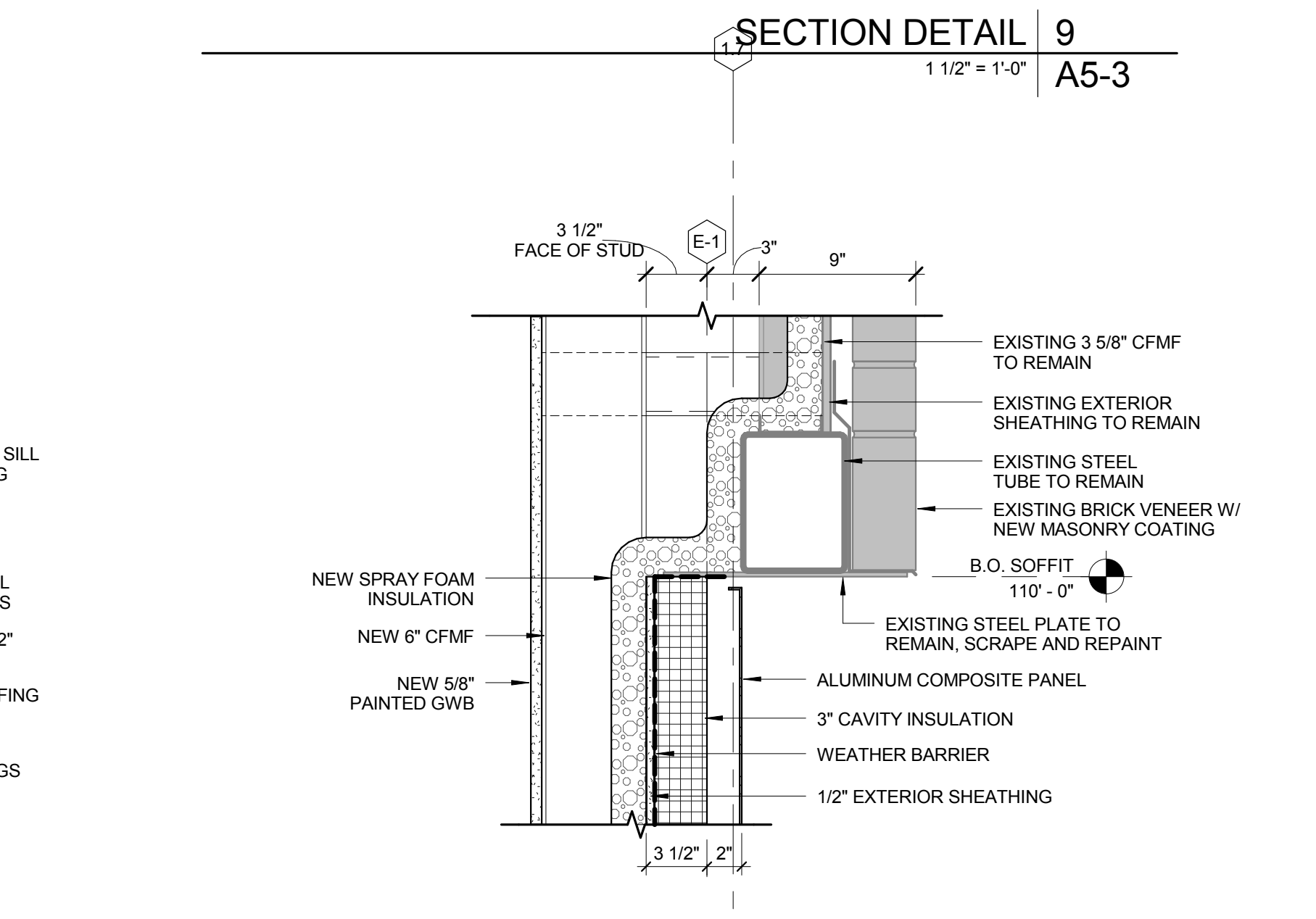
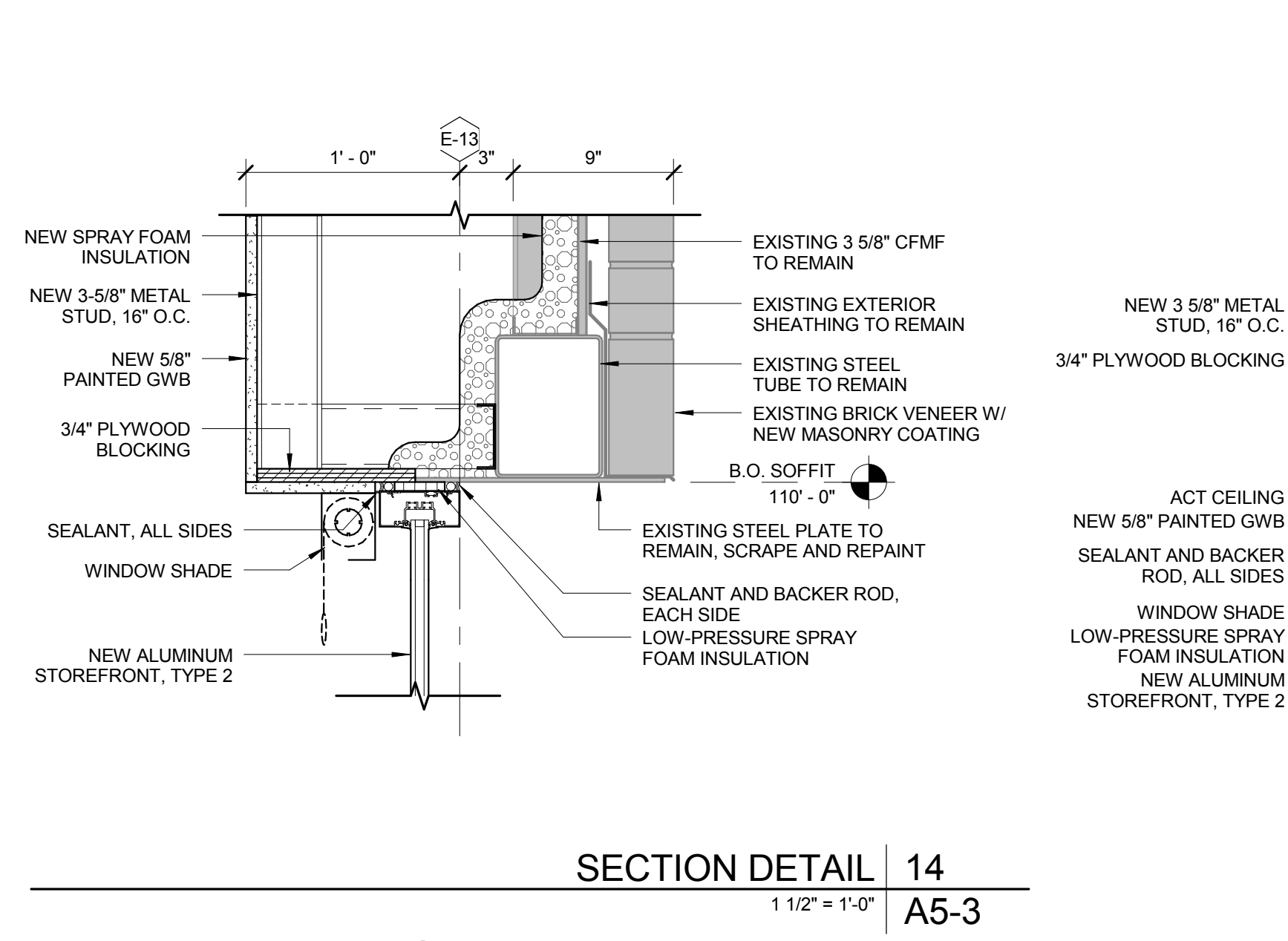
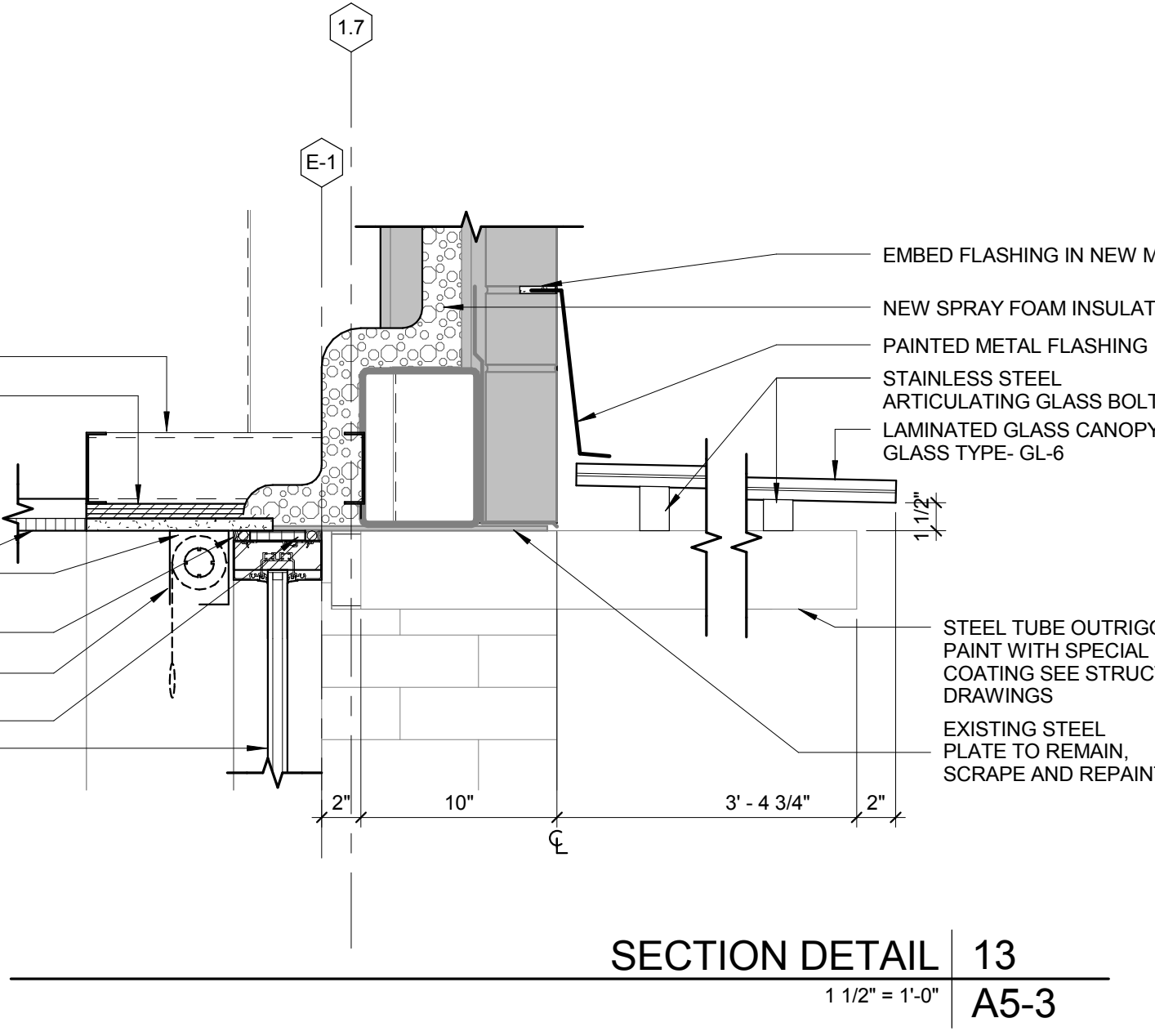
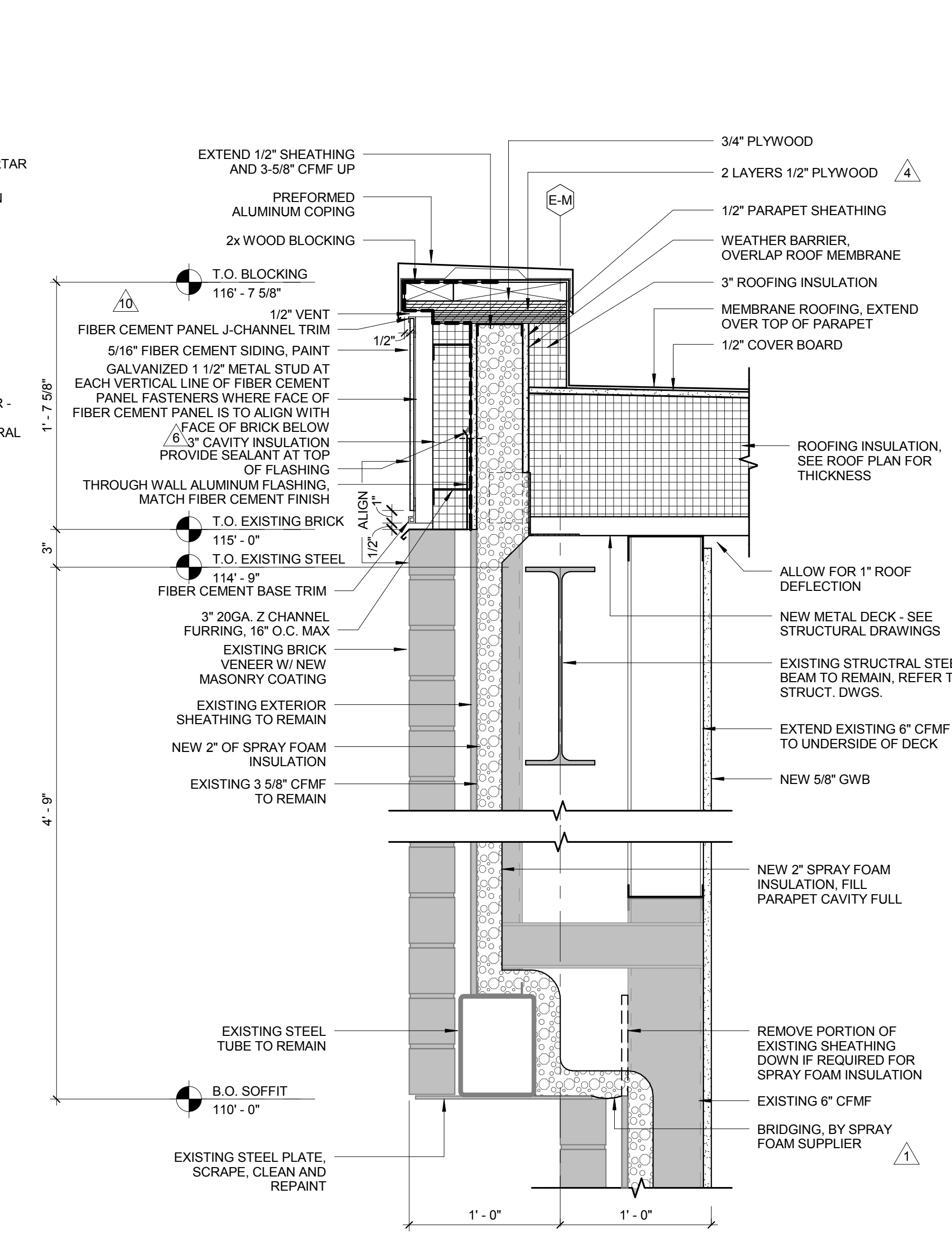
4093 Cleveland Ave.
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commission no. 1333.00
scale: As indicated
date: 04.21.2016
revised:

BID DRAWINGS

A5-3



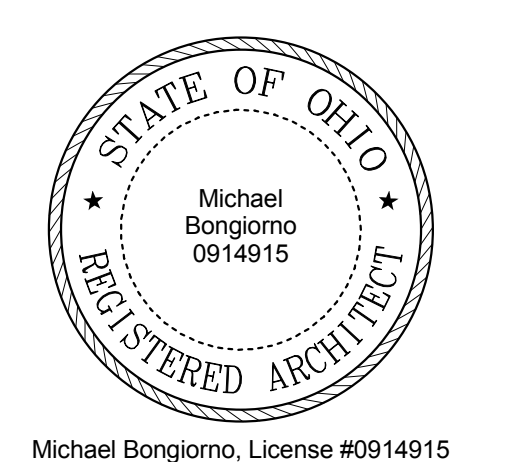
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LANDSCAPE ARCHITECT
MKSK
462 South Ludlow Alley Columbus, OH 43215 phone 614 621 2709 fax 614 621 3054

CIVIL AND MEP ENGINEER
Kongra
1650 Watermark Dr. Columbus, OH 43215 phone 614 487 1650 fax 614 487 8861

STRUCTURAL ENGINEER
SMRH
1166 Dublin Rd., Ste 200 Columbus, Ohio 43215-1038 phone 614 481 9800 fax 614 481 9353

#	DATE	DESCRIPTION
1	19 Jan 2016	FC 011
2	08 Feb 2016	Bulletin 024
3	11 Feb 2016	Bulletin 026
4	11 May 2016	Bulletin 044
5	31 May 2016	Bulletin 044 REVISED
6	04 Jan 2017	Bulletin 065

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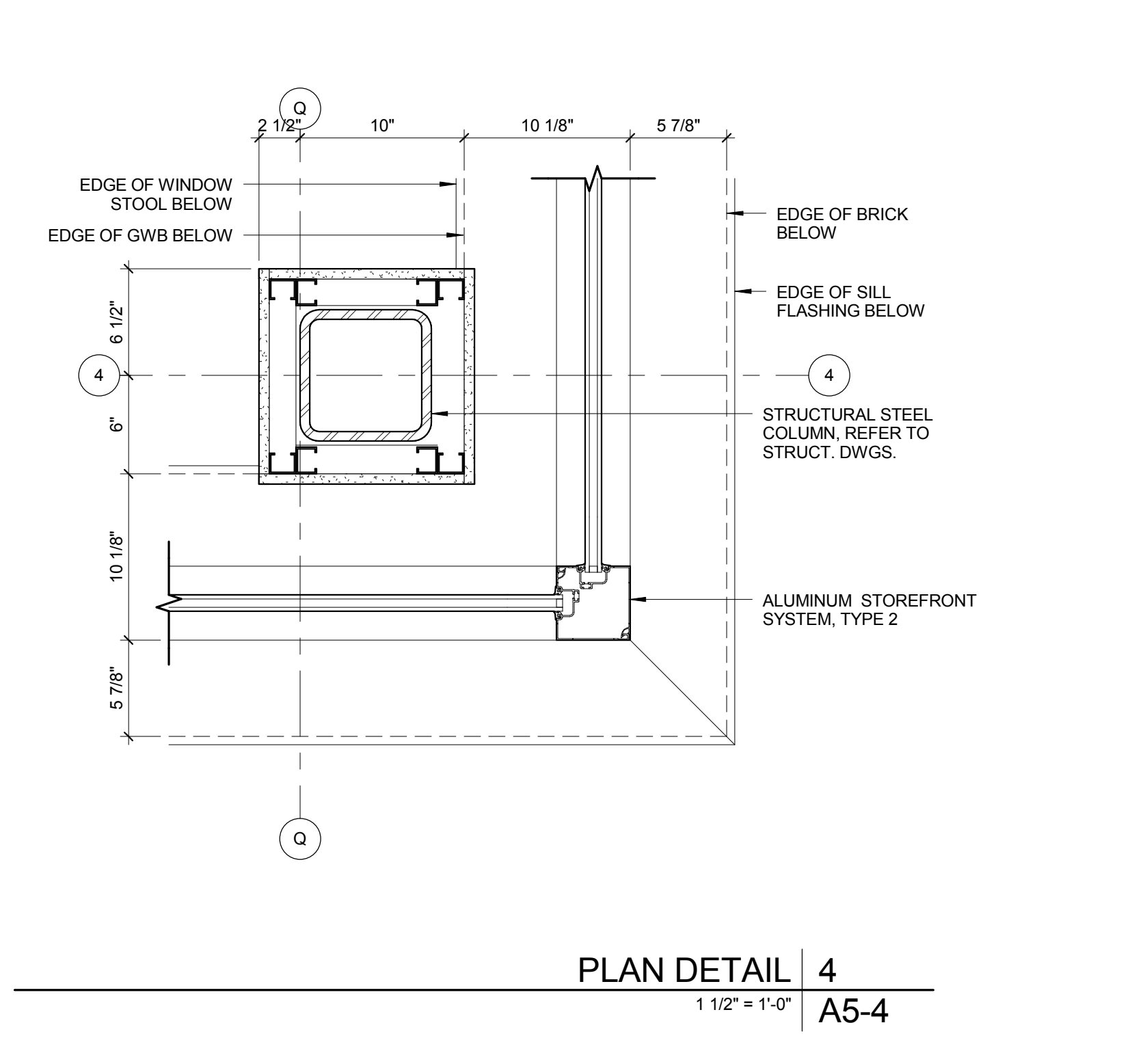
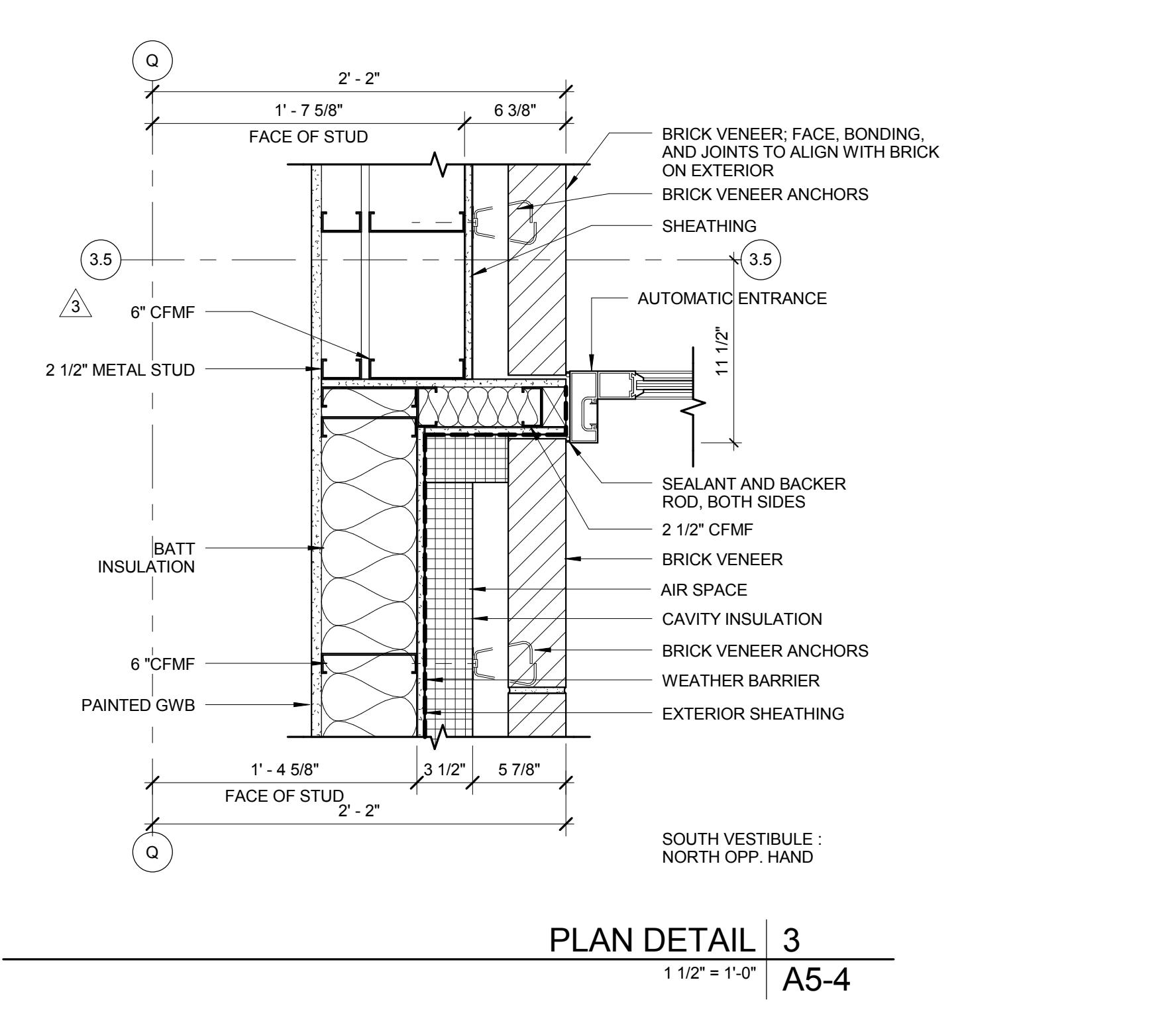
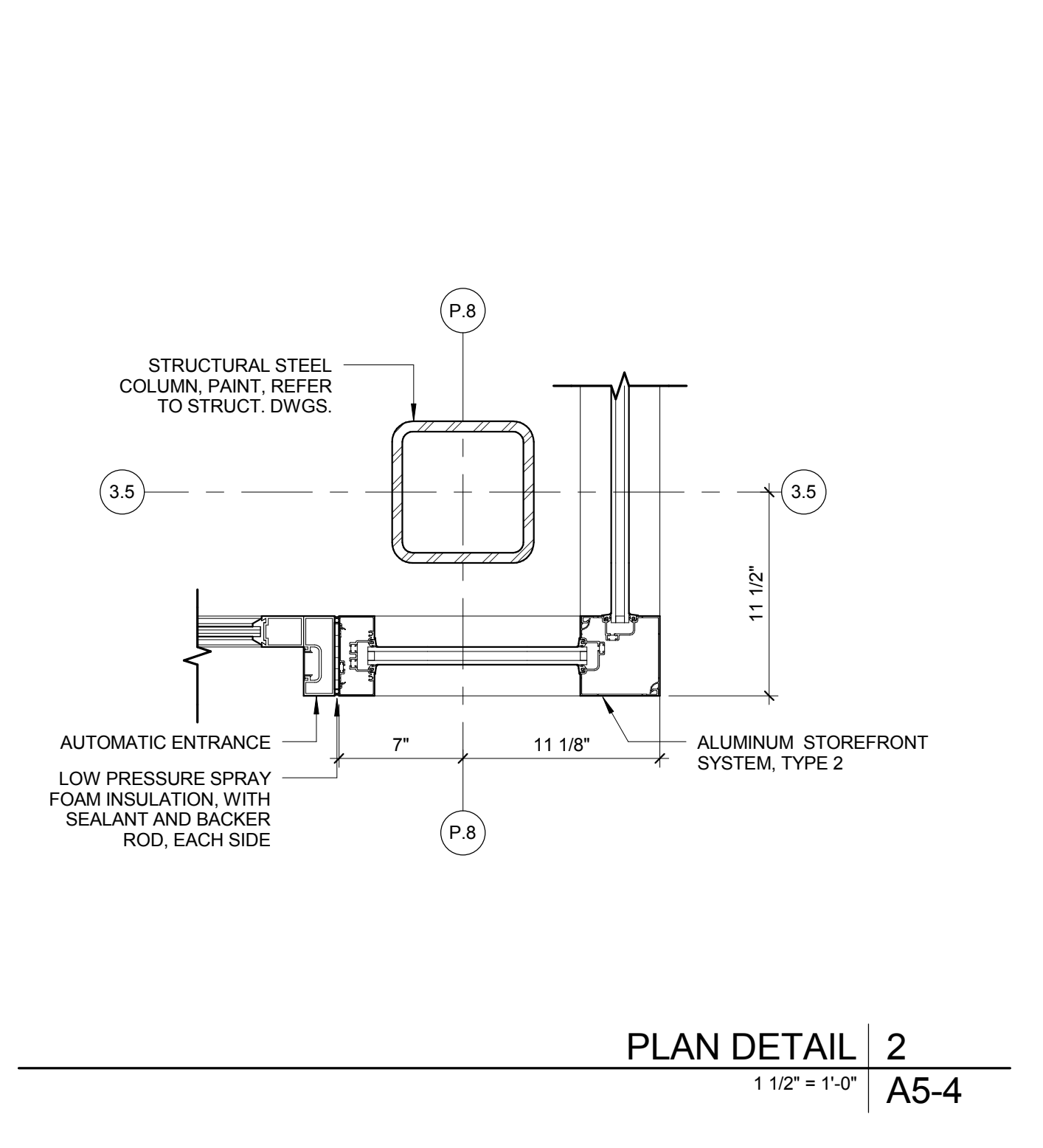
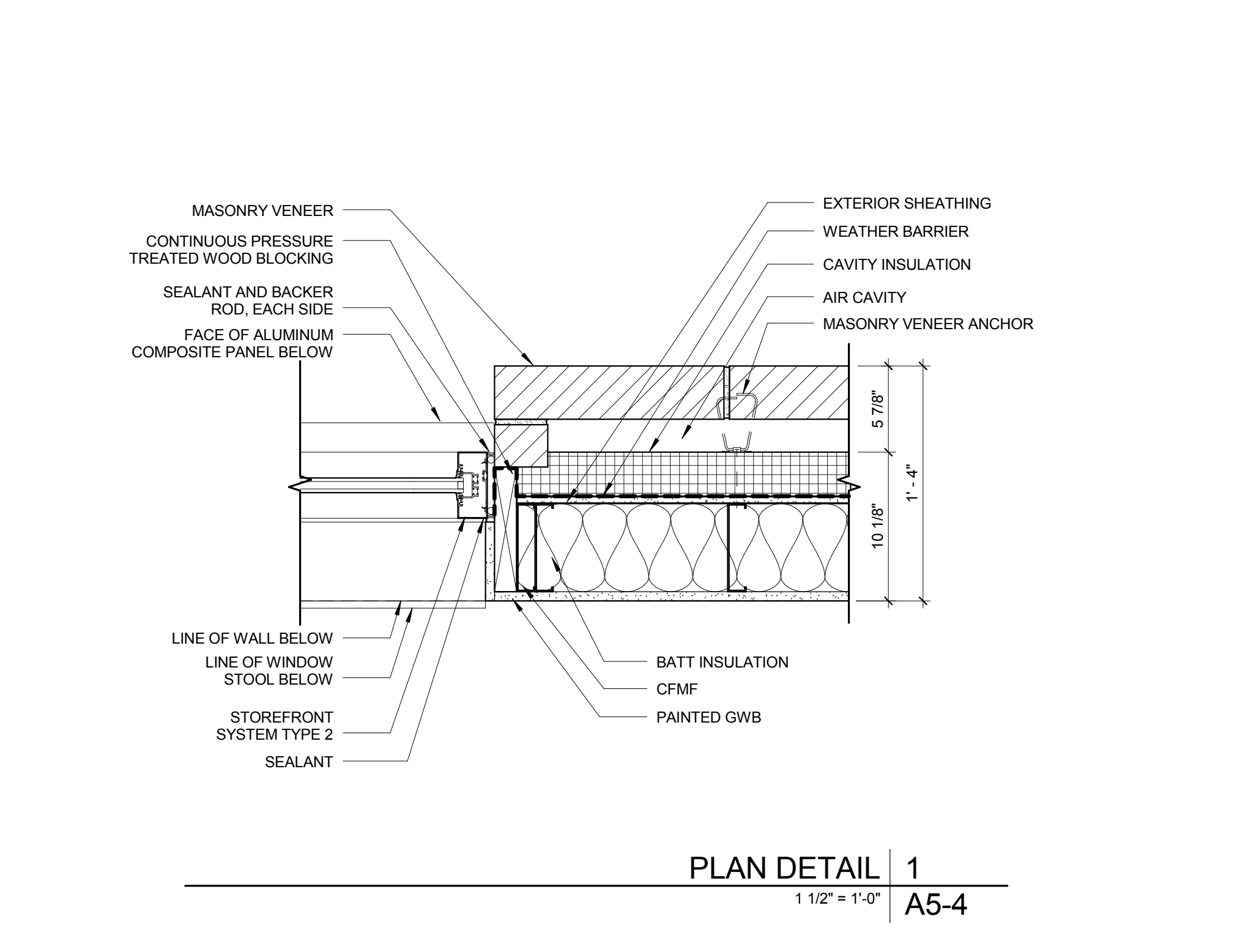
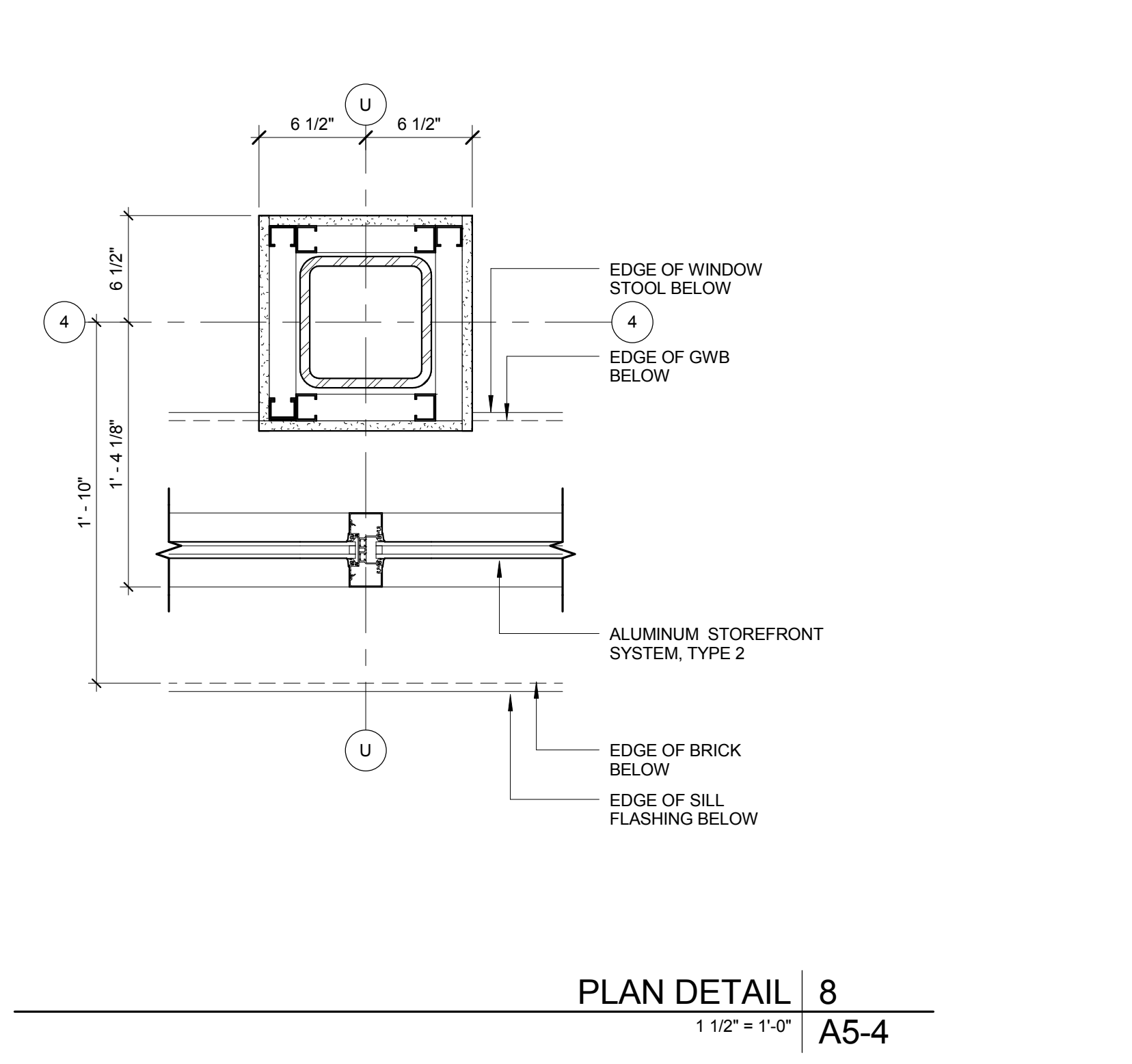
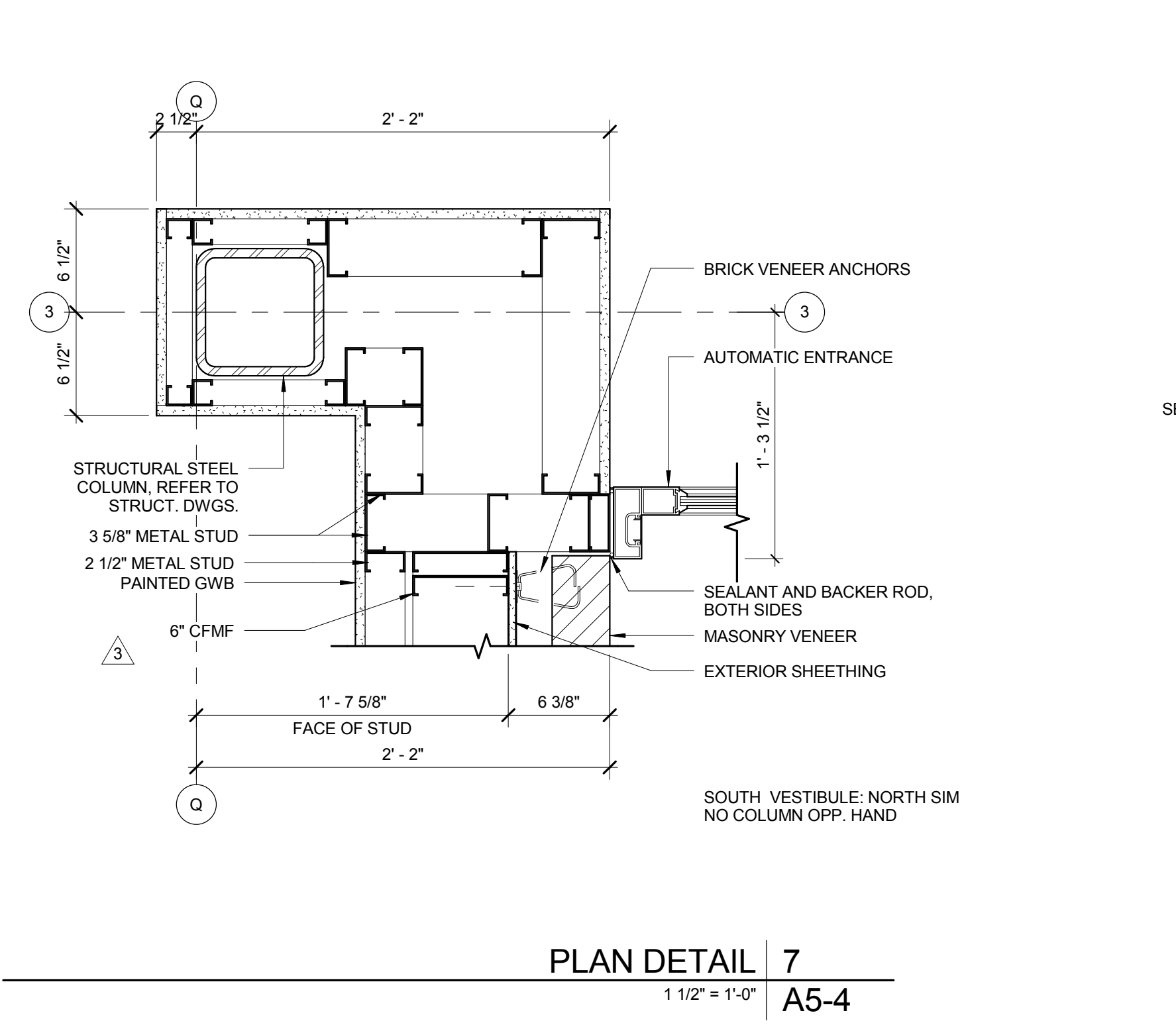
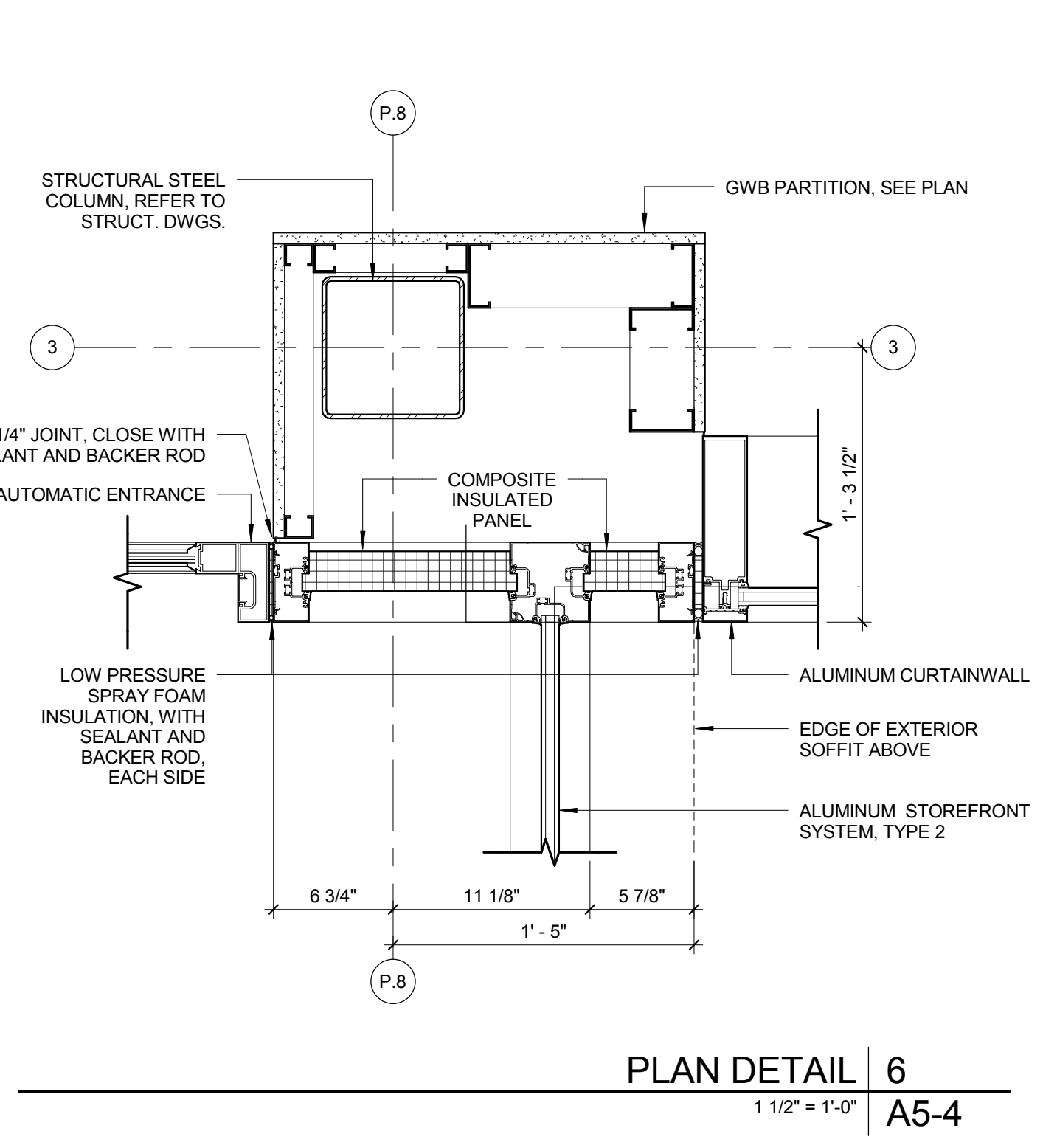
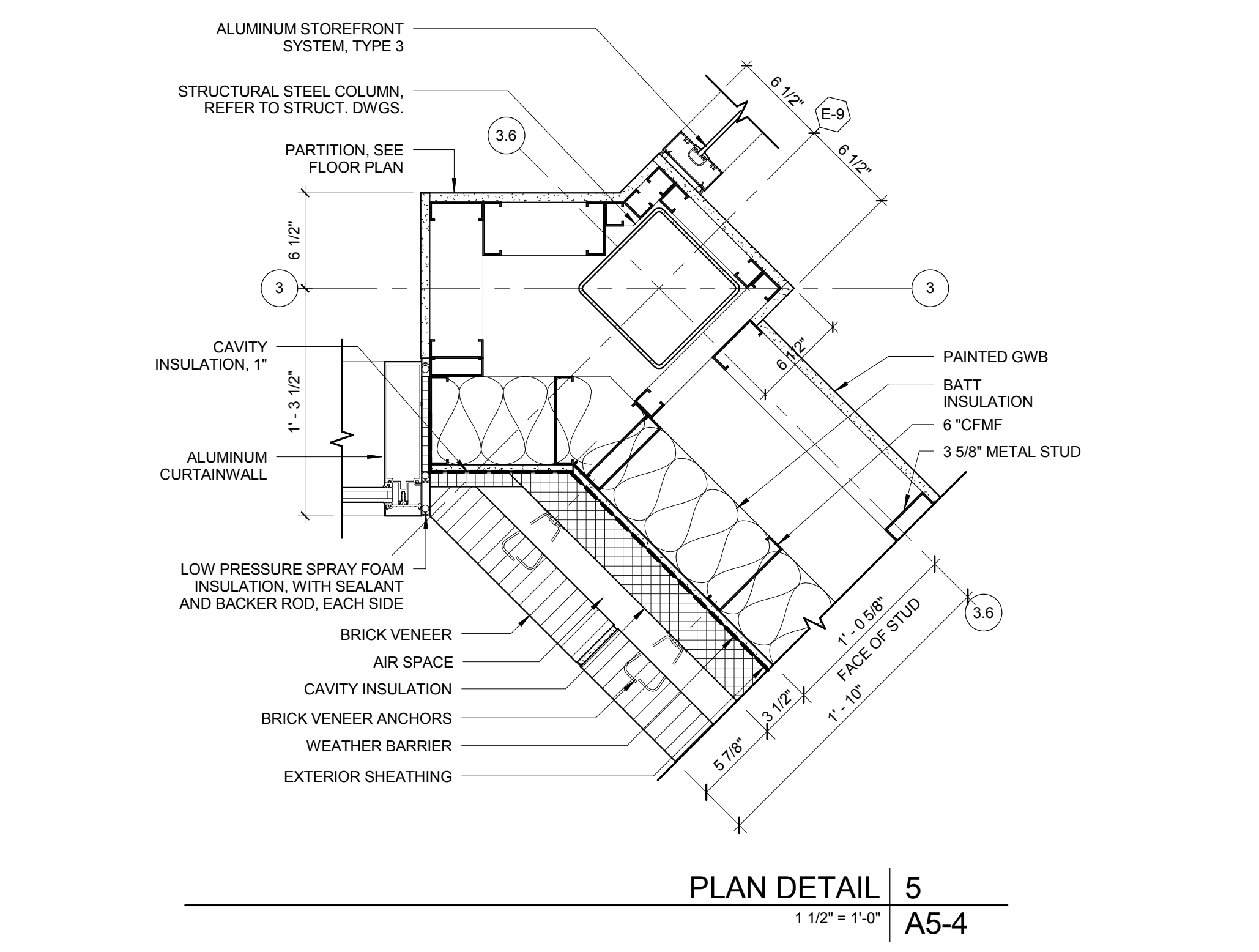
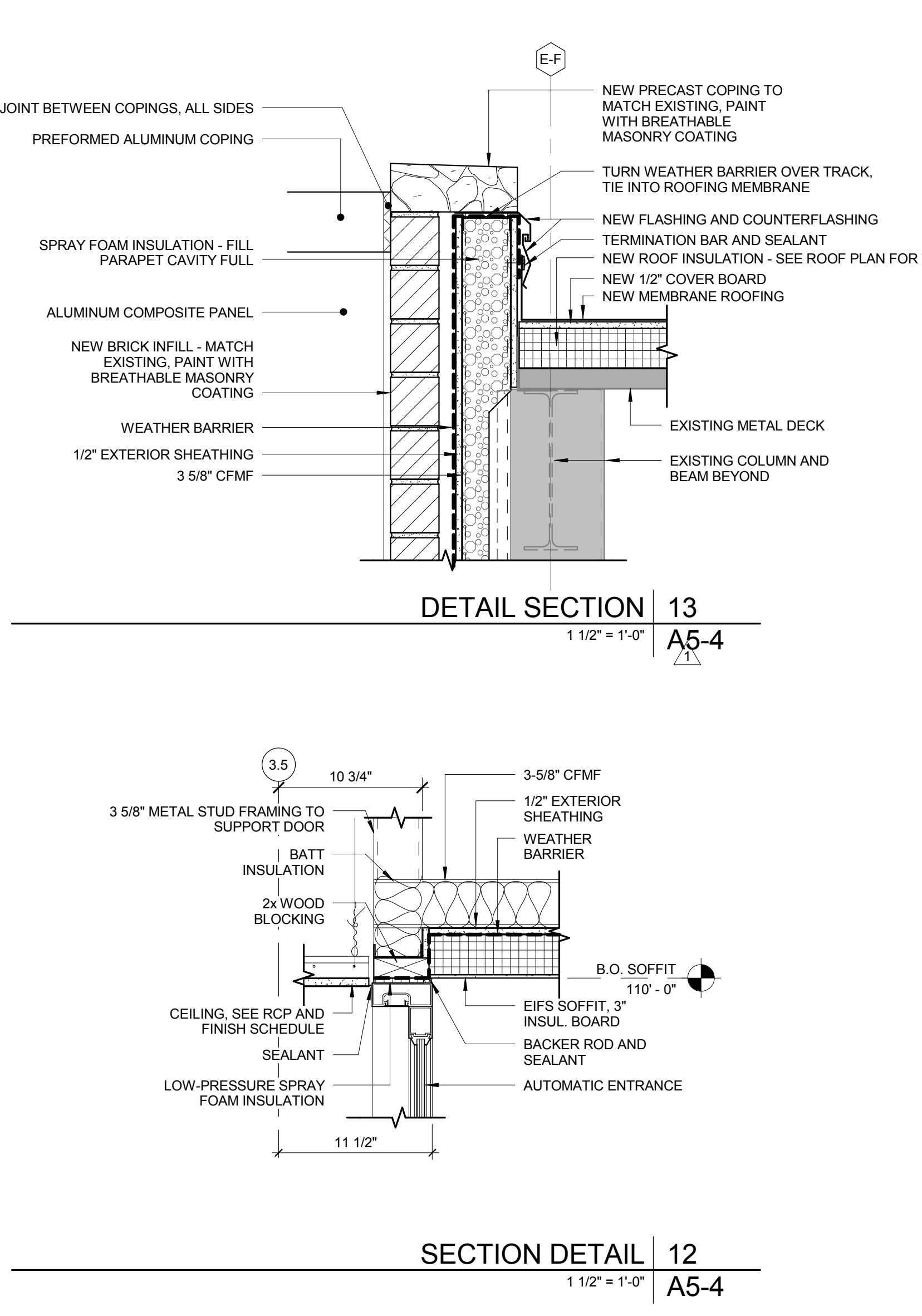
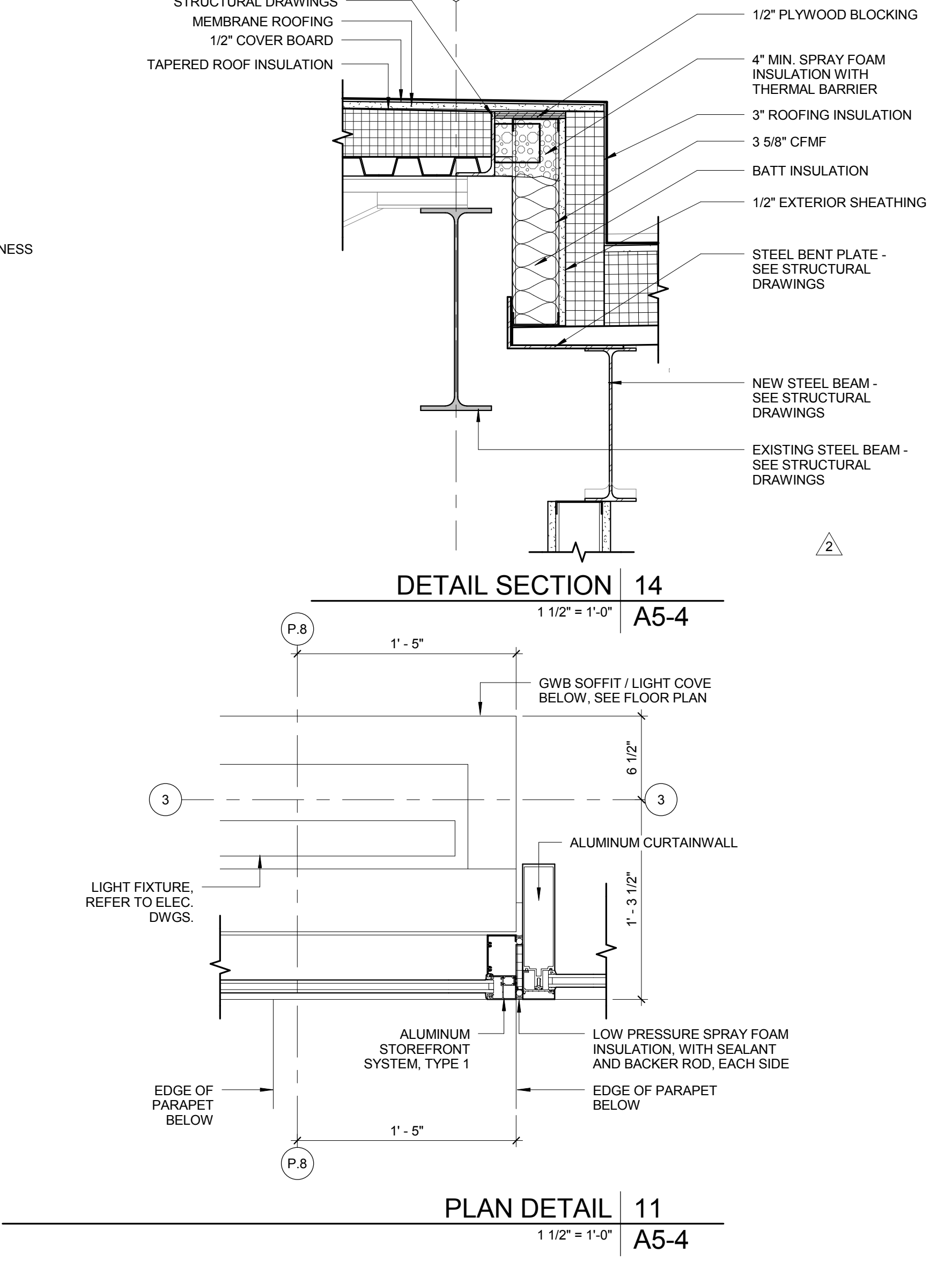
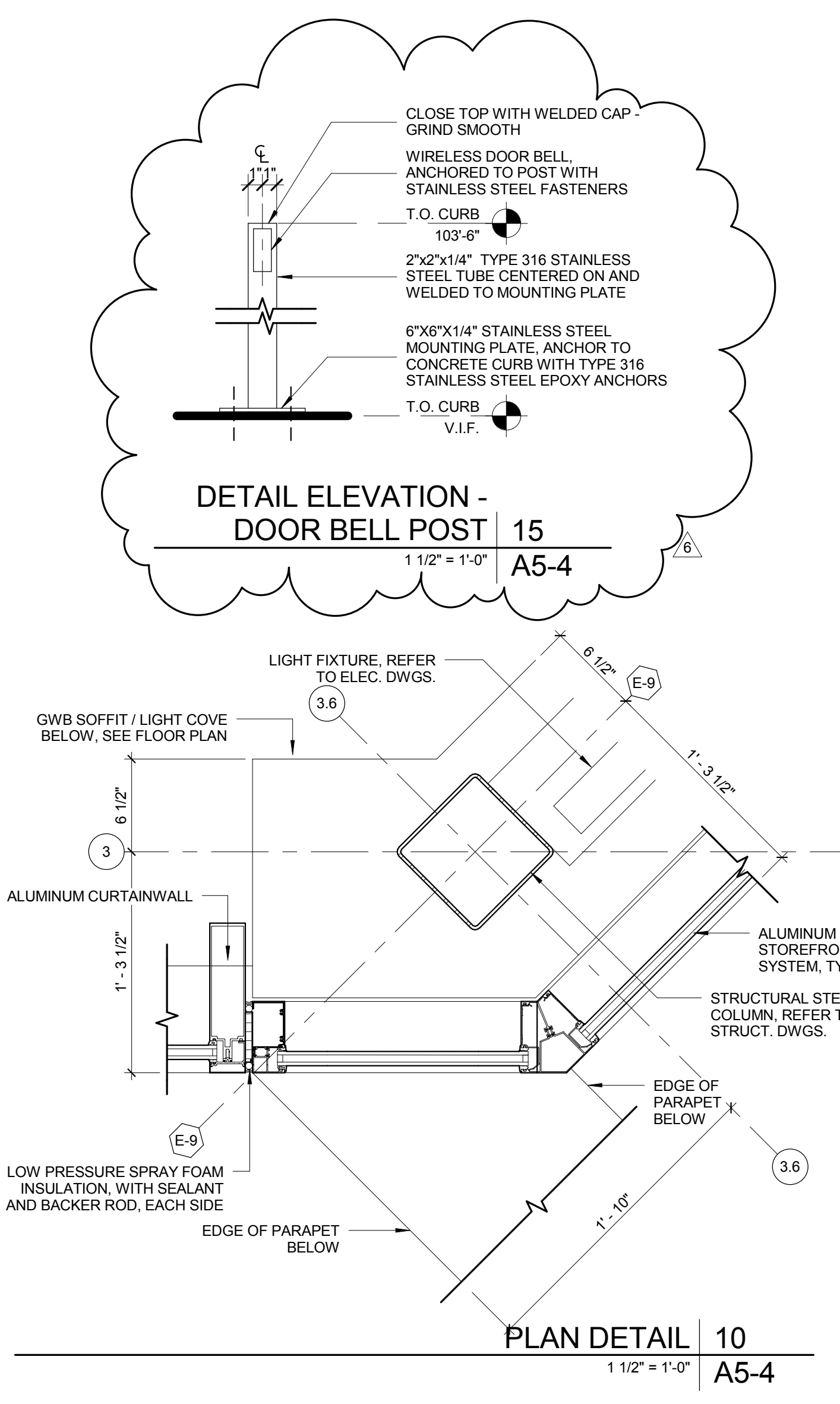
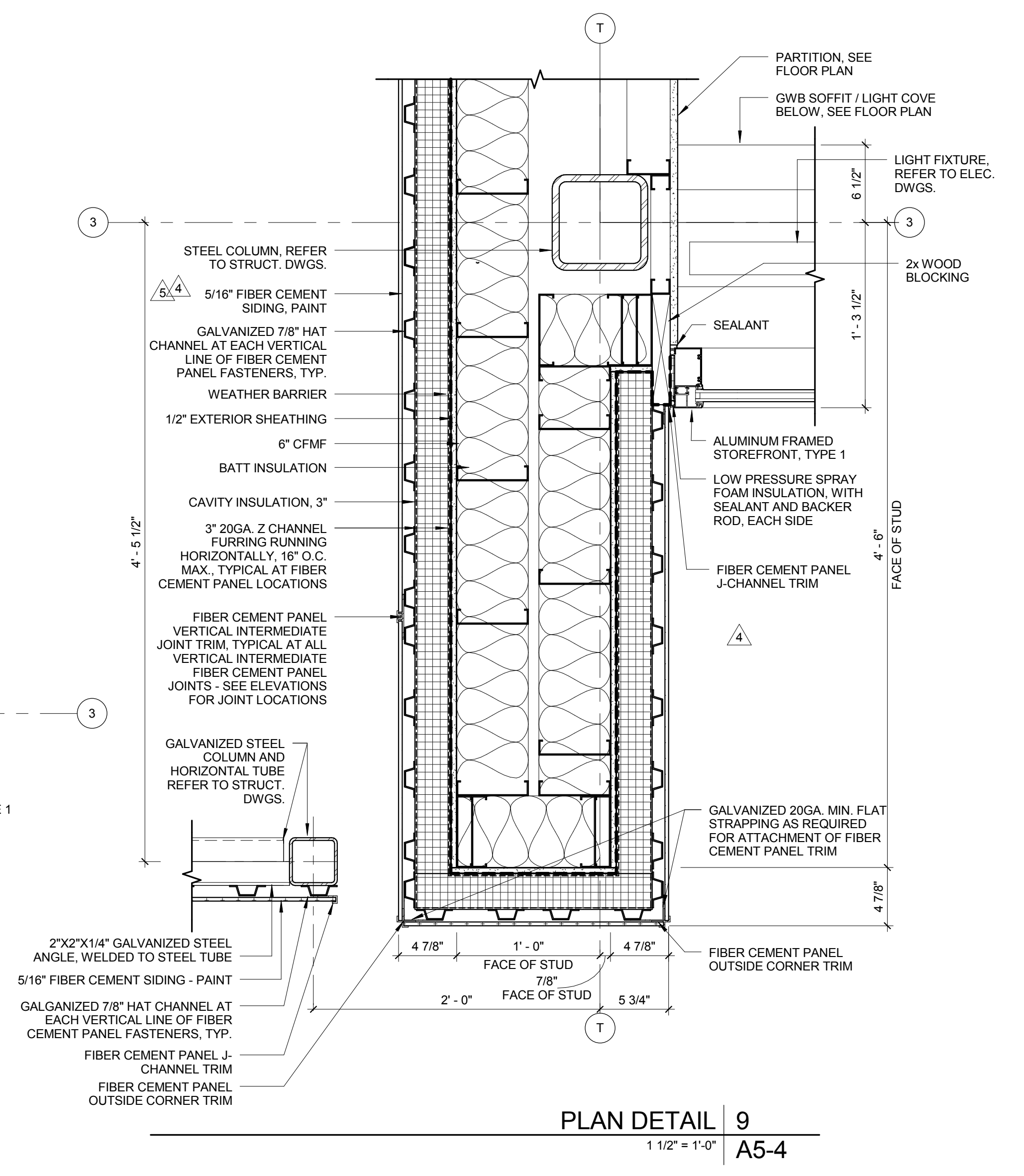
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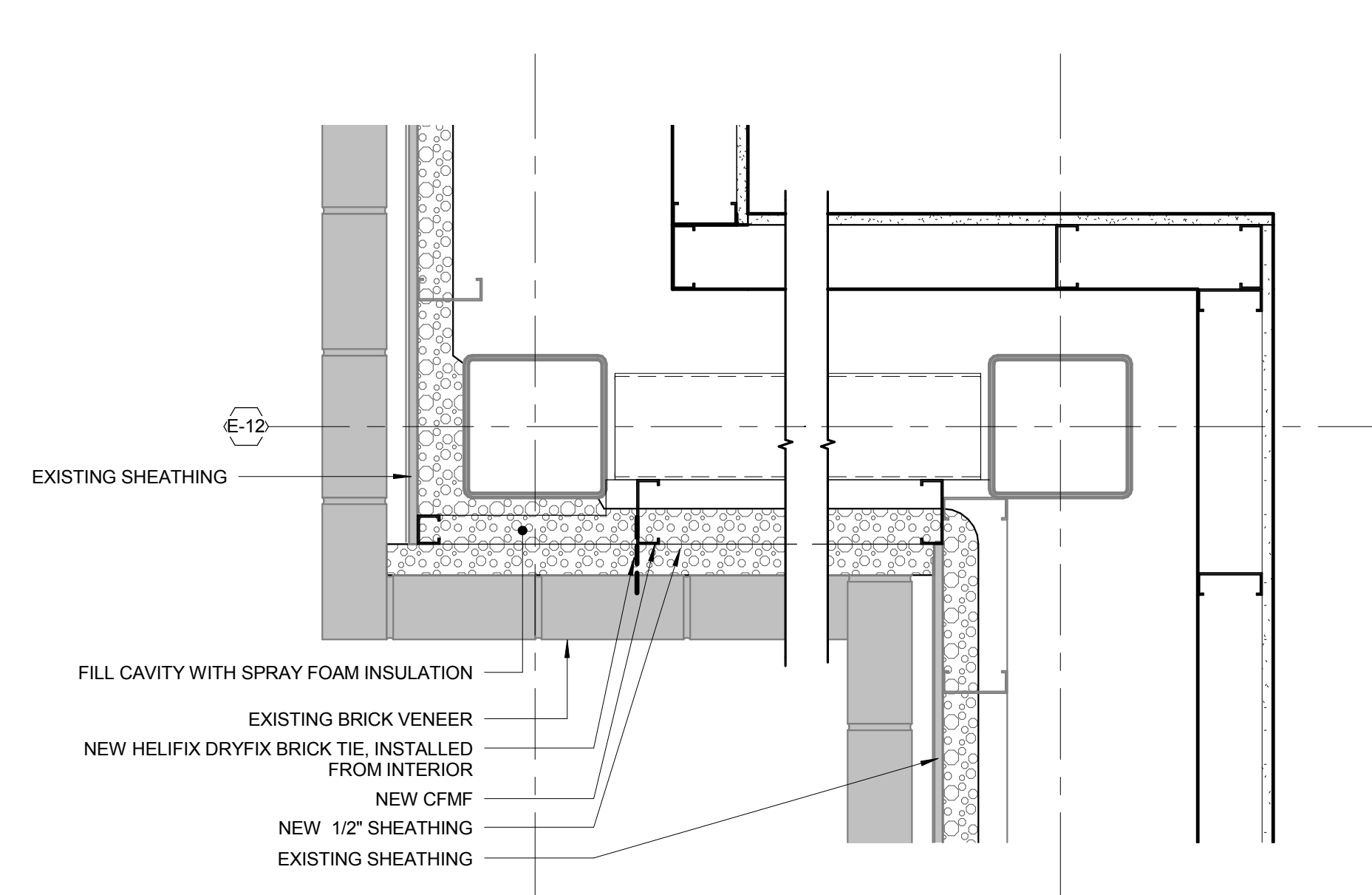
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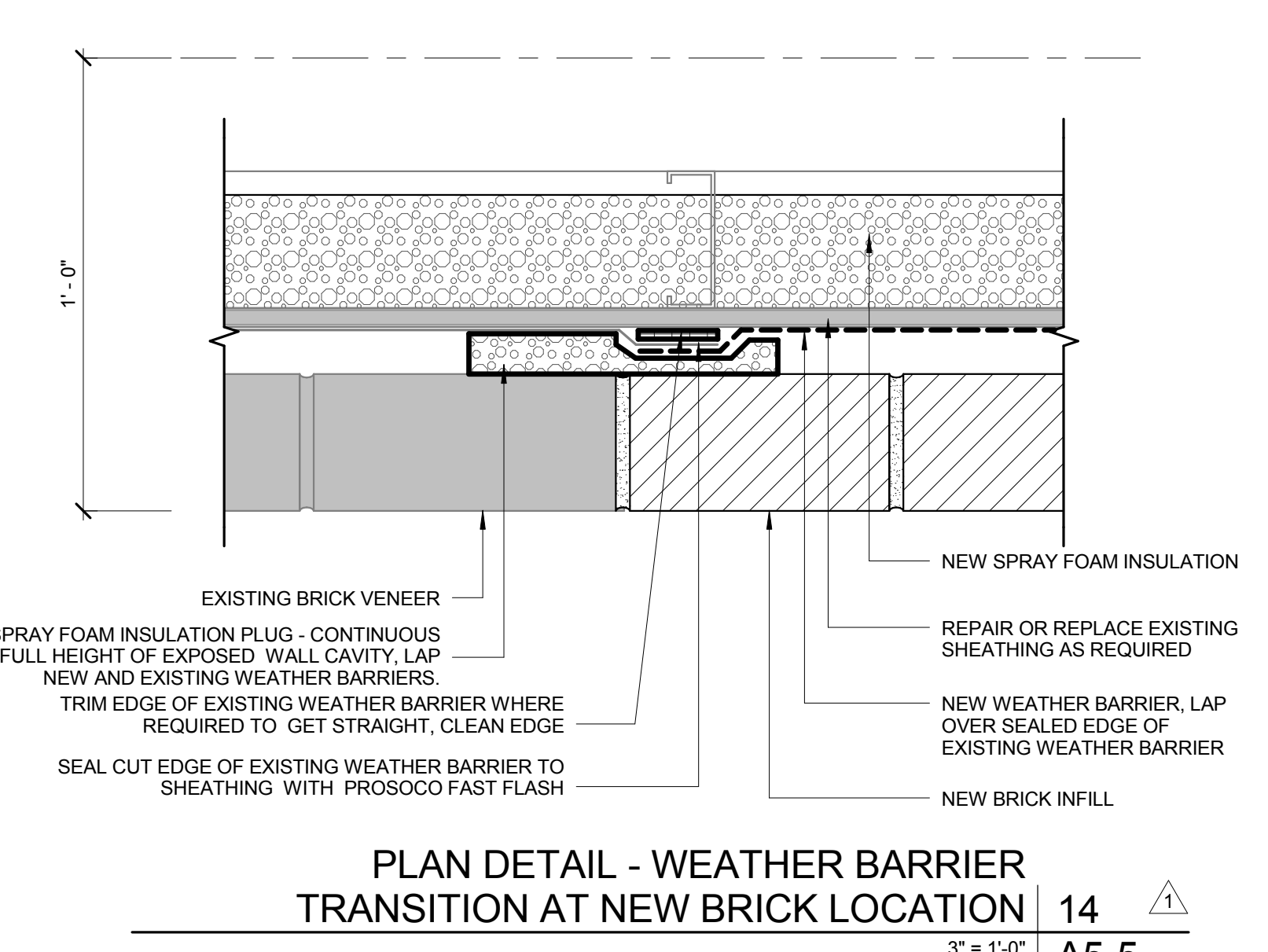
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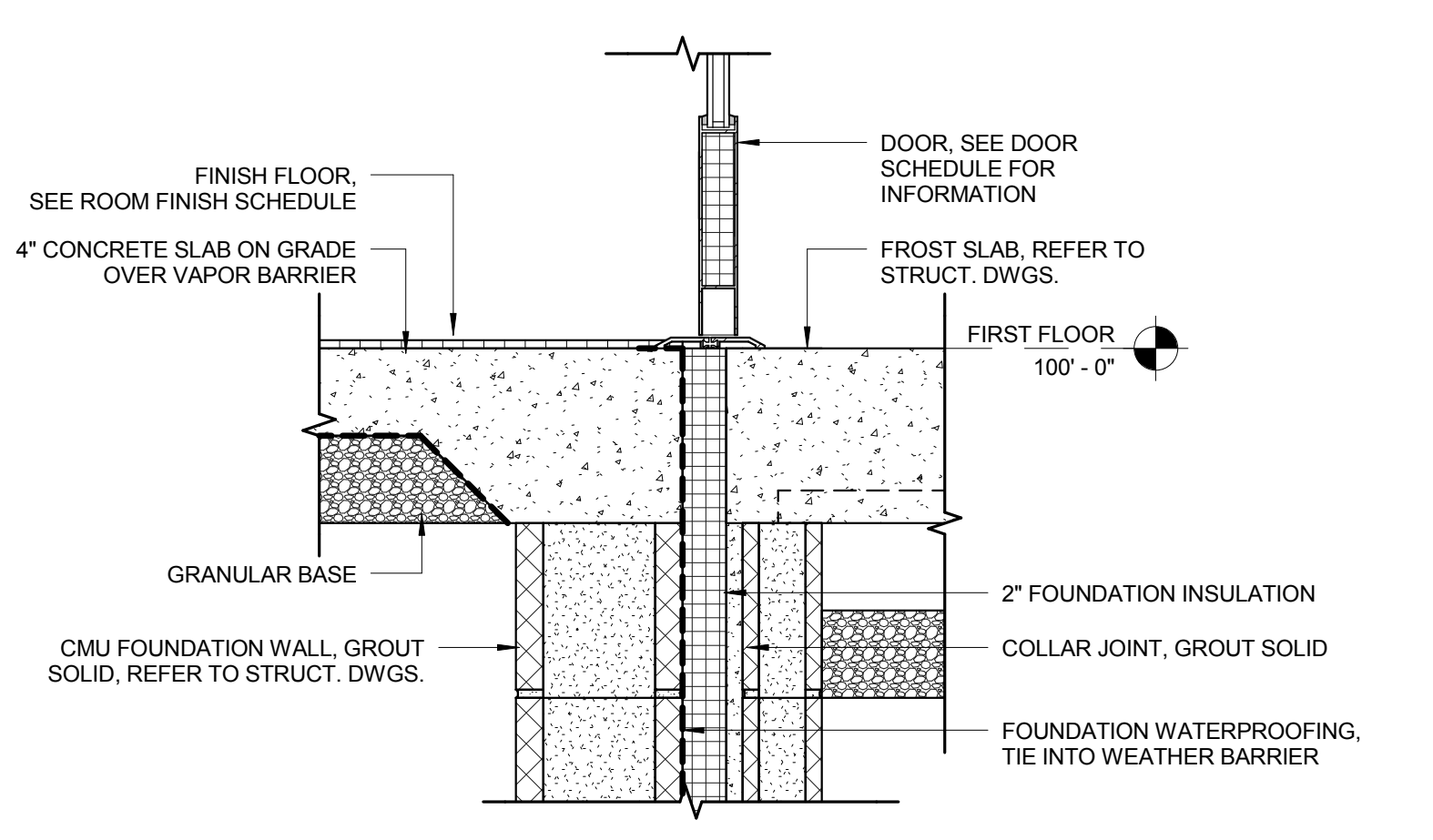
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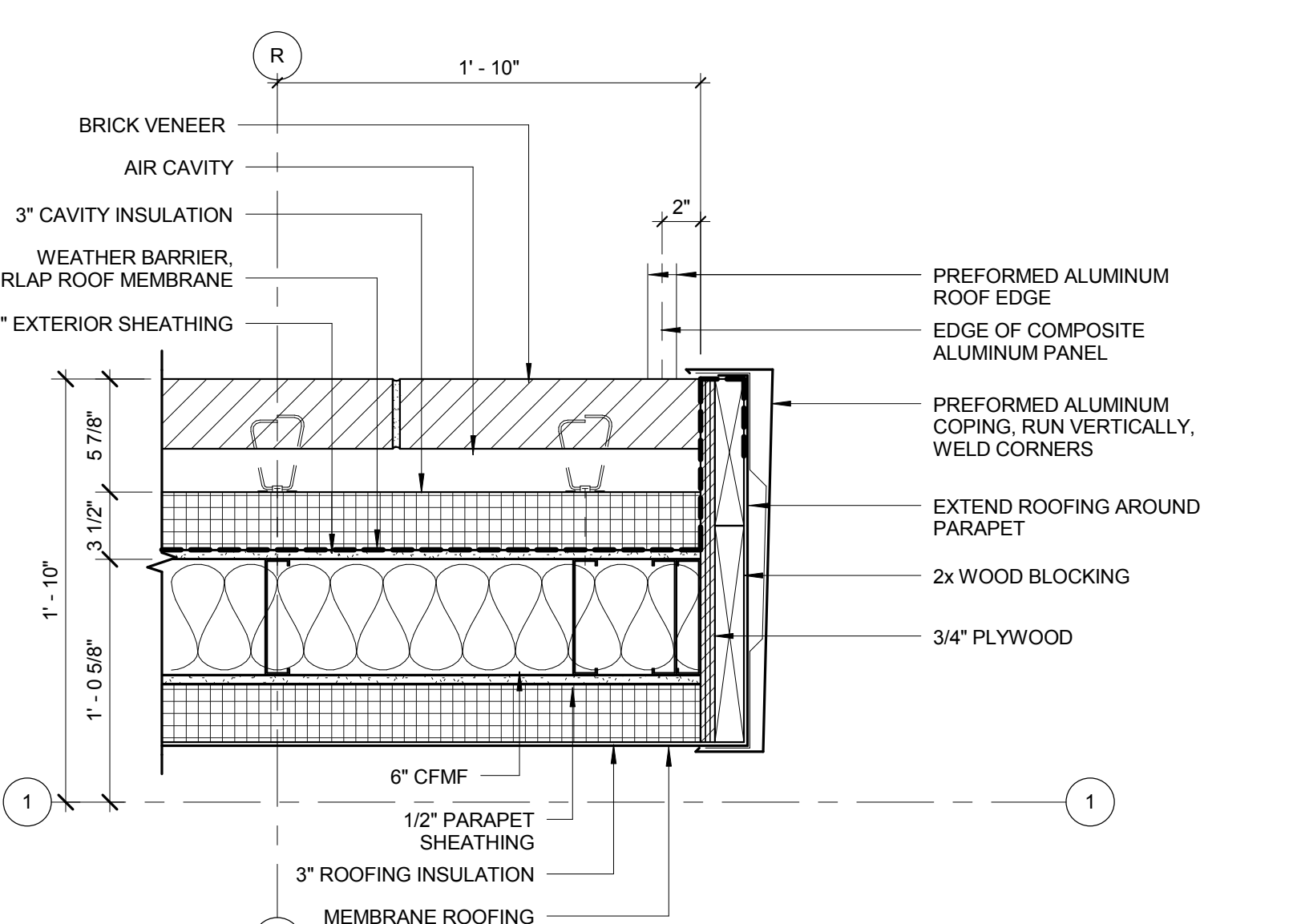
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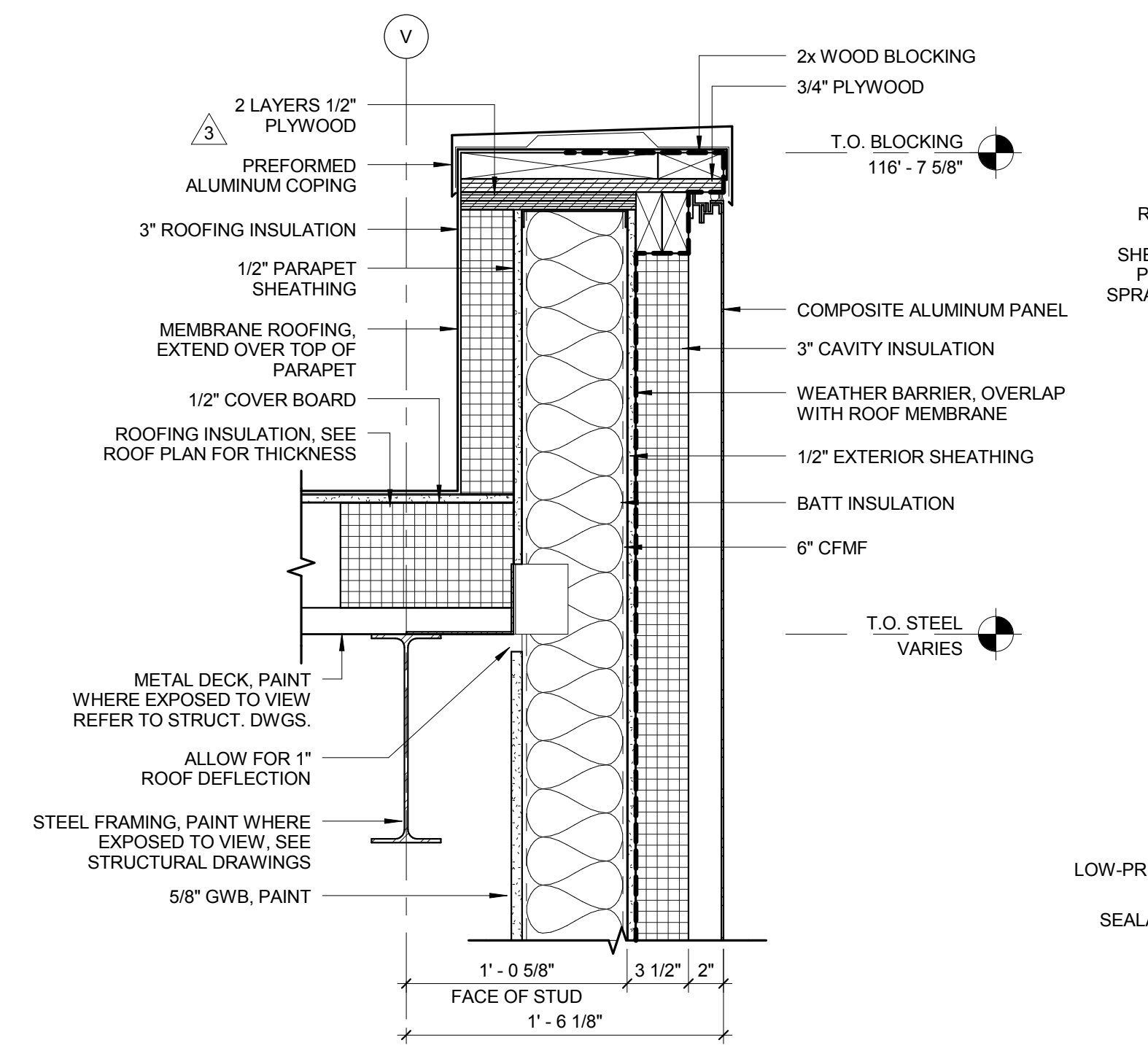
PLAN DETAIL - WEATHER BARRIER TRANSITION AT NEW BRICK LOCATION 14
3" = 1'-0"
A5-5



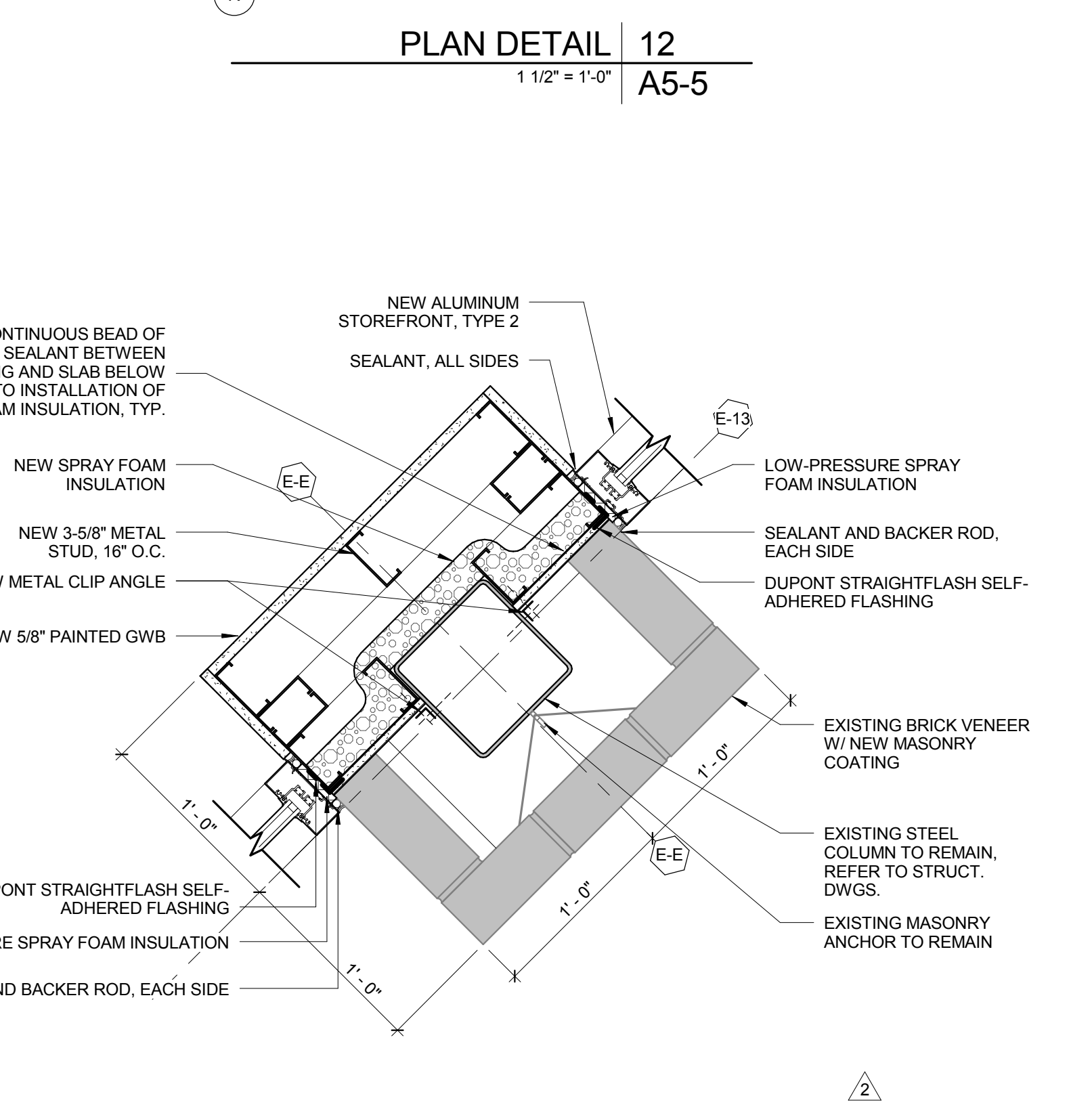
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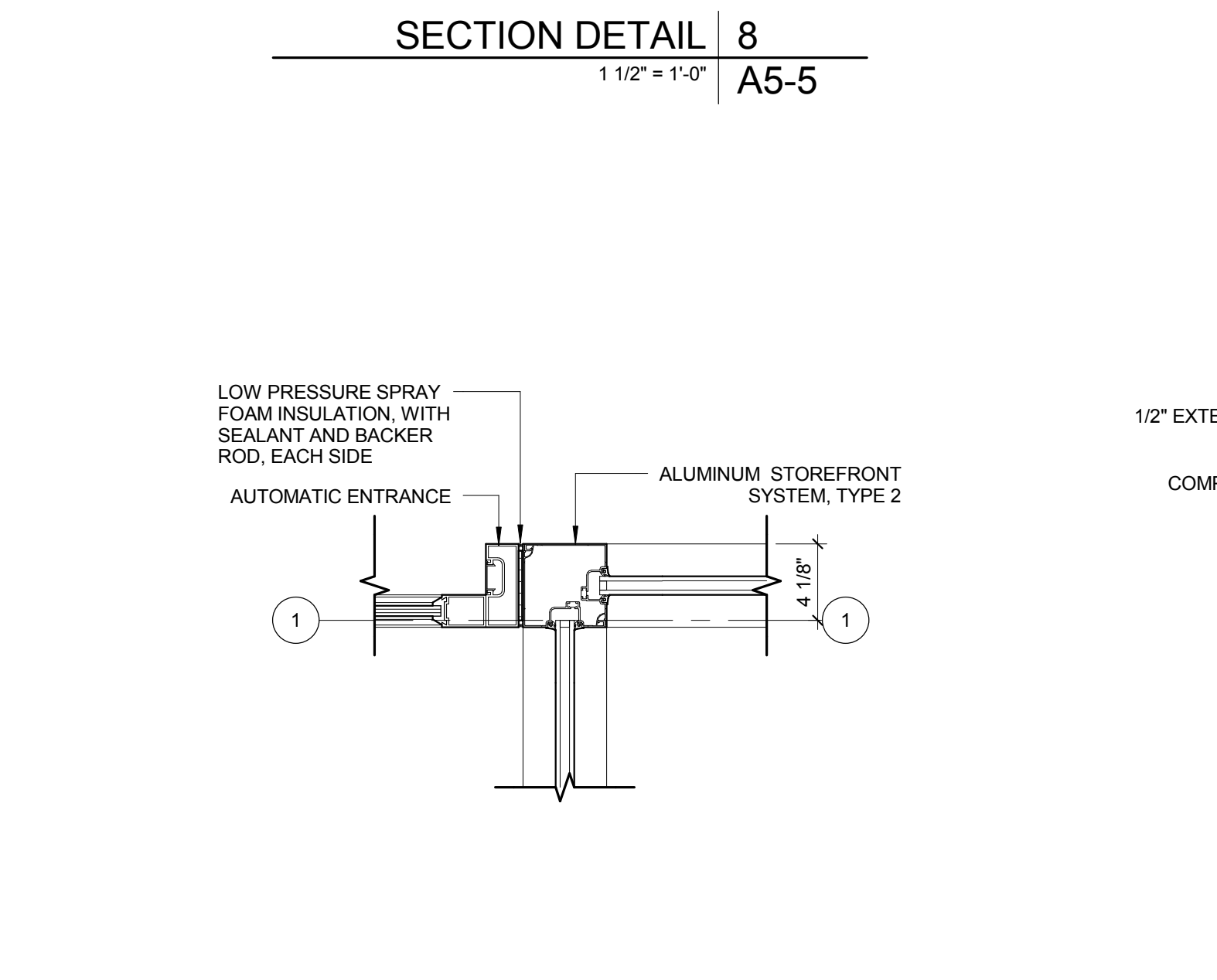
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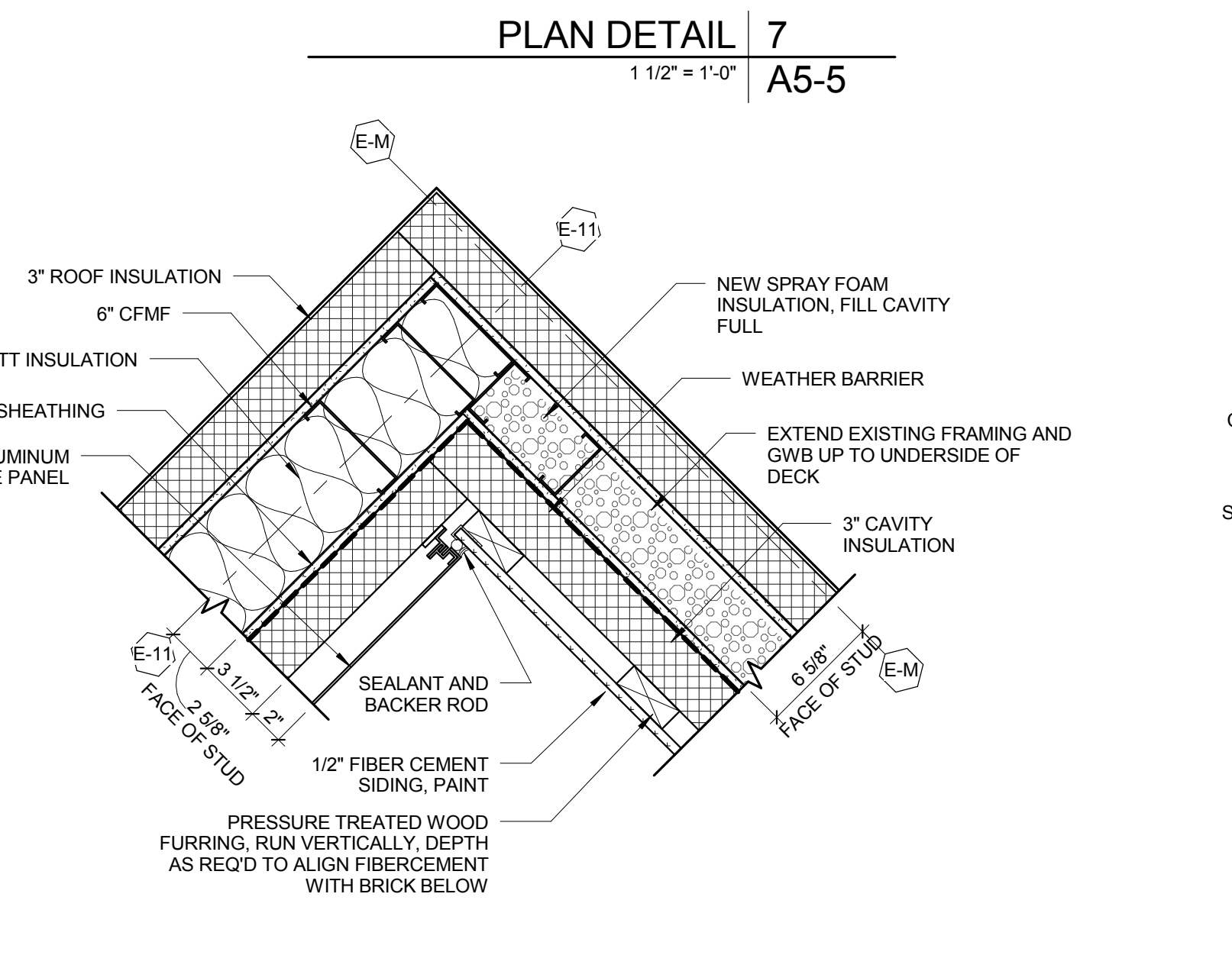
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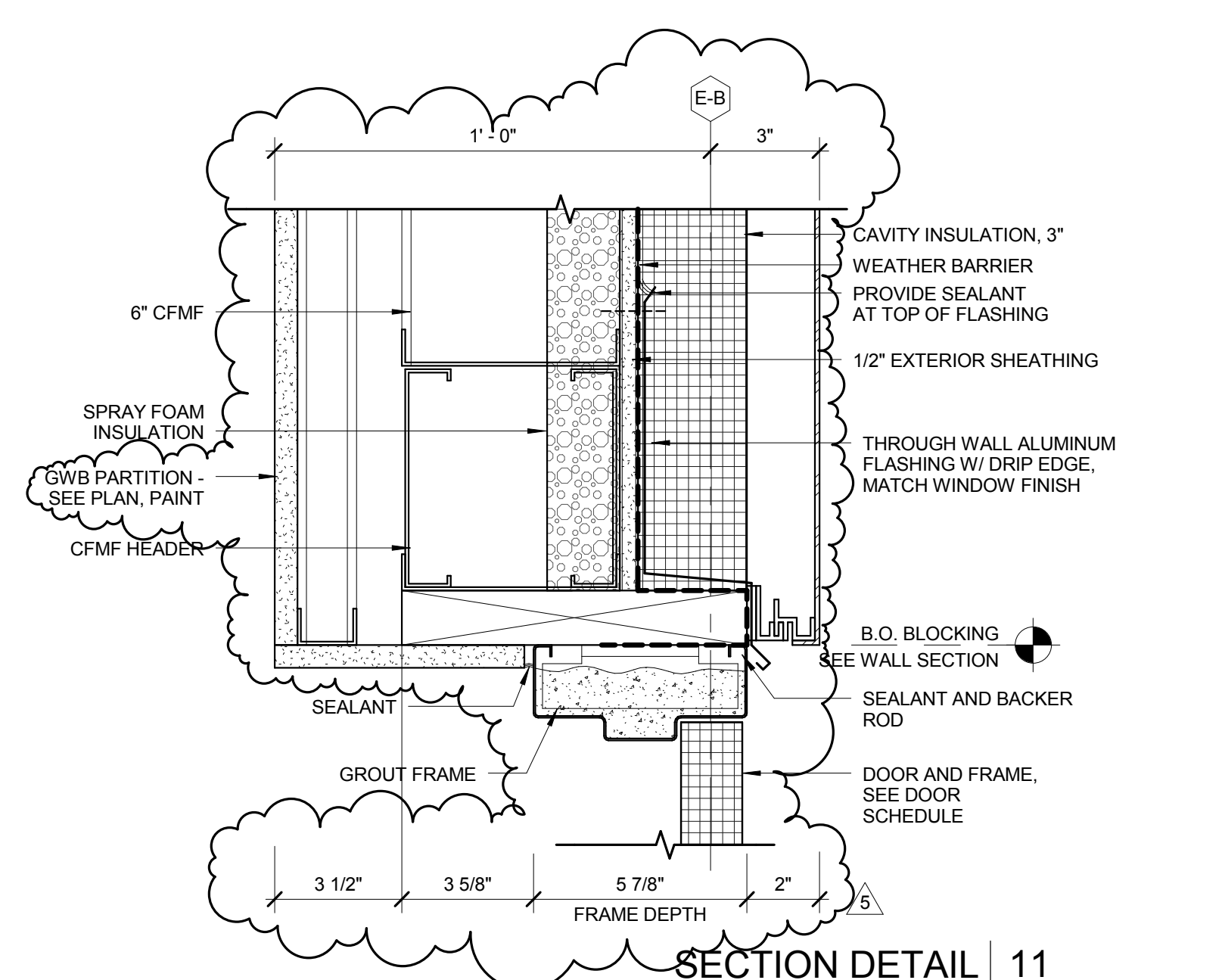
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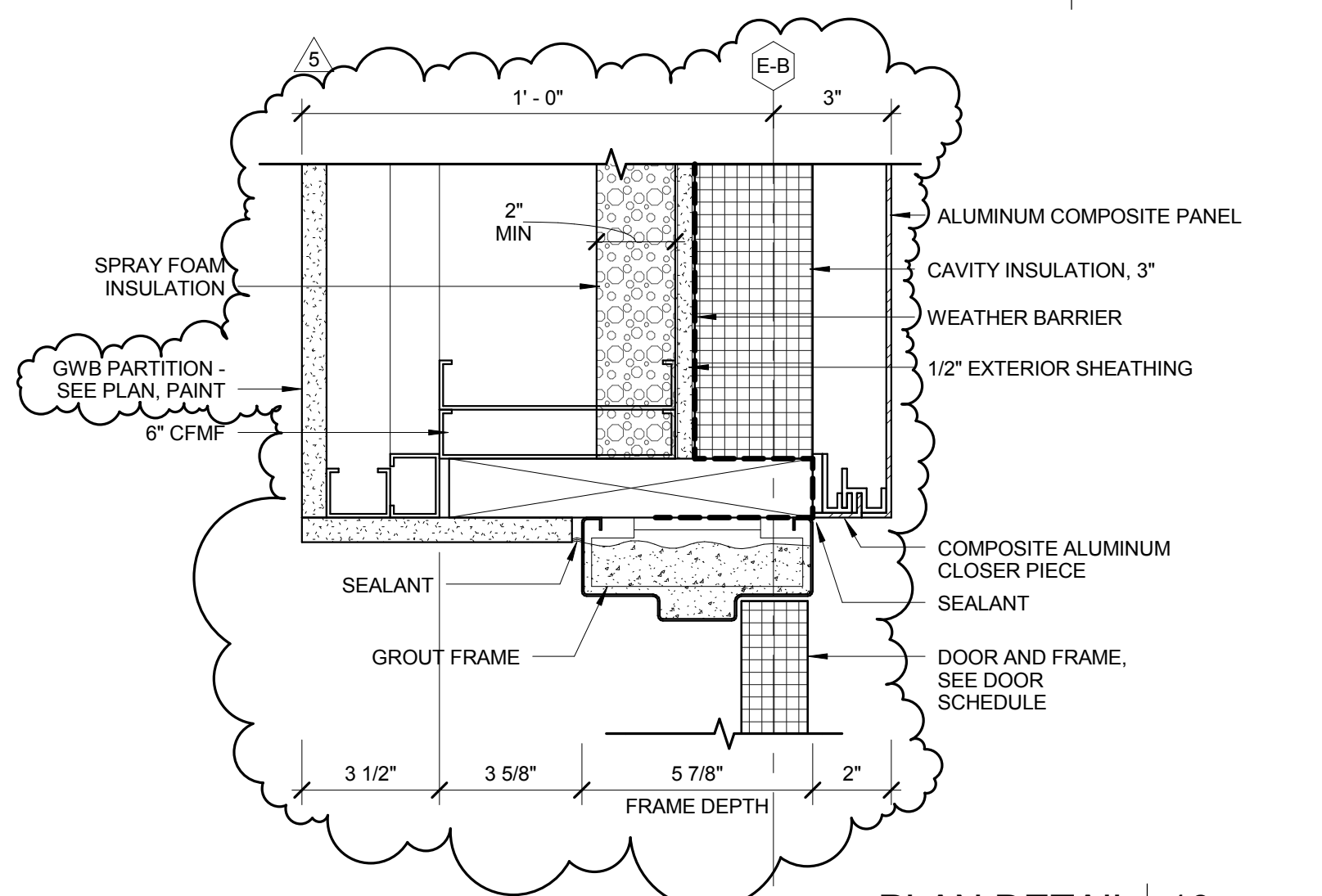
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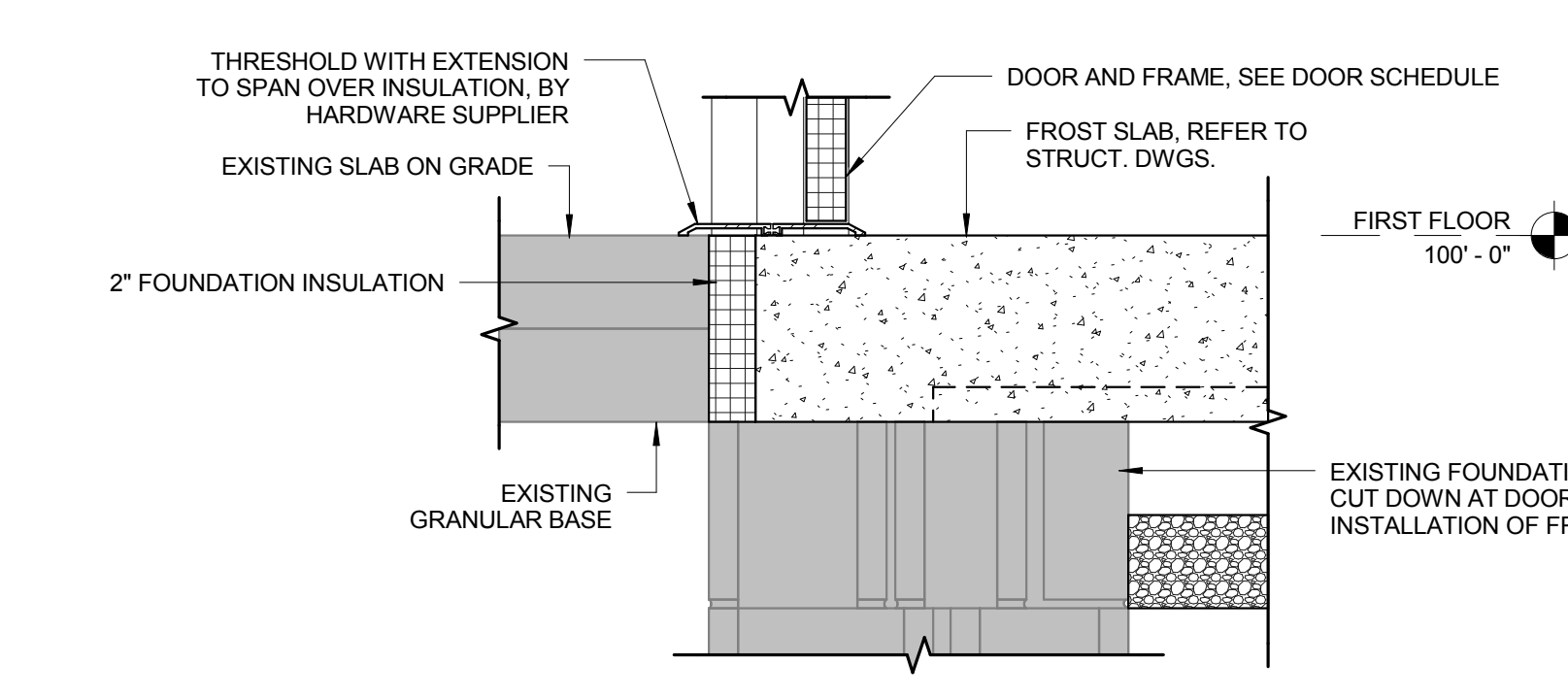
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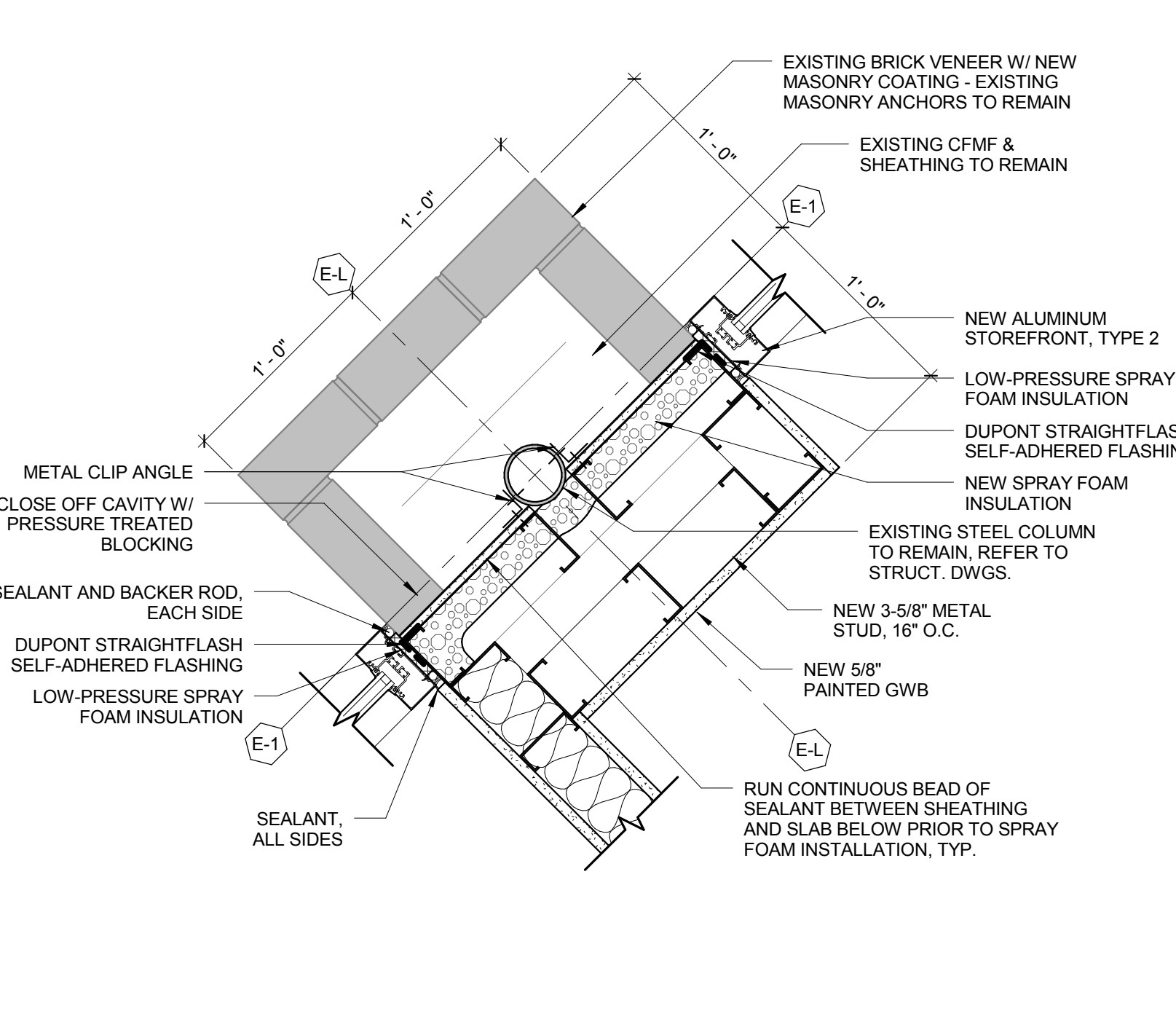
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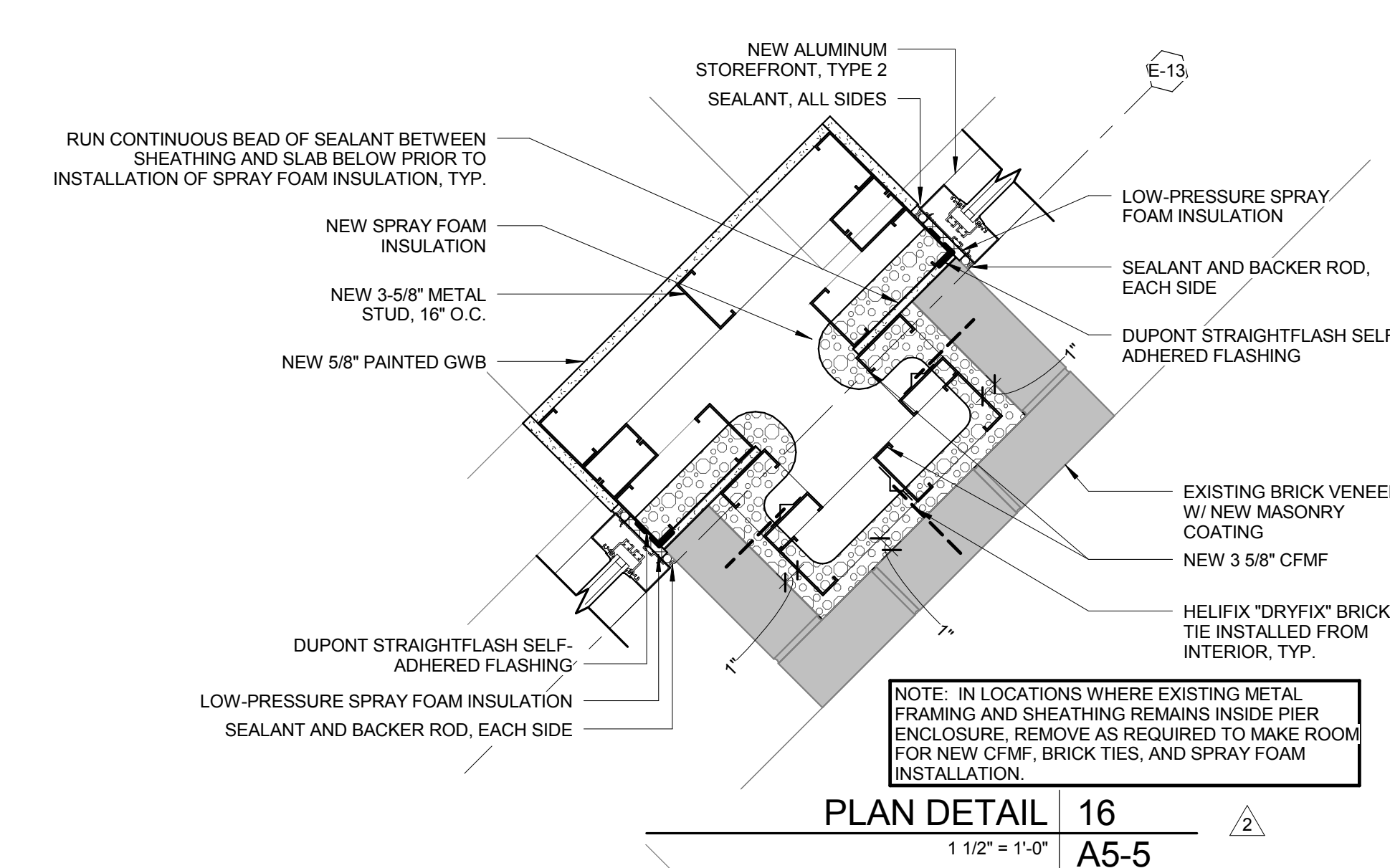
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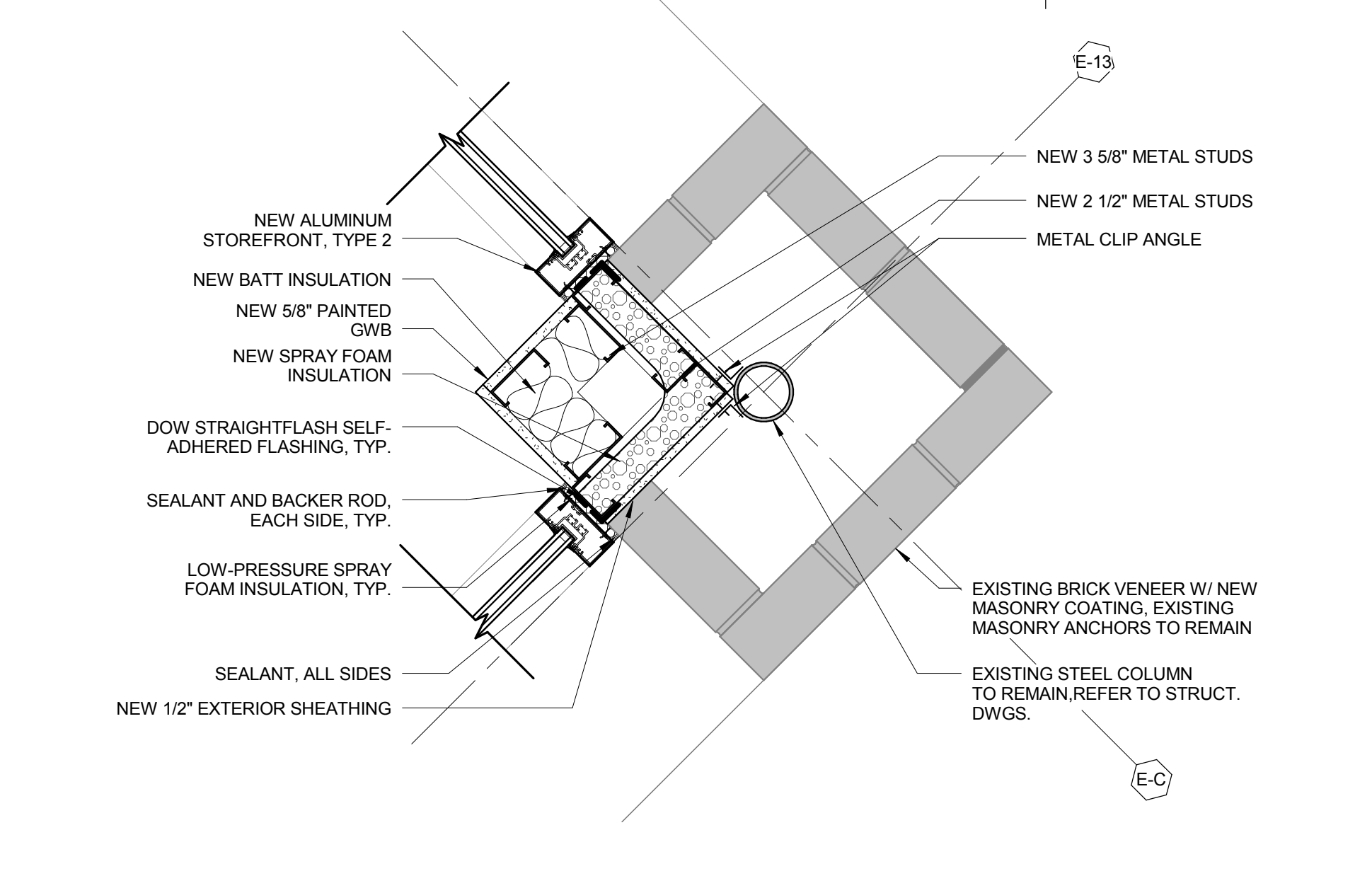
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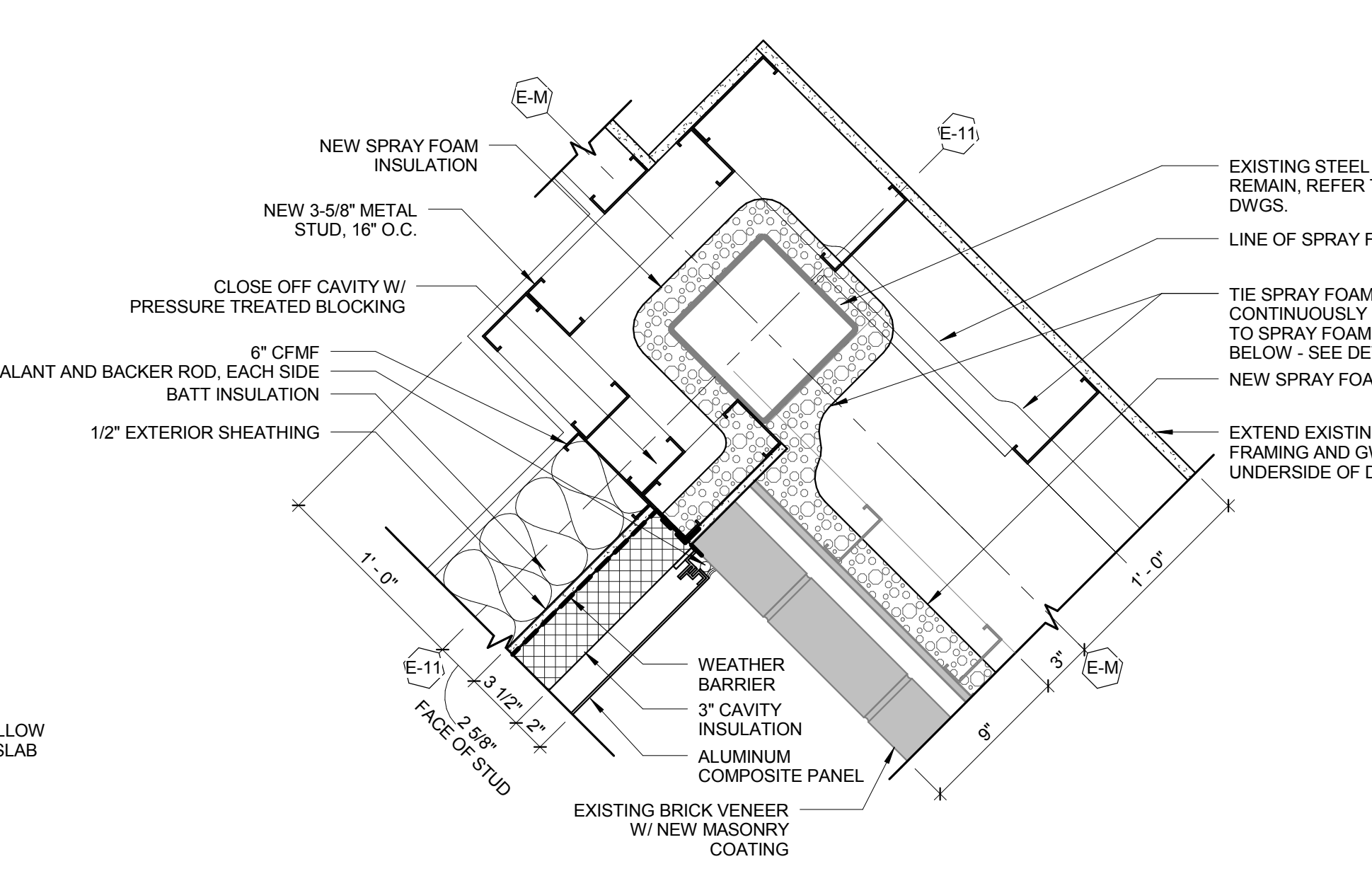
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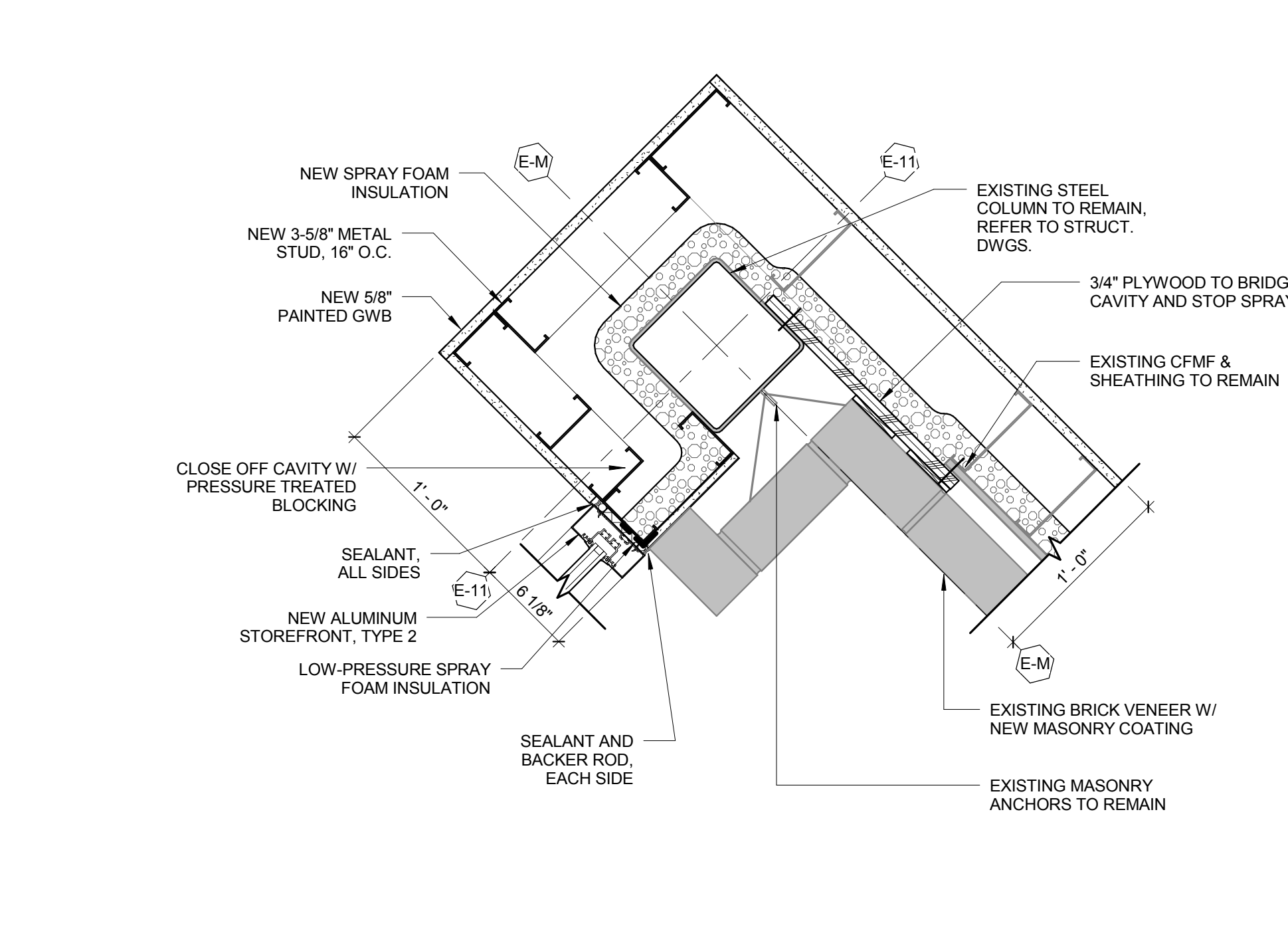
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A5-5



PLAN DETAIL 9
1 1/2" = 1'-0"
A5-5



PLAN DETAIL 5
1 1/2" = 1'-0"
A5-5



PLAN DETAIL 1
1 1/2" = 1'-0"
A5-5

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Michael Bongiorno
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LANDSCAPE ARCHITECT
MKSK
462 South Ludlow Alley Columbus, OH 43215 phone 614 621 2709 fax 614 621 3034

CIVIL AND MEP ENGINEER
Kongb
1650 Watermark Dr. Columbus, OH 43215 phone 614 487 1650 fax 614 487 8881

STRUCTURAL ENGINEER
SMH
1166 Dublin Rd., Ste 200 Columbus, Ohio 43215 phone 614 481 9800 fax 614 481 9353

#	DATE	DESCRIPTION
1	01 Dec 2015	Bulletin 008
2	17 Dec 2015	Bulletin 013
3	08 Jan 2016	Bulletin 017
4	18 Jan 2016	Bulletin 014
5	05 Feb 2016	REVISED Bulletin 025

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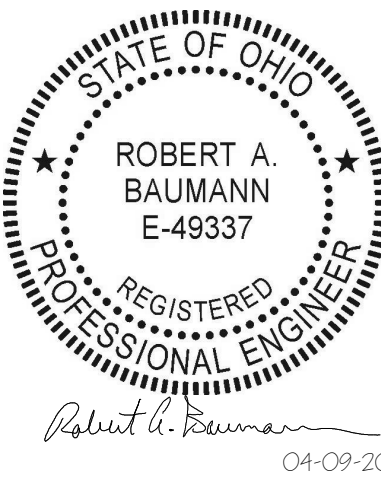
KMRK
462 South Ludlow Alley phone 614 621 2709
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CIVIL AND MEP ENGINEER

KORDA
1650 Watermark Dr. phone 614 487 1650
Columbus, OH 43215 fax 614 487 8861

STRUCTURAL ENGINEER

SMRH
1166 Dublin Rd., Ste 200 phone 614 481 9800
Columbus, Ohio 43215-1038 fax 614 481 9303



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GENERAL NOTES

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053100 - STEEL DECKING

- SPECIFICATIONS AND STANDARDS:
 - DESIGN FABRICATION AND ERECTION OF STEEL DECK SHALL BE GOVERNED BY THE LATEST EDITION OF THE AMERICAN IRON AND STEEL INSTITUTE, SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS
 - PROPERTIES OF THE STRUCTURAL STEEL DECK SHALL BE COMPUTED IN ACCORDANCE WITH THE REFERENCE STANDARD. THE PROPERTIES SHALL BE PUBLISHED IN THE MANUFACTURERS CATALOG.
 - AWS STANDARD WELDING SYMBOLS.
 - AWS D1.3 SPECIFICATIONS FOR WELDING SHEET STEEL IN STRUCTURES.
 - WELDING SHALL BE PERFORMED ONLY BY OPERATORS QUALIFIED, BY THE AWS STANDARD QUALIFICATION PROCEDURE, TO PERFORM THE PARTICULAR TYPE OF WORK REQUIRED.
- MATERIALS:
 - GALVANIZED STEEL DECK: ASTM A653 STRUCTURAL QUALITY GRADE 33 WITH COATING DESIGNATION G60.
 - WELDING ELECTRODES: AWS A5.1, A5.5 OR A5.18 SERIES E60.
- ERECTION AND CONNECTIONS:
 - MINIMUM BEARING: 2 INCHES UNLESS OTHERWISE SHOWN.
 - ANCHOR STEEL DECK TO STEEL SUPPORTING MEMBERS WITH 5/8" DIAMETER PUDDLE WELDS AT A MAXIMUM AVERAGE SPACING OF 12 INCHES UNLESS SHOWN OTHERWISE.
 - FOR DECK SPANS GREATER THAN FIVE FEET, SIDE LAP FASTENERS SHALL BE SPACED AT INTERVALS NOT EXCEEDING 36 INCHES, UNLESS NOTED, USING #10 SCREWS, 5/8" DIAMETER PUDDLE WELDS, OR 1" LONG FILLET WELDS.
- OPENINGS IN STEEL DECK:
 - OPENINGS CUT IN THE STEEL DECK SHALL BE REINFORCED OR SHALL BE SUPPORTED ON STEEL ANGLE FRAMES. COORDINATE SIZES AND LOCATIONS WITH THE MECHANICAL AND ARCHITECTURAL DRAWINGS.
 - OPENINGS IN STEEL DECK EQUAL TO OR LESS THAN 12"x12" SHALL BE REINFORCED WITH A 24"x24" - 16 GAGE PLATE SCREWED OR WELDED TO THE DECK RIBS ON ALL SIDES OF THE OPENING.
 - OPENINGS IN ROOF DECK GREATER THAN 12"x12" SHALL BE SUPPORTED ON STEEL ANGLE FRAMES.

054000 COLD-FORMED METAL FRAMING

- SPECIFICATIONS AND STANDARDS:
 - STRUCTURAL PROPERTIES OF COLD-FORMED METAL FRAMING SHALL BE COMPUTED IN ACCORDANCE WITH AISI SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS AND SHALL BE PUBLISHED IN THE MANUFACTURERS CATALOG.
 - WELDING SHALL BE PERFORMED ONLY BY QUALIFIED OPERATORS USING PROPER EQUIPMENT FOR THE PARTICULAR TYPE OF WORK REQUIRED.
 - AWS STANDARD WELDING SYMBOL.
 - AWS D1.3 SPECIFICATION FOR WELDING SHEET STEEL IN STRUCTURES.
- MATERIALS:
 - MEMBERS 54 MILS AND HEAVIER: ASTM A1003, GRADE 50, TYPE H.
 - MEMBERS 43 MILS AND LIGHTER: ASTM A1003, GRADE 33, TYPE H.
 - TRACK AND BRIDGING MATERIALS: ASTM A1003, GRADE 33, TYPE H.
 - FRAMING SHALL BE GALVANIZED PER ASTM A653, G60.
 - WELDING ELECTRODES: AWS A5.1, A5.5 OR A5.18 SERIES E60.
- CONNECTIONS:
 - CUT FRAMING COMPONENTS TO FIT SQUARELY AGAINST ABUTTING MEMBERS AND HOLD FIRMLY IN POSITION UNTIL PROPERLY FASTENED.
 - PANELS SHALL BE SQUARE AND BRACED AGAINST RACKING.
 - WIRE TYING OF STRUCTURAL FRAMING COMPONENTS IS NOT PERMITTED.
 - COMPONENTS SHALL BE FASTENED TOGETHER WITH A MINIMUM OF 2-#8 SCREWS OR AS SHOWN ON THE DRAWINGS.
- MISCELLANEOUS REQUIREMENTS:
 - ATTACH TRACK TO THE FLOOR AND OVERHEAD STRUCTURE AS NOTED.
 - SEAT STUDS SQUARELY TO THE FLOOR AND OVERHEAD TRACK AND CONNECT AS NOTED.
 - SPLICES IN STRUCTURAL FRAMING MEMBERS ARE NOT PERMITTED WITHOUT APPROVAL OF THE STRUCTURAL ENGINEER.
 - DO NOT ALLOW AXIAL LOADS TO STUDS UNTIL ALL BRIDGING, CONNECTIONS, AND ATTACHMENT OF COLLATERAL MATERIALS ARE COMPLETE.
 - ATTACH V-BAR BRIDGING TO BOTH FLANGES OF WALL STUDS. SPACE BRIDGING AT 4'-0" c/c OR IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS.

042000 - CONCRETE UNIT MASONRY

- COMPRESSIVE STRENGTH OF MASONRY (f'm) 1500 PSI, DETERMINED BY UNIT STRENGTH OR PRISM METHOD.
- MASONRY MATERIALS:
 - HOLLOW AND SOLID LOAD BEARING CONCRETE MASONRY UNITS - ASTM C90 - NORMAL WEIGHT.
- MORTAR:
 - CONCRETE UNIT MASONRY WALLS - ASTM C270 TYPE S.
- COARSE MASONRY GROUT: SEE REINFORCED CONCRETE GENERAL NOTES.
- MASONRY REINFORCEMENT:
 - HORIZONTAL JOINT REINFORCEMENT: 9 GA DEFORMED WIRE, LADDER TYPE REINFORCEMENT.
 - IN EVERY SECOND BLOCK COURSE, FULL HEIGHT, AND WHERE SHOWN ON DRAWINGS.
 - IN FIRST BED JOINT ABOVE AND BELOW OPENINGS EXTENDING 24" BEYOND OPENING.
 - LAP REINFORCEMENT A FULL WIDTH AT CORNERS AND INTERSECTIONS.
- POST-INSTALLED SLEEVE ANCHORS:

ONE PIECE WRAP AROUND EXPANSION SLEEVE. THE ENTIRE ANCHOR SHALL BE CARBON STEEL MEETING THE FOLLOWING REQUIREMENTS.

ANCHOR SIZE	MINIMUM EMBEDMENT	PULLOUT (LBS)	SHEAR (LBS)
1/4" DIAMETER	1"	250	235
3/8" DIAMETER	1 1/4"	400	650
1/2" DIAMETER	2 1/4"	800	950
5/8" DIAMETER	3"	950	1250
3/4" DIAMETER	3 1/2"	1250	1900

LOADS INDICATED ABOVE ARE SERVICE LOADS IN POUNDS FOR SLEEVE ANCHORS IN CONCRETE MASONRY CONSTRUCTION. ANCHORS TO BE INSTALLED IN FULLY GROUTED CELLS.
- POST-INSTALLED ADHESIVE ANCHORS:

EMBED IN EPOXY RESIN ADHESIVE. THE ENTIRE ANCHOR SHALL BE ASTM A36 CARBON STEEL MEETING THE FOLLOWING REQUIREMENTS:

ANCHOR SIZE	MINIMUM EMBEDMENT	PULLOUT(LBS.)	SHEAR(LBS)
3/8" DIAMETER	3 1/2"	1550	1050
1/2" DIAMETER	4 1/4"	1750	1900
5/8" DIAMETER	5"	2250	2000
3/4" DIAMETER	7"	3700	2000

LOADS INDICATED ABOVE ARE SERVICE LOADS FOR STEEL ANCHORS IN CONCRETE MASONRY CONSTRUCTION. ANCHORS TO BE INSTALLED IN FULLY GROUTED CELLS.

051200 - STRUCTURAL STEEL FRAMING

- SPECIFICATIONS AND STANDARDS:

UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION AND ERECTION SHALL BE GOVERNED BY

 - ANSI/AISC 360 - SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS. ASD
 - AISC 341 SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS, INCLUDING SUPPLEMENT 1.
 - AISC 303 - CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES.
 - AWS STANDARD WELDING SYMBOLS.
 - AWS D1.1 STRUCTURAL WELDING CODE - STEEL. WELDING SHALL BE PERFORMED ONLY BY OPERATORS QUALIFIED, BY THE AWS STANDARD QUALIFICATION PROCEDURE, TO PERFORM THE PARTICULAR TYPE OF WORK REQUIRED.
 - SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS.
- TESTING:
 - WELDS: VISUAL TESTING SHALL BE PERFORMED BY AN INDEPENDENT TESTING LABORATORY ON ALL CRITICAL WELDS AND ON 25% OF NONCRITICAL WELDS. INADEQUATE WELDS SHALL BE STRENGTHENED OR CUT OUT AND REPLACED AS DIRECTED. CRITICAL WELDS SHALL BE DEFINED AS ALL FULL PENETRATION WELDS, ALL WELDS IN MOMENT CONNECTIONS AND AS NOTED AS CRITICAL WELDS ON THE STRUCTURAL DETAILS.
 - STRUCTURAL STEEL: PROVIDE MILL REPORTS FOR PROPERLY IDENTIFIED MATERIALS ON REQUEST.
 - A325 AND A490 BOLTS: PROVIDE BOLT INSPECTION AS DETAILED IN SECTION 9 OF SPECIFICATIONS FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS.
- MATERIALS:
 - "W" SHAPES: ASTM A992 Fy = 50 KSI.
 - CHANNELS: ASTM A36.
 - ANGLES, PLATES AND BARS: ASTM A36.
 - RECTANGULAR HOLLOW STRUCTURAL SECTIONS: ASTM A500 GR B, Fy = 46 KSI.
 - ROUND HOLLOW STRUCTURAL SECTIONS: ASTM A500, GR. B, Fy = 42 KSI.
 - WELDING ELECTRODES: AWS A5.1 OR A5.5 SERIES E70.
 - BOLTS: ASTM A325.
 - ANCHOR RODS: ASTM F1554 GR.36.
 - PAINT AND PROTECTION - NONE EXCEPT AS NOTED BELOW:
 - INTERIOR MEMBERS EXPOSED TO VIEW IN THE FINISHED STRUCTURE - PRIME COAT, TOUCH UP AFTER ERECTION.
 - MEMBERS EXPOSED TO WEATHER IN FINISHED STRUCTURE, SHELF ANGLES AND LINTELS IN EXTERIOR WALLS - GALVANIZED PER ASTM A123 AFTER FABRICATION.
 - SHRINKAGE-RESISTANT GROUT: ASTM C1107, NON-METALLIC AGGREGATE, NON-CORROSIVE, NON-STAINING.
- LINTELS:
 - LINTELS FOR EXTERIOR WALL OPENINGS - HOT DIPPED GALVANIZED.
 - 8" BEARING EACH SIDE OF OPENINGS UNLESS NOTED.
 - UNLESS SHOWN OTHERWISE, PROVIDE 1 ANGLE FOR EACH 4" WALL THICKNESS AS FOLLOWS:

MASONRY OPENING	ANGLE SIZE
3'-6" OR LESS	L 3 1/2x3 1/2x1/4
3'-7" TO 5'-0"	L 4x3 1/2x1/4 LLV
5'-1" TO 8'-0"	L 5x3 1/2x5/16 LLV
8'-1" TO 10'-0"	L 6x3 1/2x5/16 LLV
 - UNLESS SHOWN OTHERWISE, PROVIDE A W8x15+1/4" BOTTOM PLATE FOR OPENINGS LARGER THAN 10'-1" BUT LESS THAN 12'-0" WIDE AND EXTEND THE PLATE THE LENGTH OF THE BEARING EACH SIDE OF THE MASONRY OPENING. MINIMUM BEARING EACH SIDE SHALL BE 8" UNLESS OTHERWISE NOTED.
- CONNECTION REQUIREMENTS:
 - DESIGN CONNECTIONS FOR VERTICAL REACTIONS SHOWN ON DRAWINGS OR FOR FULL CAPACITY OF MEMBER WHERE NO REACTION IS SHOWN.
 - DESIGN MOMENT BEAM CONNECTIONS FOR VALUES SHOWN OR FOR FULL MOMENT CAPACITY OF MEMBER.
 - CONNECTIONS SHOWN AND DETAILED ON THE DRAWINGS MAY BE REDESIGNED BY THE STRUCTURAL STEEL CONTRACTOR FOR EQUAL FORCES PROVIDED THE SAME ARRANGEMENT OF MEMBERS IS USED AND THE OVERALL SIZE OF THE CONNECTION DOES NOT EXCEED THAT OF THE CONNECTION DETAILED.
 - OBTAIN APPROVAL FROM STRUCTURAL ENGINEER FOR TYPES OF CONNECTIONS BEFORE FABRICATION.
 - ALL BOLTED CONNECTIONS TO BE SHEAR/BEARING TYPE WITH BOLTS IN THE SNUG TIGHT CONDITION UNLESS NOTED OTHERWISE.
- MISCELLANEOUS REQUIREMENTS:
 - PROVIDE HOLES FOR OTHERS. IF SECTION IS WEAKENED BY MORE THAN 15% BY AN OPENING NOT SHOWN ON THE DRAWINGS, OBTAIN PRIOR APPROVAL.
 - STEEL FRAMING FOR OPENINGS FOR SUPPORTING OR CONNECTING TO MECHANICAL OR OTHER EQUIPMENT IS SHOWN FOR BRIDGING PURPOSES ONLY. CONTRACTOR SHALL COORDINATE SIZES AND LOCATIONS WITH MECHANICAL AND OTHER REQUIREMENTS BEFORE PROCEEDING WITH THE WORK. CONTRACTOR SHALL COORDINATE SIZES AND LOCATIONS OF STEEL ANGLE FRAMES FOR OPENINGS THAT ARE SHOWN ON THE MECHANICAL AND ARCHITECTURAL DRAWINGS.
 - STEEL BELOW GRADE TO BE PROTECTED BY A MINIMUM OF 3" OF CONCRETE OR 4" OF MASONRY.
 - ANGLE SUPPORTS FOR METAL DECK RIBS AT COLUMNS WHEN THE COLUMN SIZE PREVENTS THE RIBS FROM CONTINUING TO THE BEAMS THAT ARE SUPPORTING THE DECK AT COLUMN LINES.

052100 - STEEL JOIST FRAMING

- DESIGN, MANUFACTURING, AND ERECTION: ACCORDING TO THE STANDARD SPECIFICATIONS, LOAD TABLES & WEIGHT TABLES FOR STEEL JOISTS & JOIST GIRDERS ADOPTED BY THE STEEL JOIST INSTITUTE.
- STEEL JOISTS OF THE SAME DEPTH AND CHORD DESIGNATION SHALL HAVE MEMBER SIZES OF UNIFORM CONSISTENCY.
- PAINT ALL JOISTS WITH MANUFACTURERS STANDARD SHOP PRIMER EXCEPT THAT BLACK ASPHALT NOT PERMITTED.
- PROVIDE ADDITIONAL WEB MEMBERS AS REQUIRED AT CONCENTRATED LOADS THAT DO NOT OCCUR AT PANEL POINTS.
- BRIDGING:
 - ANCHOR ALL BRIDGING TO INTERSECTING WALLS AND BEAMS UNLESS OTHERWISE SHOWN
 - BRIDGING QUANTITY AND SPACING AS REQUIRED BY SJI.
- CONNECTIONS TO SUPPORTING STEEL:
 - WELDING - 2" OF 1/8" FILLET EA. SIDE FOR K AND KCS JOISTS.
 - BOLTING - (2) 1/2" DIAMETER A307 FOR K AND KCS JOISTS.
 - BOLT JOISTS AT OR NEAREST TO COLUMNS.
 - EXTEND BOTTOM CHORD OF JOISTS IN LINE WITH COLUMNS TO STABILIZER PLATES ON COLUMNS OR BEAMS.
- PROVIDE MATCHING HEIGHT SEATS ON JOISTS THAT HAVE COMMON BEARING.
- ADJACENT JOISTS OF THE SAME DEPTH ARE TO HAVE WEB MEMBERS IN LINE TO PERMIT PASSAGE OF MECHANICAL DUCTS.

01000 - GENERAL STRUCTURAL NOTES

- THE GENERAL STRUCTURAL NOTES ARE INTENDED TO AUGMENT THE DRAWINGS AND SPECIFICATIONS. SHOULD CONFLICTS EXIST BETWEEN THE DRAWINGS AND SPECIFICATIONS AND THE GENERAL STRUCTURAL NOTES, THE STRICTEST PROVISION SHALL GOVERN.
- GOVERNING CODE: OHIO BUILDING CODE - 2014 EDITION.
- SEE STRUCTURAL PLANS FOR DESIGN SOIL BEARING PRESSURE AND LIVE LOADS. LIVE LOADS REDUCED IN ACCORDANCE WITH THE GOVERNING CODE.
- ROOF SNOW LOAD:

GROUND SNOW LOAD (Pg)	- 20 PSF
SNOW EXPOSURE FACTOR (Ce)	- 1.0
IMPORTANCE FACTOR (Is)	- 1.0
THERMAL FACTOR (Ct)	- 1.0
FLAT ROOF SNOW LOAD (Pf)	- 20 PSF
- WIND LOAD:

BASIC WIND SPEED	- 90 MPH
IMPORTANCE FACTOR (Iw)	- 1.0
EXPOSURE CATEGORY	- EXPOSURE B
INTERNAL PRESSURE COEFFICIENT(G Cp)	- ±0.18
- SEISMIC LOAD:

OCCUPANCY CATEGORY	- II
IMPORTANCE FACTOR (Ie)	- 1.0
MAPPED SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD (Ss)	- 0.15
MAPPED SPECTRAL RESPONSE ACCELERATION AT ONE-SECOND PERIOD (S1)	- 0.06
SITE CLASS	- D
SPECTRAL RESPONSE COEFFICIENT AT SHORT PERIOD (SDs)	- 0.16
SPECTRAL RESPONSE COEFFICIENT AT ONE-SECOND PERIOD (SD1)	- 0.09
SEISMIC DESIGN CATEGORY	- B
DESIGN BASE SHEAR	- 14.7k
SEISMIC RESPONSE COEFFICIENT (Cs)	- 0.045

BASIC SEISMIC FORCE RESISTING SYSTEM:
STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE (R=3, Cd=3)
DESIGN BY EQUIVALENT LATERAL FORCE PROCEDURE

- MECHANICAL FRAMING LOADS, OPENINGS, AND STRUCTURE IN ANY WAY RELATED TO MECHANICAL REQUIREMENTS ARE SHOWN FOR BIDDING PURPOSES ONLY. CONTRACTOR SHALL OBTAIN APPROVAL OF MECHANICAL AND OTHER TRADES BEFORE PROCEEDING WITH SUCH PORTION OF THE WORK. EXCESS COST RELATED TO VARIATION IN MECHANICAL REQUIREMENTS TO BE BORNE BY MECHANICAL CONTRACTOR. COORDINATE SIZE AND LOCATION OF ALL OPENINGS WITH THE MECHANICAL DRAWINGS.
- THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE DEMOLITION PROCEDURES, ERECTION PROCEDURES AND SEQUENCES AND TO ENSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES THE ADDITION OF WHATEVER TEMPORARY BRACING, GUYS, OR TIE-DOWNS WHICH MIGHT BE NECESSARY. SUCH MATERIAL SHALL REMAIN THE CONTRACTOR'S PROPERTY AFTER COMPLETION OF THE PROJECT.
- IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION.
- THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND CONDITIONS RELATING TO EXISTING CONSTRUCTION AND EXISTING SERVICE ON THE SITE.
- THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND LOCATIONS OF COLUMNS, WALLS, OPENINGS ETC. WITH THE ARCHITECTURAL DRAWINGS PRIOR TO PROCEEDING WITH THE WORK.

033000 - REINFORCED CONCRETE

- SPECIFICATIONS AND STANDARDS:

CONCRETE WORK, DETAILING, FABRICATION AND PLACING OF BARS AND CONCRETE SHALL BE GOVERNED BY THE APPLICABLE VERSION OF:

 - ACI 301, ACI 315, AND ACI 318.
 - CRSI RECOMMENDED PRACTICE FOR PLACING REINFORCING BARS.
 - ACI 306 AND ACI 305 FOR WINTER AND HOT WEATHER CONCRETING, RESPECTIVELY.

THE CONTRACTOR SHALL AT ALL TIMES HAVE A COPY OF THE RELEVANT SPECIFICATIONS QUOTED ABOVE ON THE SITE AND THE SUPERVISORY PERSONNEL SHALL BE THOROUGHLY FAMILIAR WITH THE CONTENTS THEREOF.
- CONTINGENCIES:
- CONCRETE REQUIREMENTS AND LOCATION IN JOB:

CLASS	LOCATION	f _c	SPECIAL REQUIREMENTS
1	FOOTINGS, PIERS, WALLS, EQUIPMENT PAD	3000 PSI	
2	EXTERIOR CONCRETE	4000 PSI	5% ±1% AIR CONTENT*
3	INTERIOR SLABS-ON-GRADE	3500 PSI	
4	MASONRY GROUT	3000 PSI	7" SLUMP 3/8" MAX. AGG.
5	EARTH FILL	1500 PSI	NO TESTS

- * REFER TO SPEC SECTION 32 13 13 FOR ADDITIONAL REQUIREMENTS.
SUBMIT CONCRETE MIXES FOR APPROVAL IN ACCORDANCE WITH ACI 301 BEFORE PLACING ANY CONCRETE.
- REINFORCING REQUIREMENTS:
 - BARs: ASTM A615 - GRADE 60.
 - WELDED WIRE REINFORCING: ASTM A185.
 - SMOOTH BARS: ASTM A36.
 - POST INSTALLED WEDGE ANCHORS:
 - THE ENTIRE ANCHOR SHALL BE CARBON STEEL.
 - THE ENTIRE ANCHOR SYSTEM SHALL BE EVALUATED TO COMPLY WITH THE APPLICABLE VERSION OF IBC SECTION 1912 AND MEETING THE REQUIREMENTS OF ACI 308.2 PREQUALIFICATION TESTS FOR USE IN CRACKED CONCRETE.
 - SUBJECT TO COMPLIANCE WITH REQUIREMENTS, PROVIDE THE PRODUCT INDICATED ON DRAWINGS OR COMPARABLE PRODUCT CAPABLE OF RESISTING LOADS EQUIVALENT TO THE BASIS OF DESIGN PRODUCT WHEN USED WITH THE SAME EMBEDMENT, ORIENTATION, EDGE DISTANCE, AND SPACING. BASIS OF DESIGN: HILTI KWIK BOLT TZ
 - POST INSTALLED ADHESIVE ANCHORS AND DOWELS:
 - THE ENTIRE ANCHOR SYSTEM SHALL BE EVALUATED TO COMPLY WITH THE APPLICABLE VERSION OF IBC AND BE CERTIFIED BY AN ICC-ES EVALUATION REPORT AS AN ALTERNATIVE ANCHOR TO THOSE DESCRIBED IN IBC SECTIONS 1911 AND 1912. THE ANCHOR SYSTEM SHALL BE EVALUATED FOR USE IN CRACKED CONCRETE WITH ACI 318-08 APPENDIX D.
 - SUBJECT TO COMPLIANCE WITH REQUIREMENTS, PROVIDE THE PRODUCT INDICATED ON DRAWINGS OR COMPARABLE PRODUCT CAPABLE OF RESISTING LOADS EQUIVALENT TO THE BASIS OF DESIGN PRODUCT WHEN USED WITH THE SAME EMBEDMENT, ORIENTATION, EDGE DISTANCE, AND SPACING. BASIS OF DESIGN: HILTI HIT HY 150 MAX SD WITH ISO 998-1 CLASS 5.8 THREADED ROD. SUBMIT PROPOSED SUBSTITUTION FOR APPROVAL WITH ACCOMPANYING ICC-ES REPORT.
 - FOOTINGS:
 - DOWELS IN FOOTINGS TO MATCH VERTICAL REINFORCING IN CONCRETE WALLS, COLUMNS AND PIERS.
 - BEND ALL BARS 24 DIAMETERS AROUND CORNERS OF FOOTINGS. BARS AT THE INSIDE FACE OF THE CORNER SHALL BE CONTINUED ACROSS TO THE OUTSIDE AND THEN BENT.
 - SLABS:
 - IF NO OTHER REINFORCING IS SHOWN IN A SLAB ON GRADE, PROVIDE 6x6-W1.4xW1.4 WWR AT MID-THICKNESS OF SLAB.
 - SPLICES:
 - COMPRESSION SPLICES: LAP 30 DIAMETERS.
 - TENSION SPLICES, WHEN PERMITTED - LAP IN ACCORDANCE WITH THE ACI CODE.
 - LAP WELDED WIRE REINFORCING 1 SPACE + 2" AT ALL EDGES AND ENDS OF SHEETS.
 - OPENINGS:
 - OPENINGS SHOWN ARE FOR BIDDING PURPOSES ONLY. CONTRACTOR SHALL COORDINATE SIZES AND LOCATIONS WITH MECHANICAL AND OTHER REQUIREMENTS BEFORE PROCEEDING WITH THE WORK.
 - IF ANY OPENING NOT SHOWN ON THE PLAN IS REQUIRED, APPROVAL MUST BE SECURED FROM THE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH THE WORK.
 - MISCELLANEOUS:
 - CONSTRUCTION JOINTS PERMITTED ONLY WHERE SHOWN OR AS APPROVED BY THE STRUCTURAL ENGINEER.
 - PROVIDE ONE #4x3'-0" DIAGONAL REINFORCING BAR AT MID-DEPTH OF SLAB AT ALL RE-ENTRANT CORNERS OF SLABS ON GRADE.



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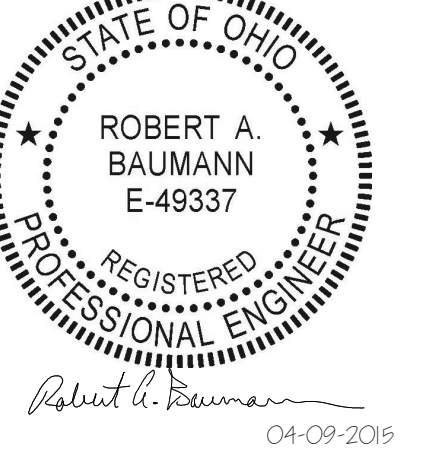
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LANDSCAPE ARCHITECT

CIVIL AND MEP ENGINEER

STRUCTURAL ENGINEER

PROFESSIONAL ENGINEER



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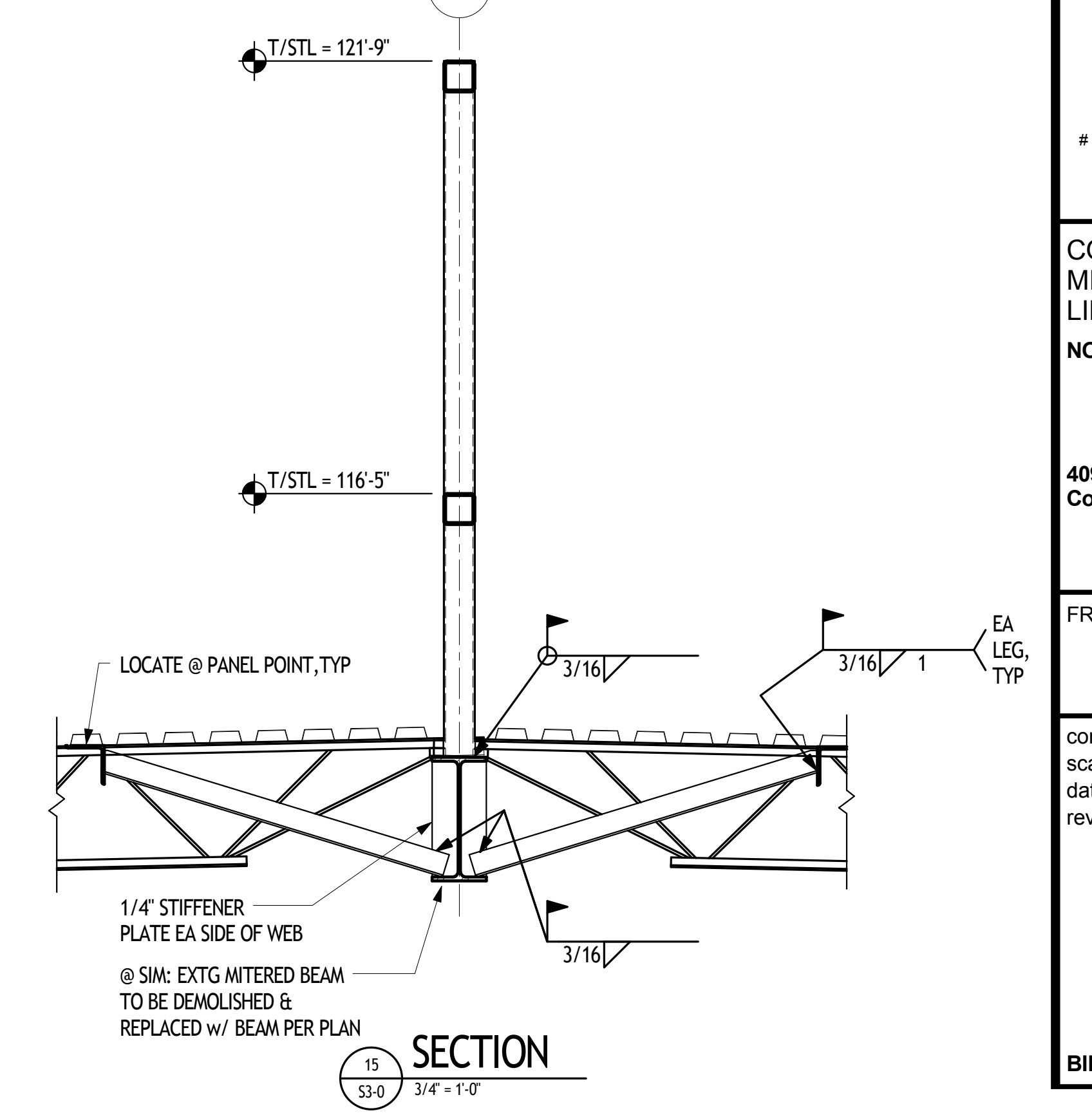
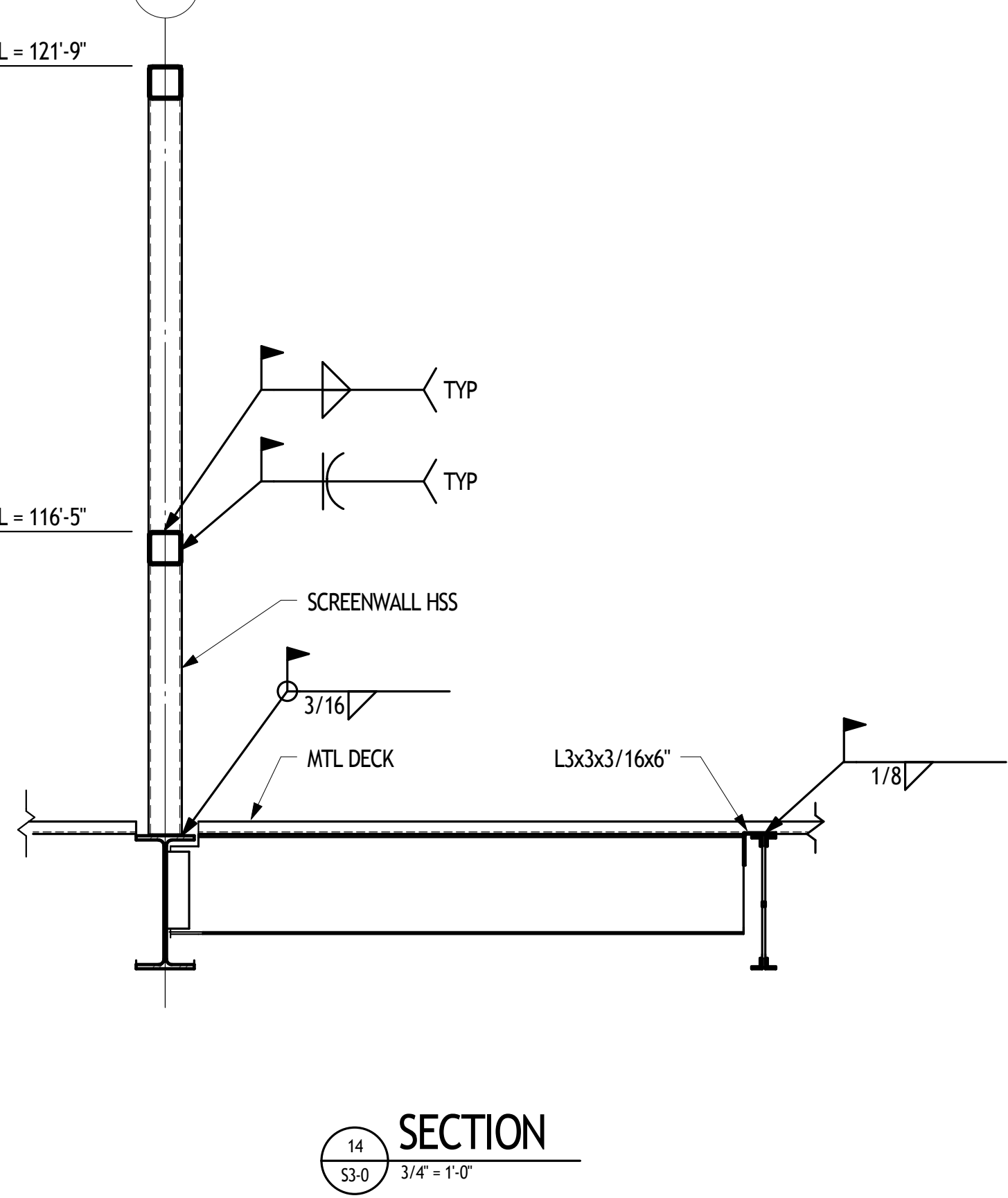
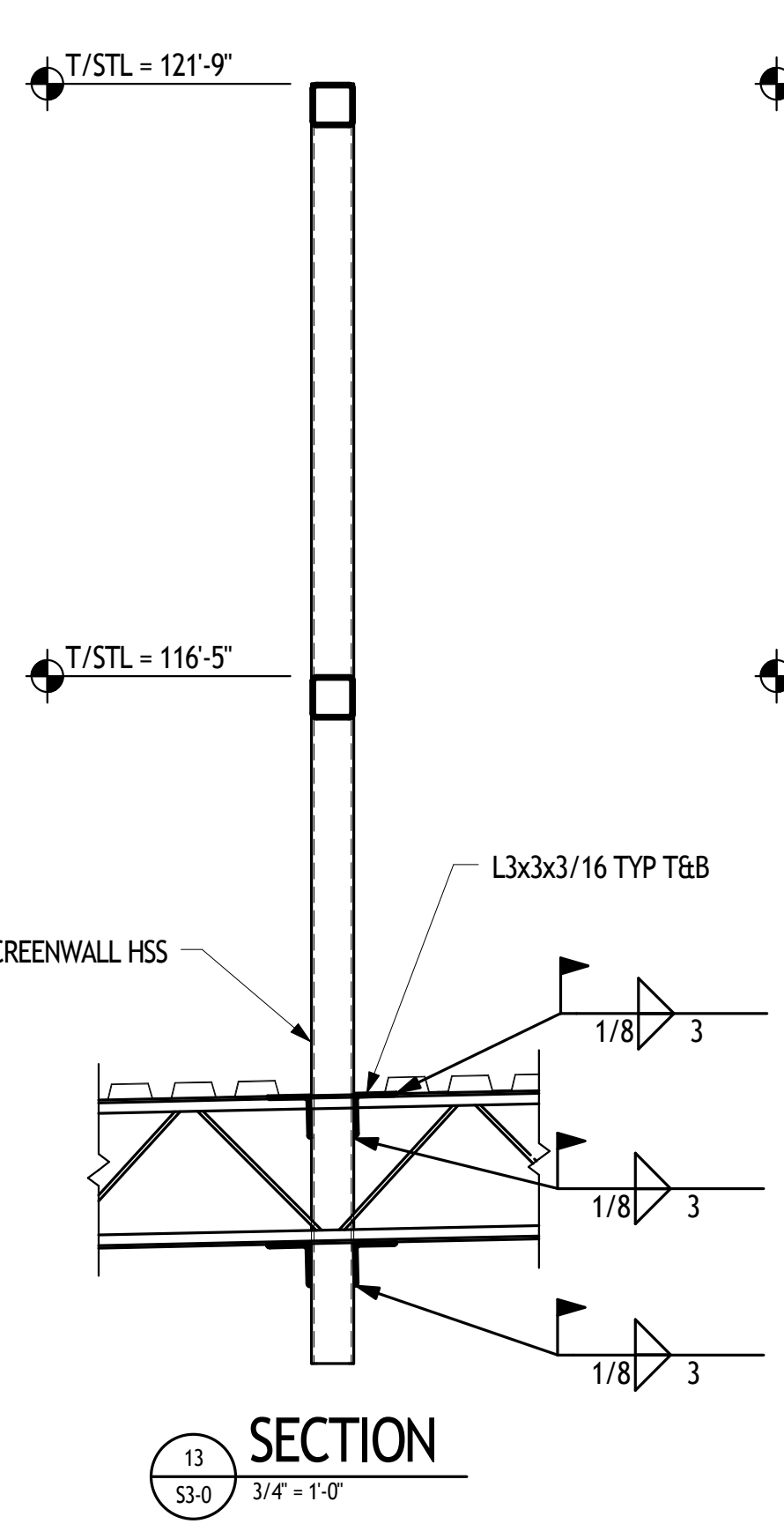
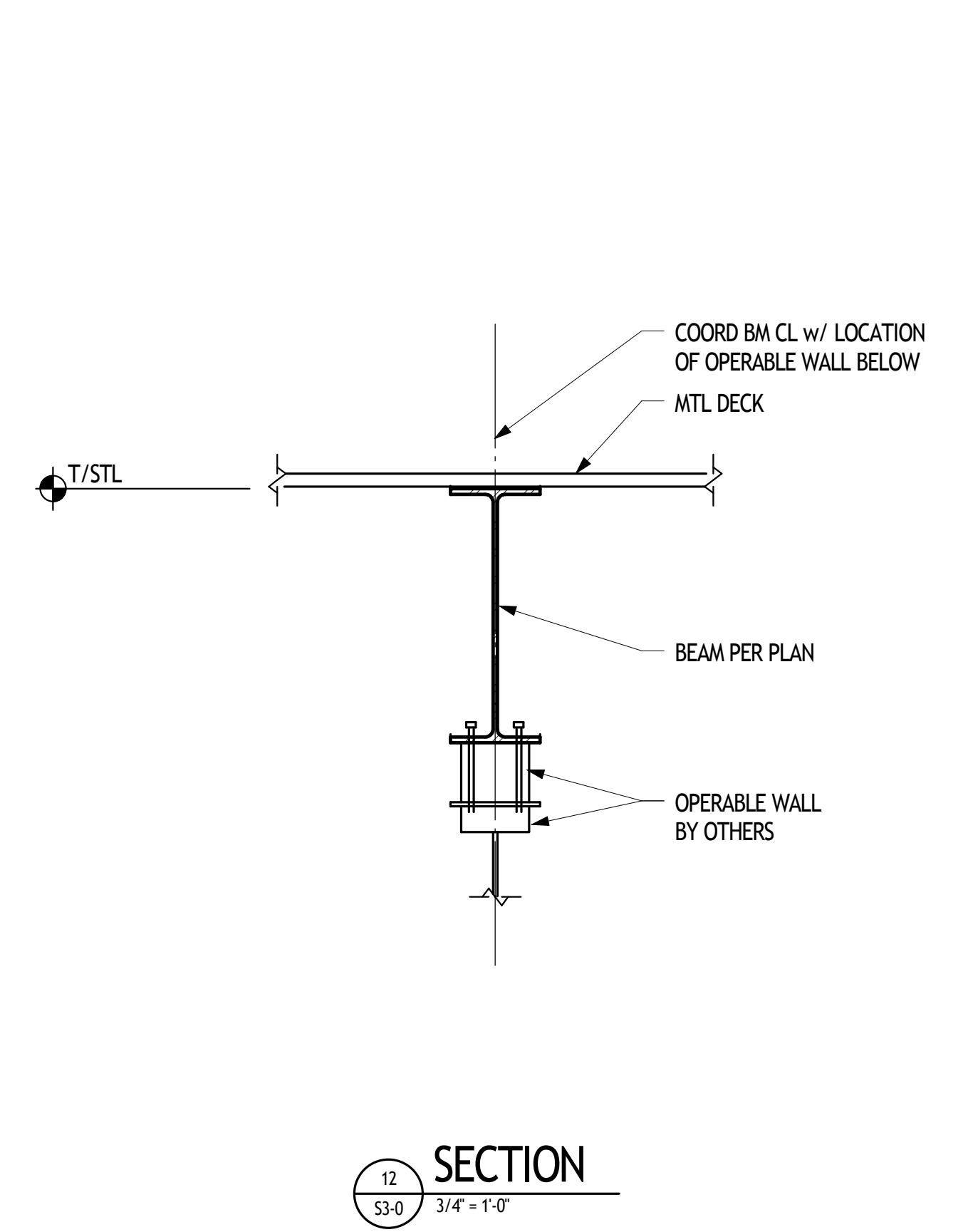
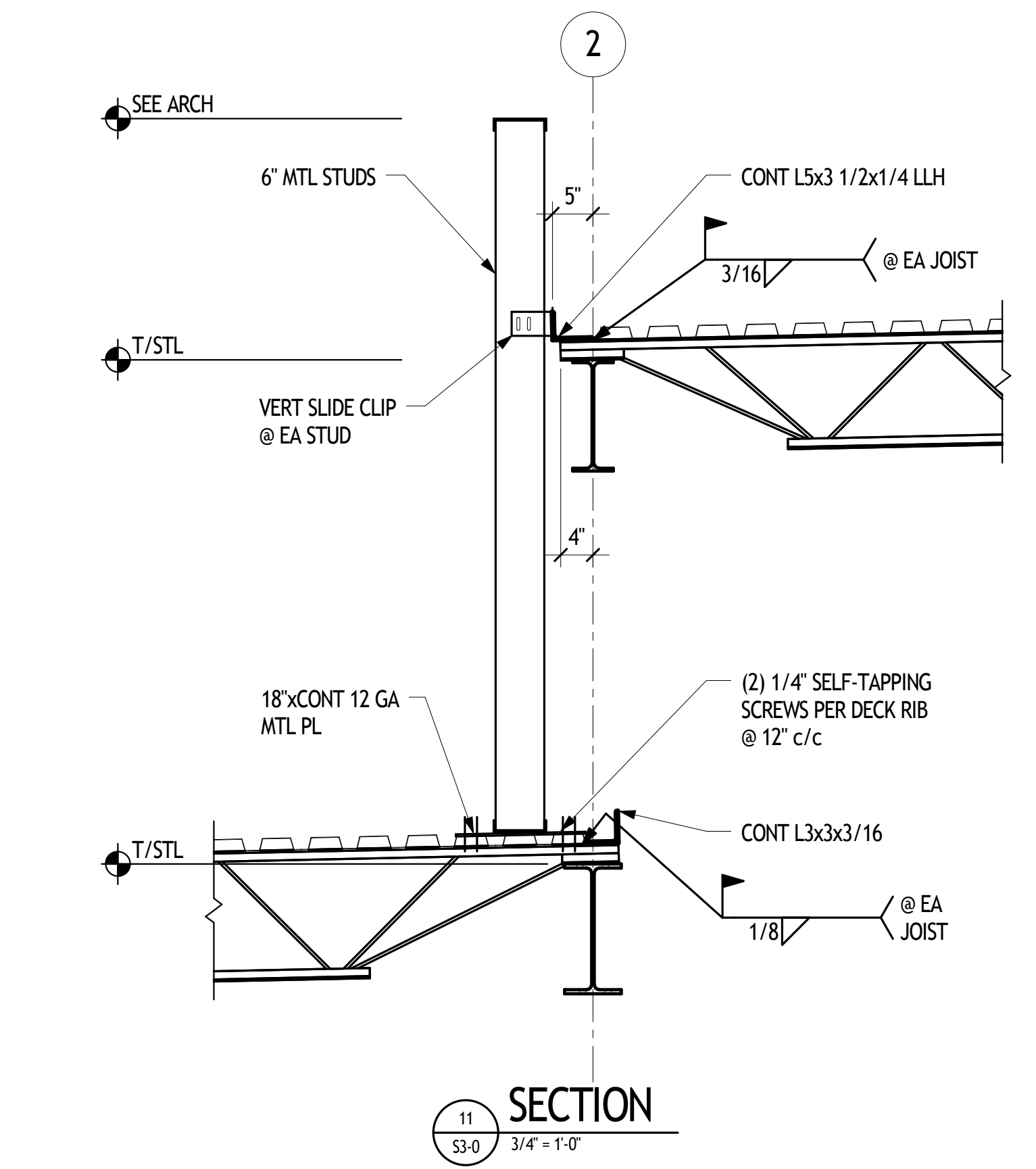
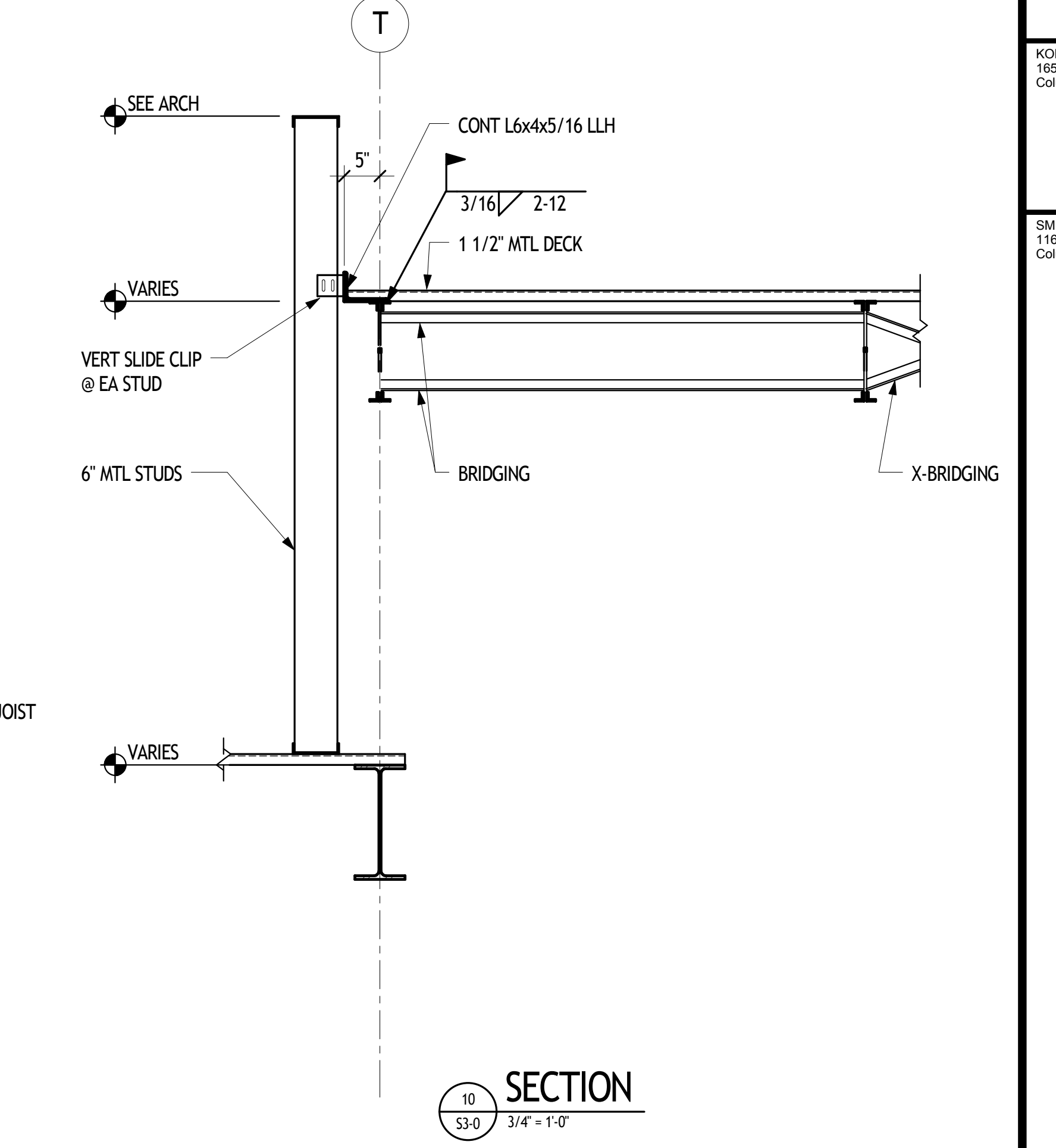
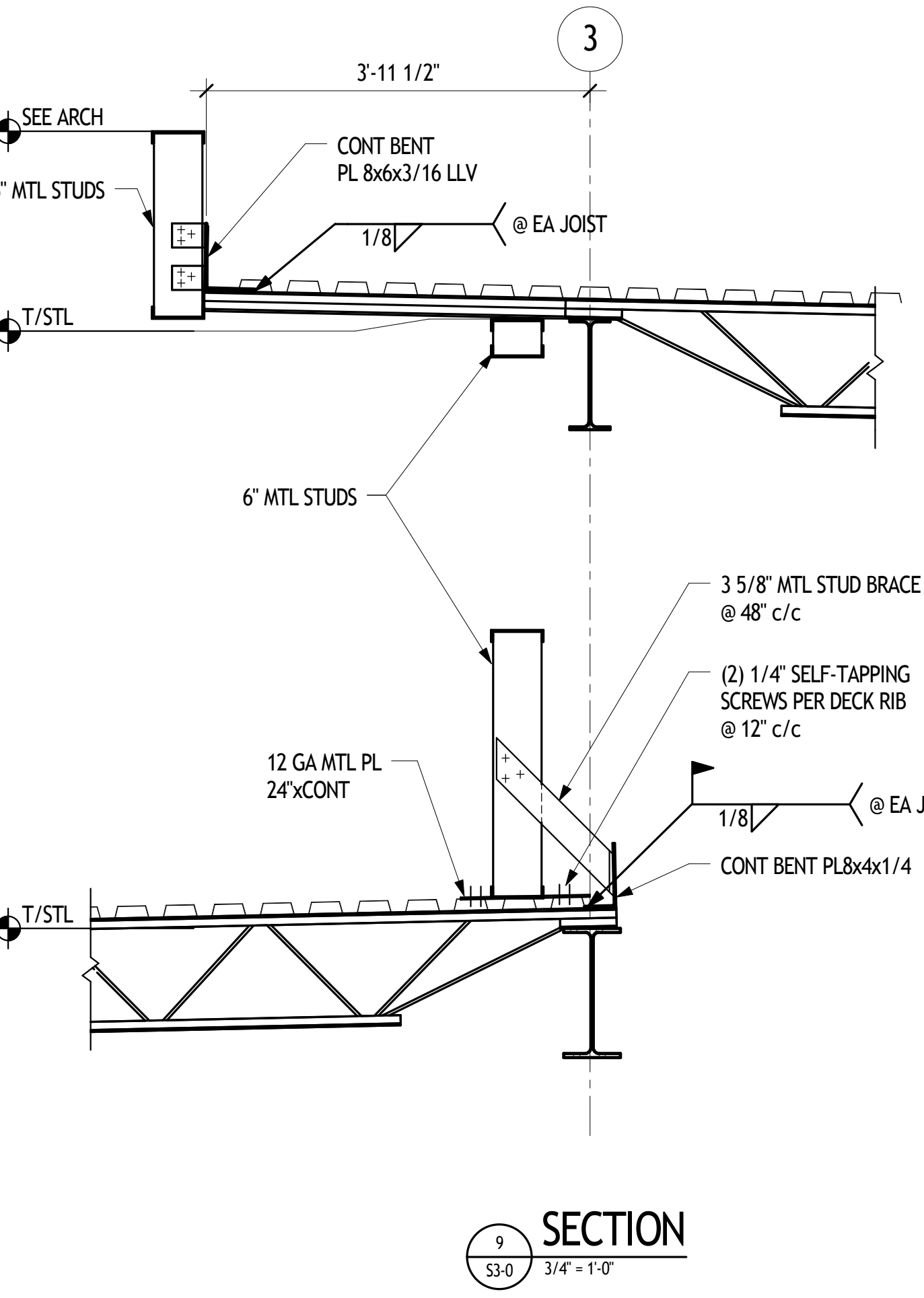
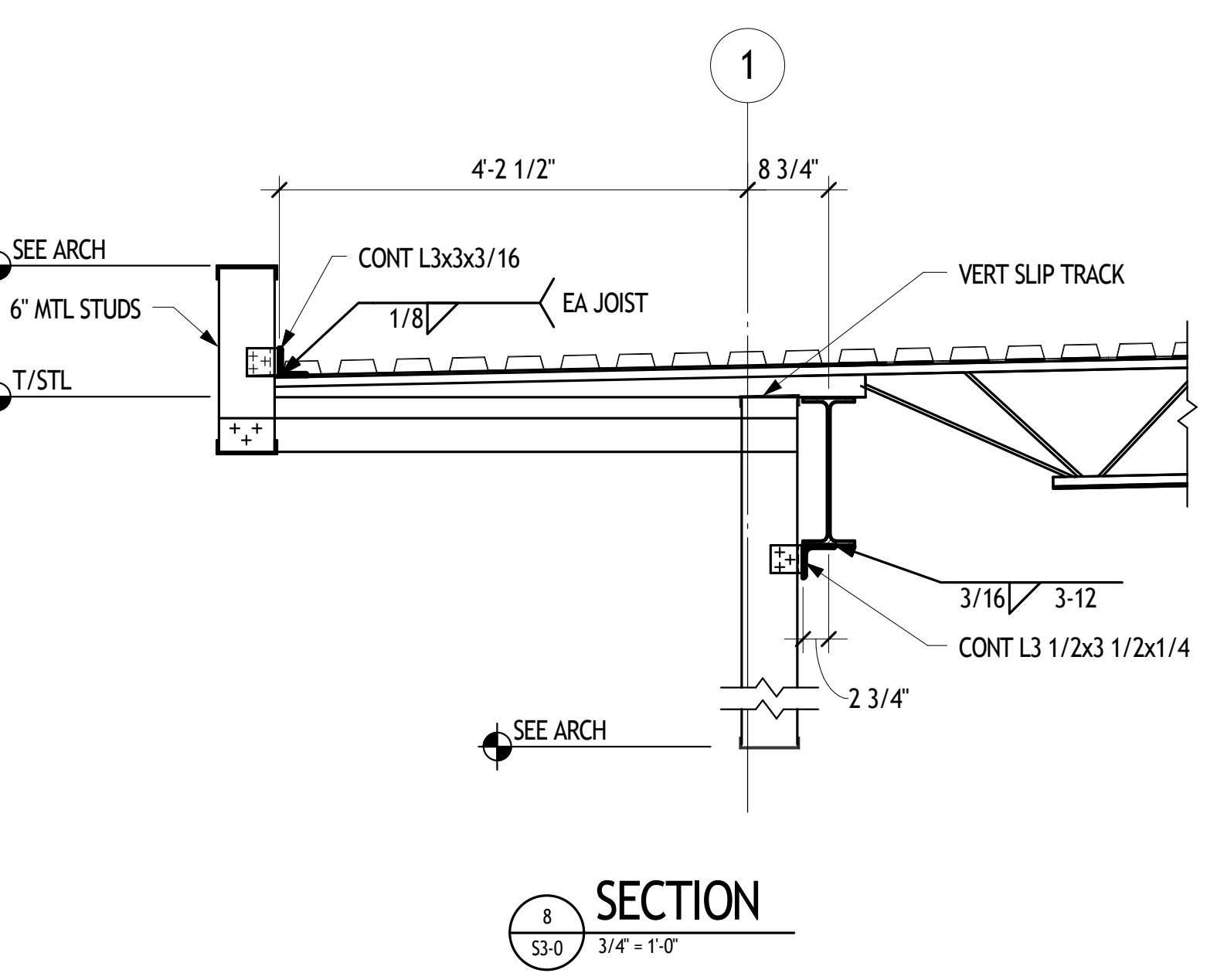
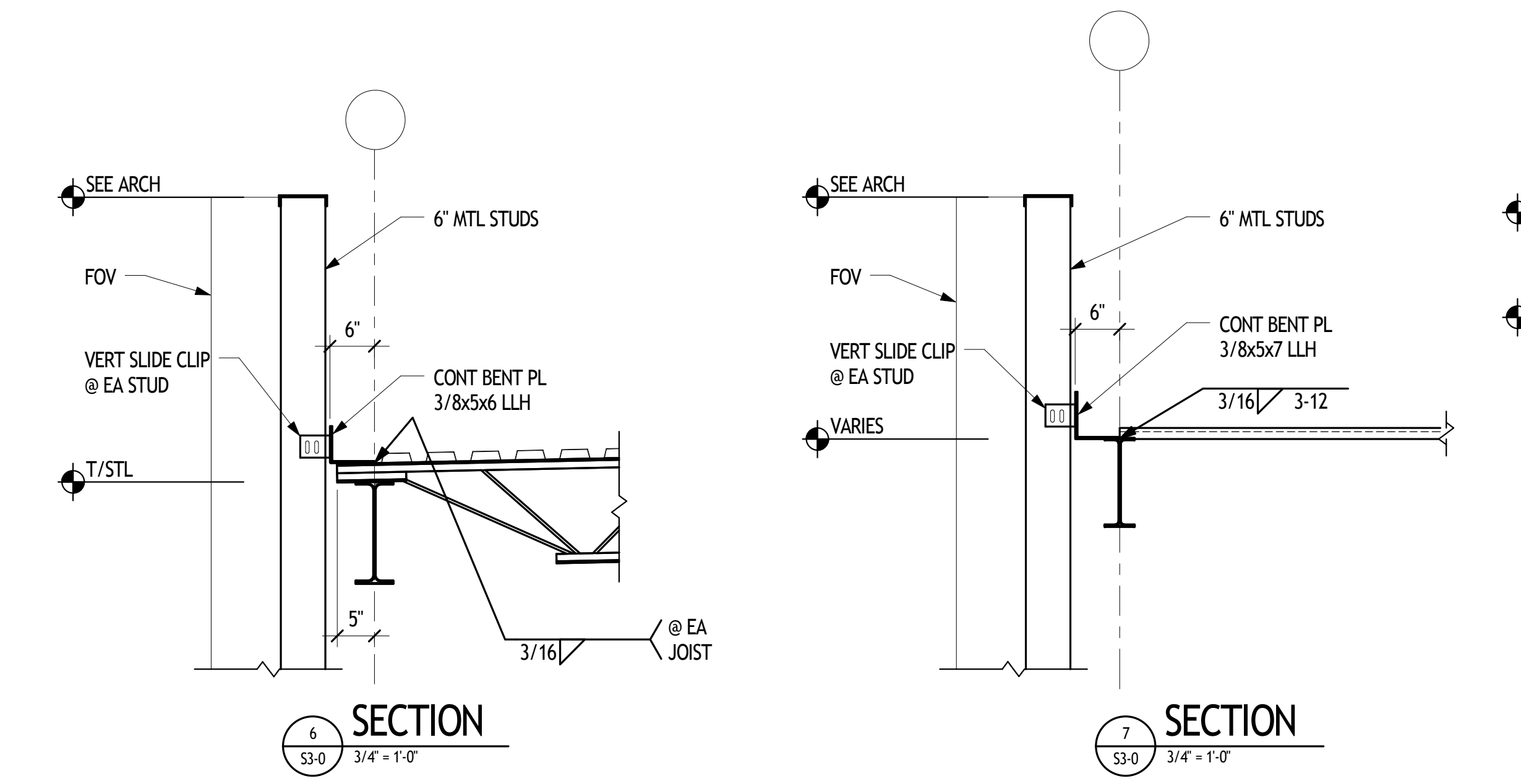
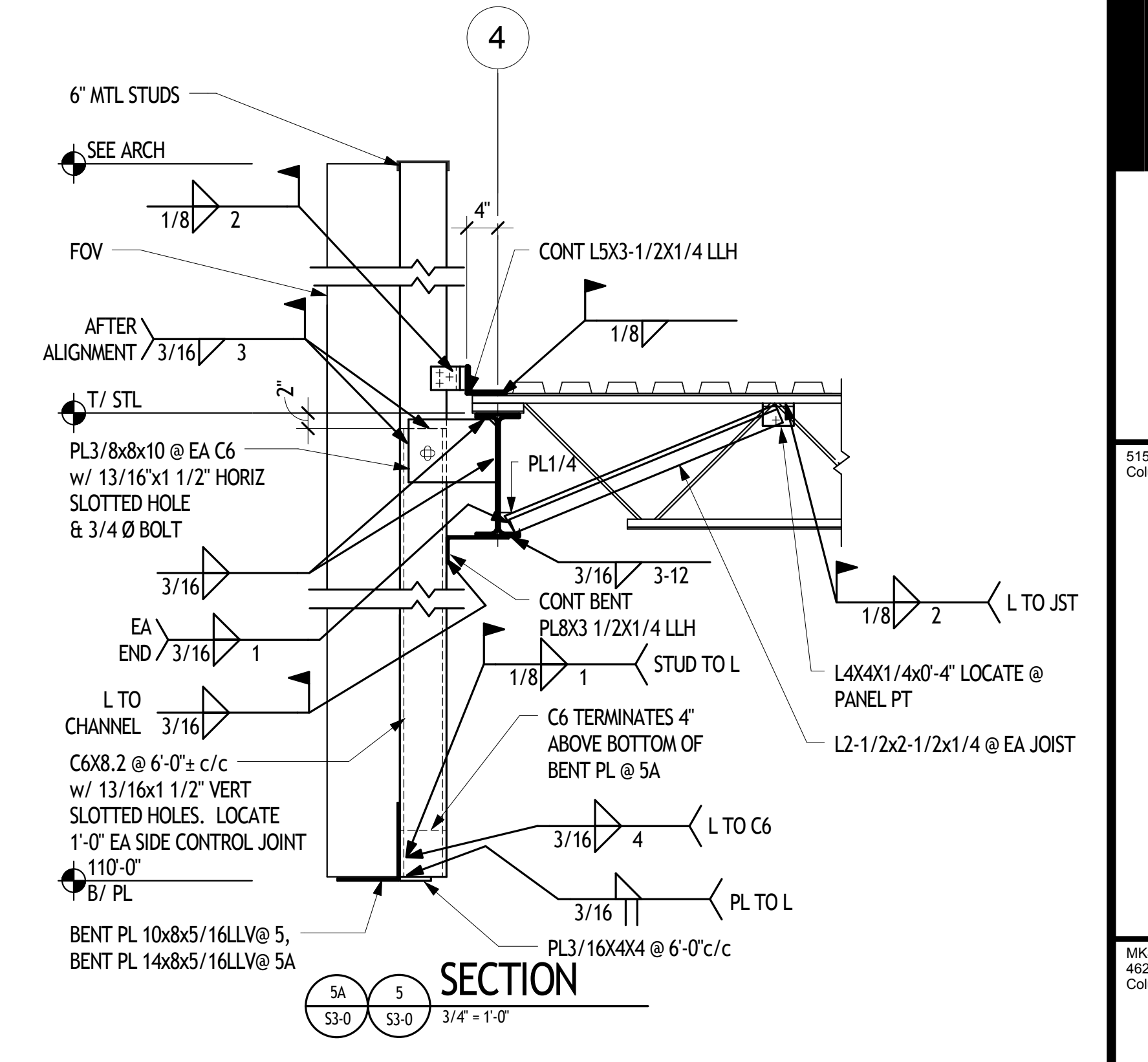
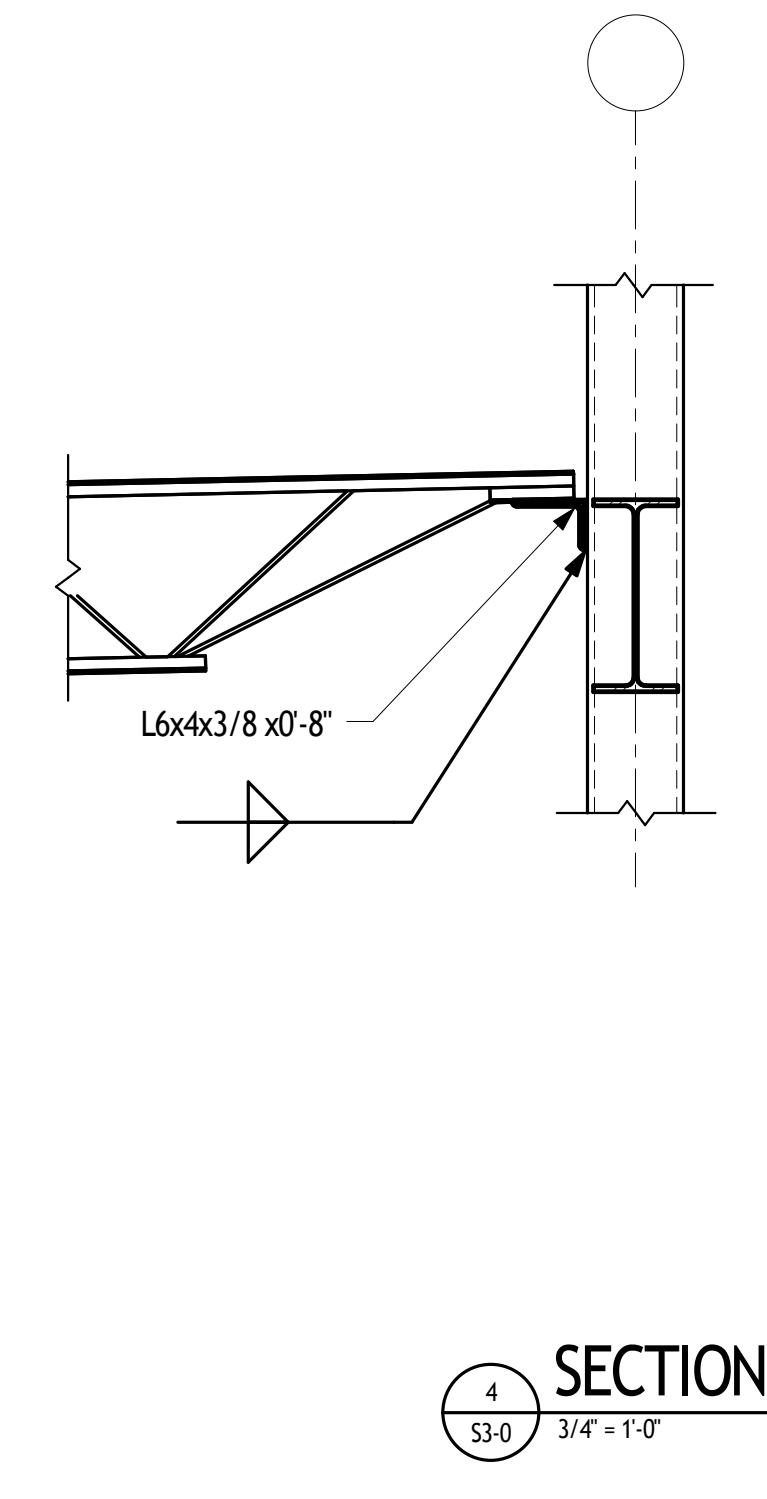
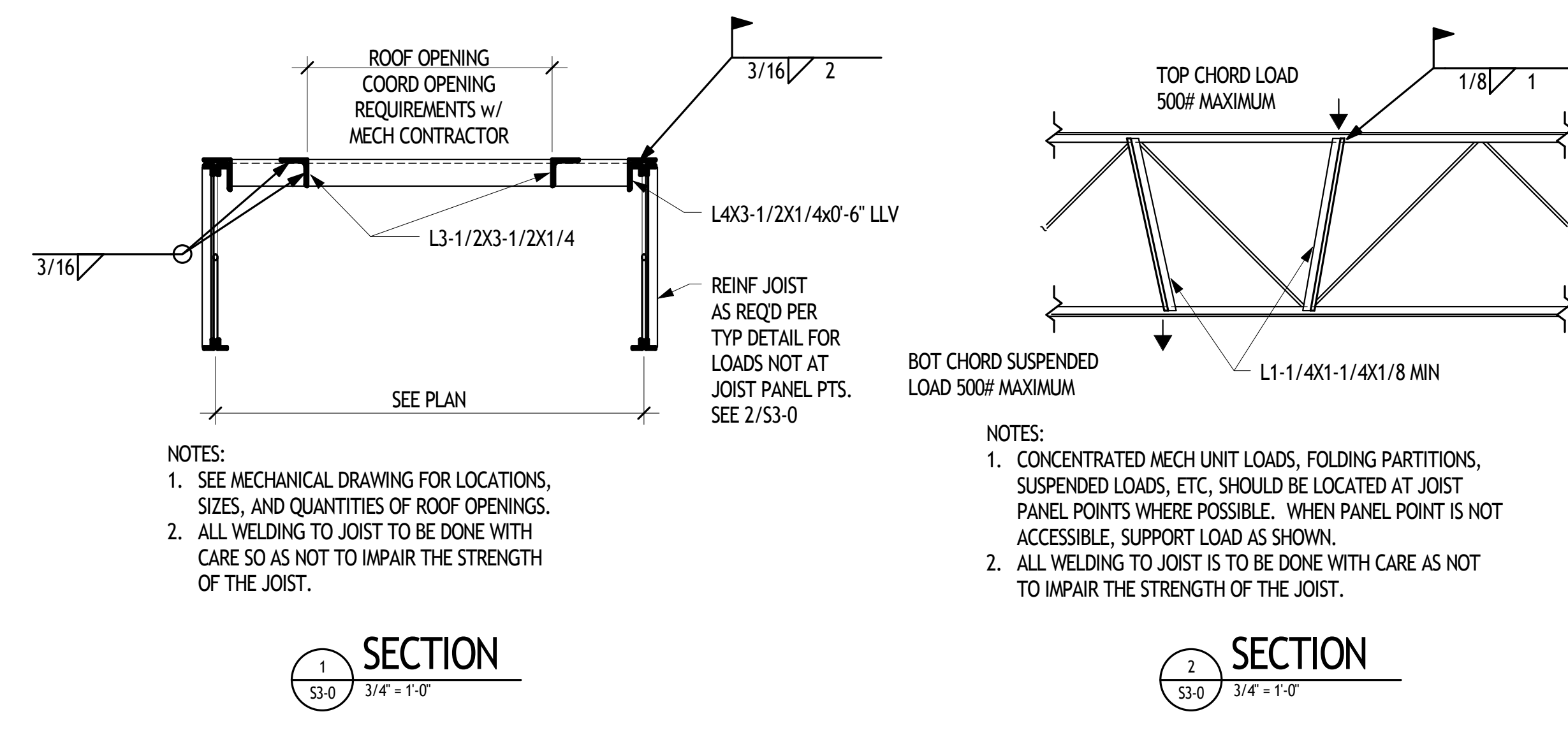
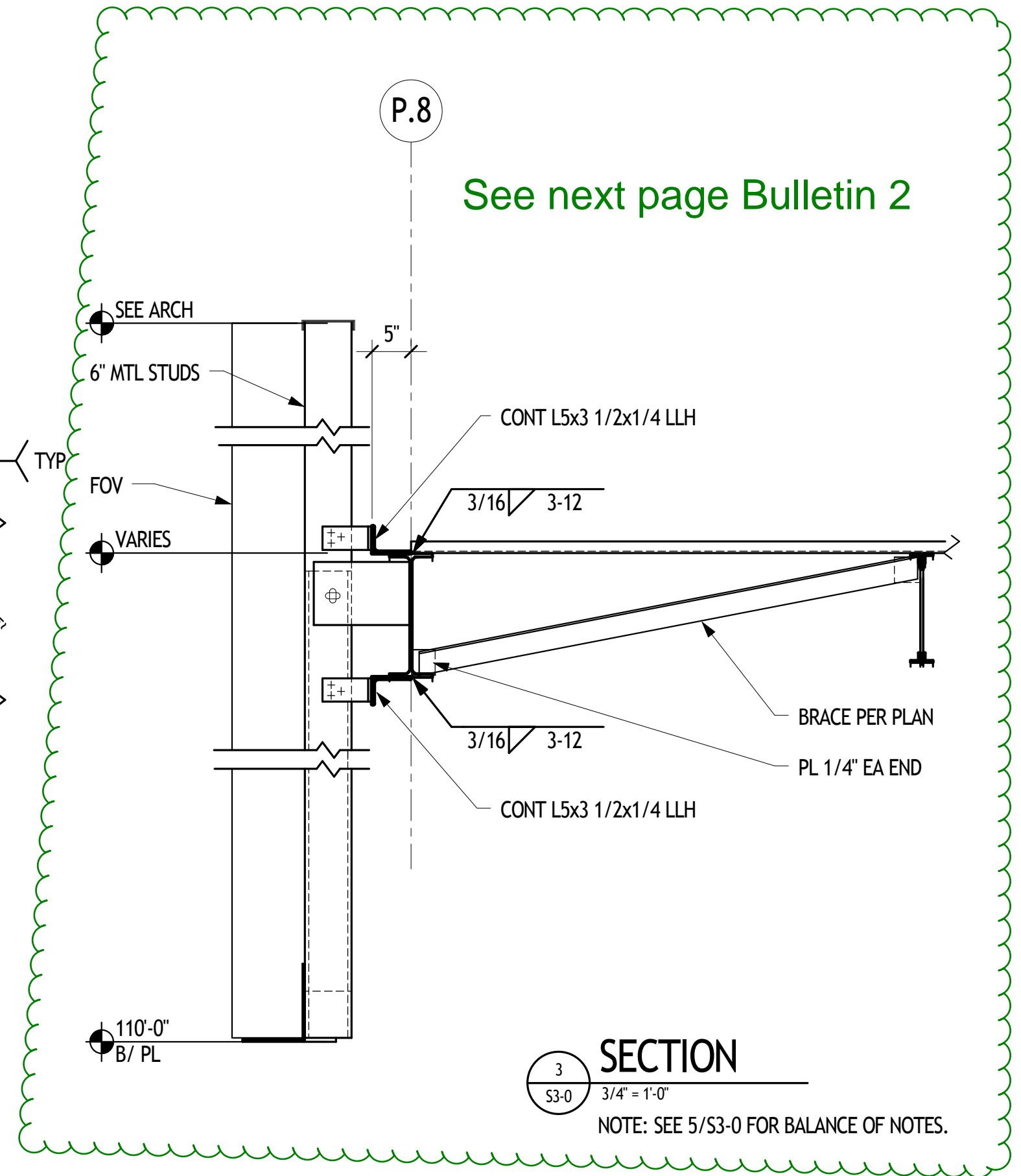
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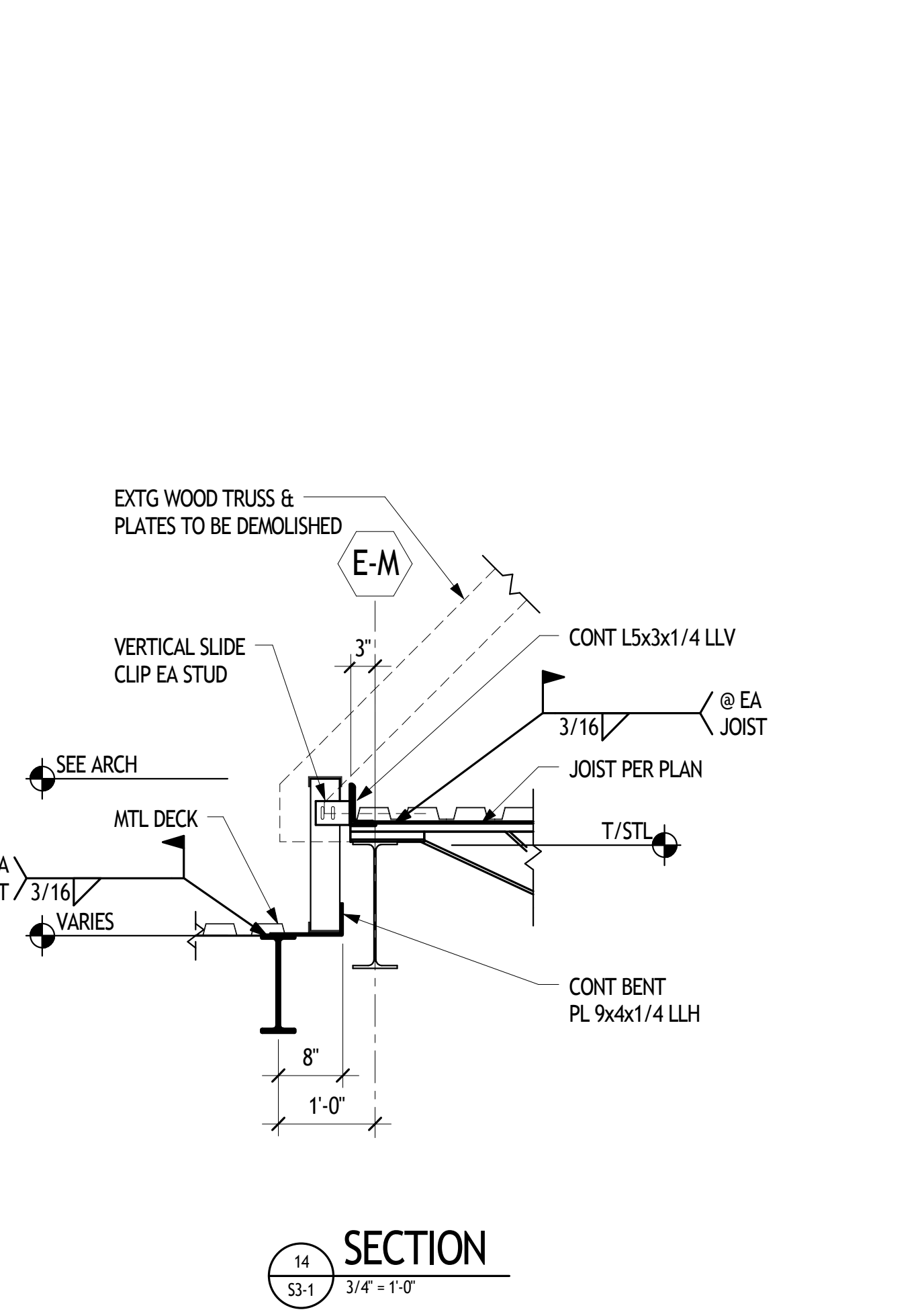
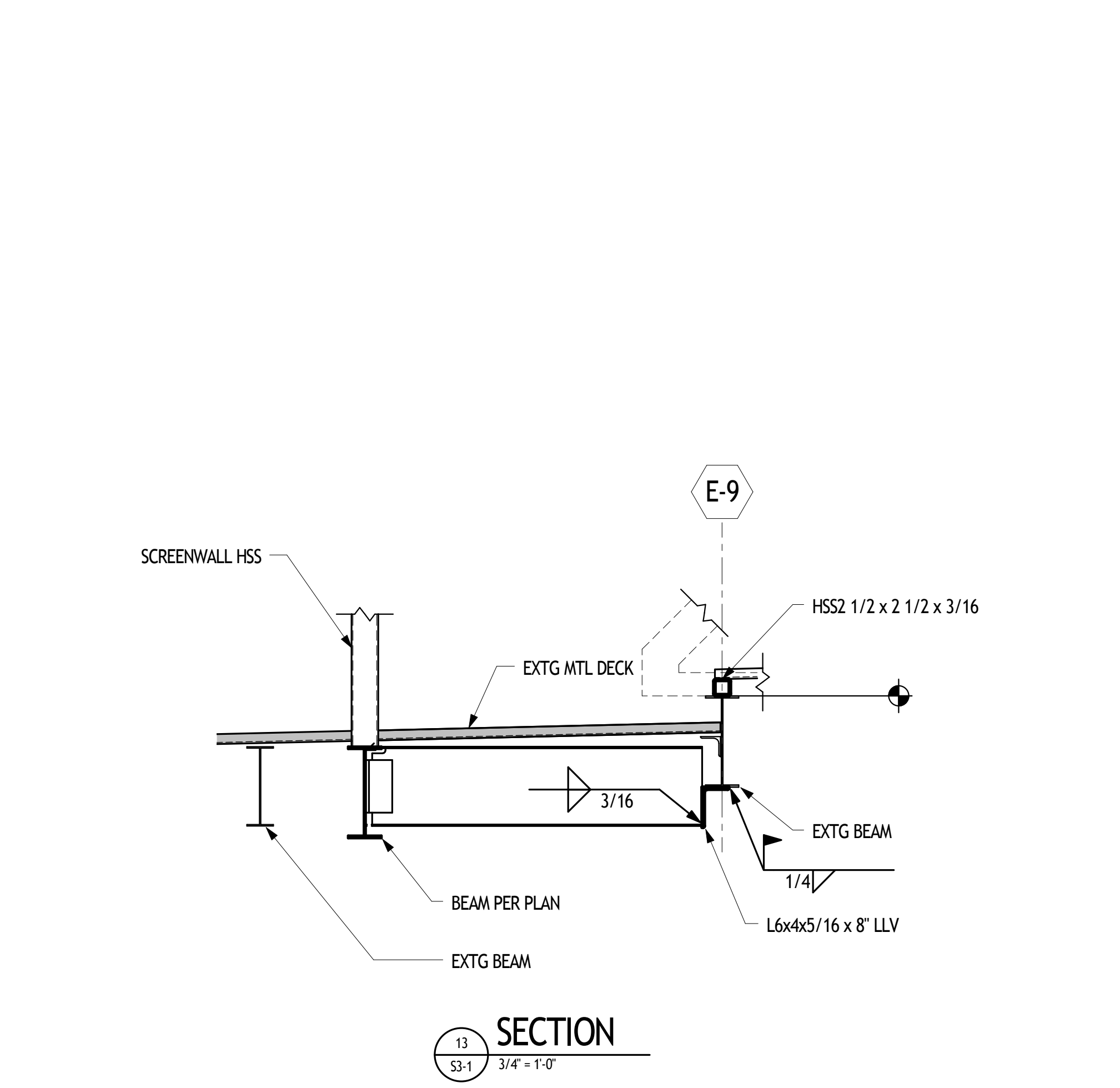
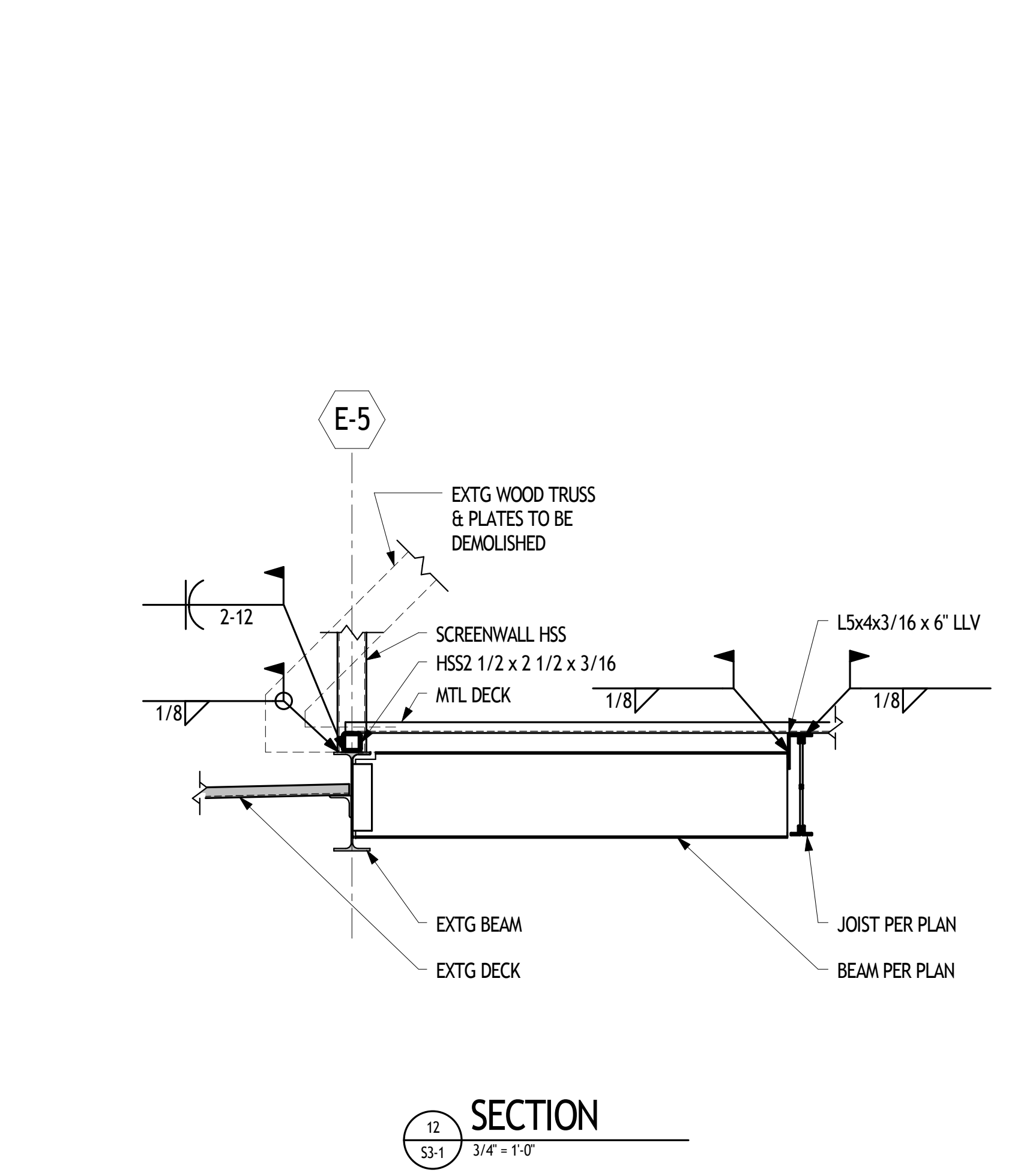
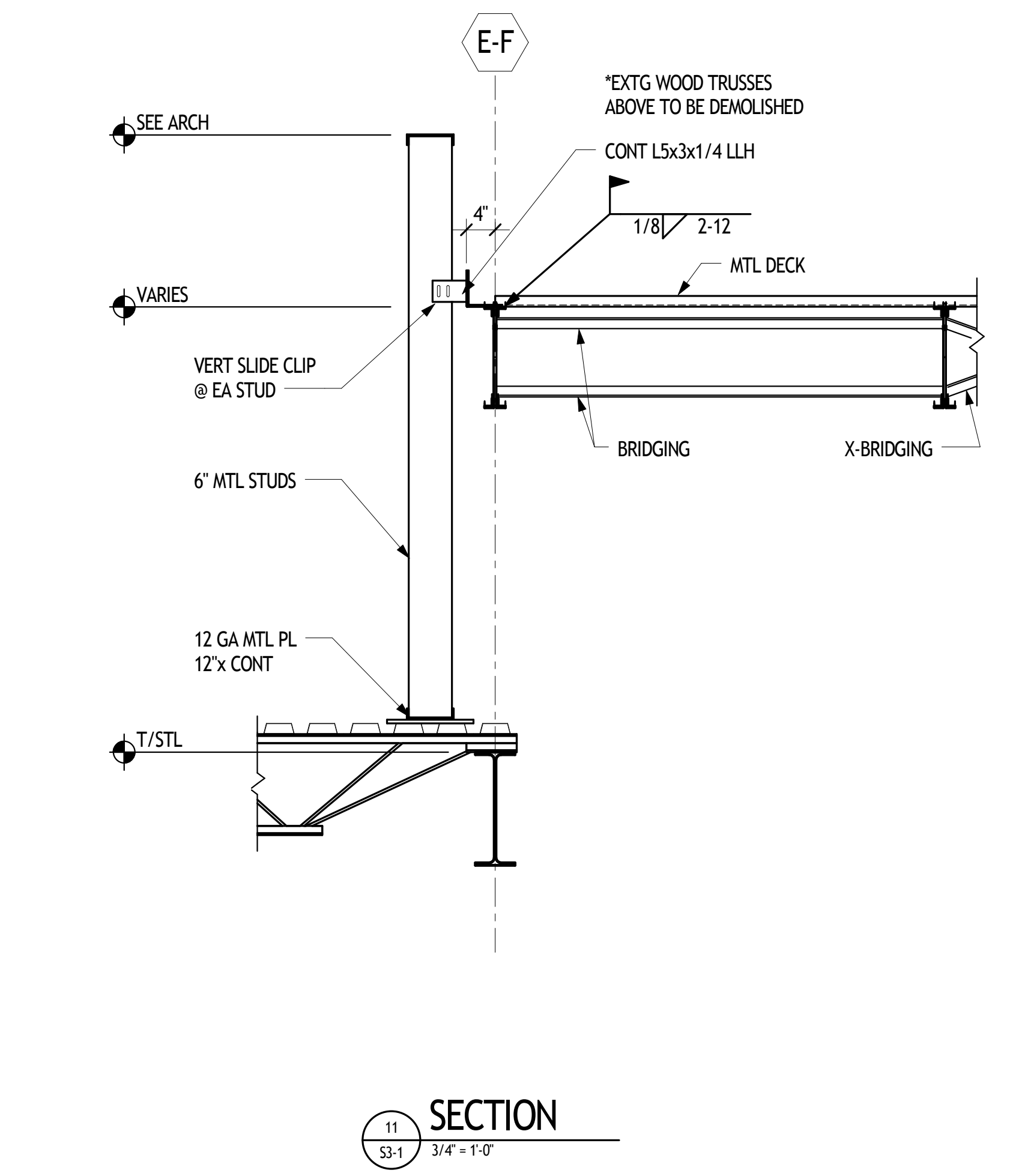
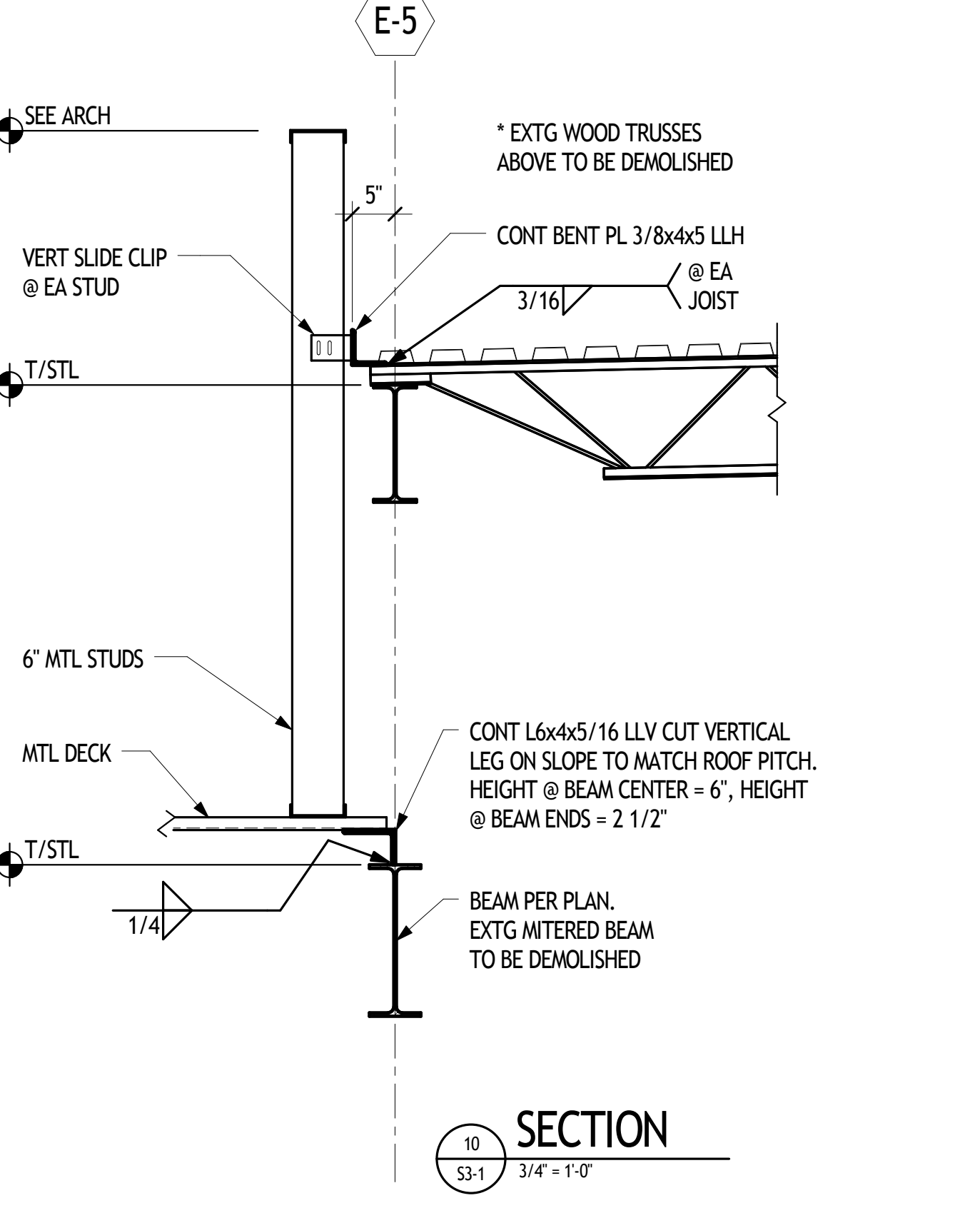
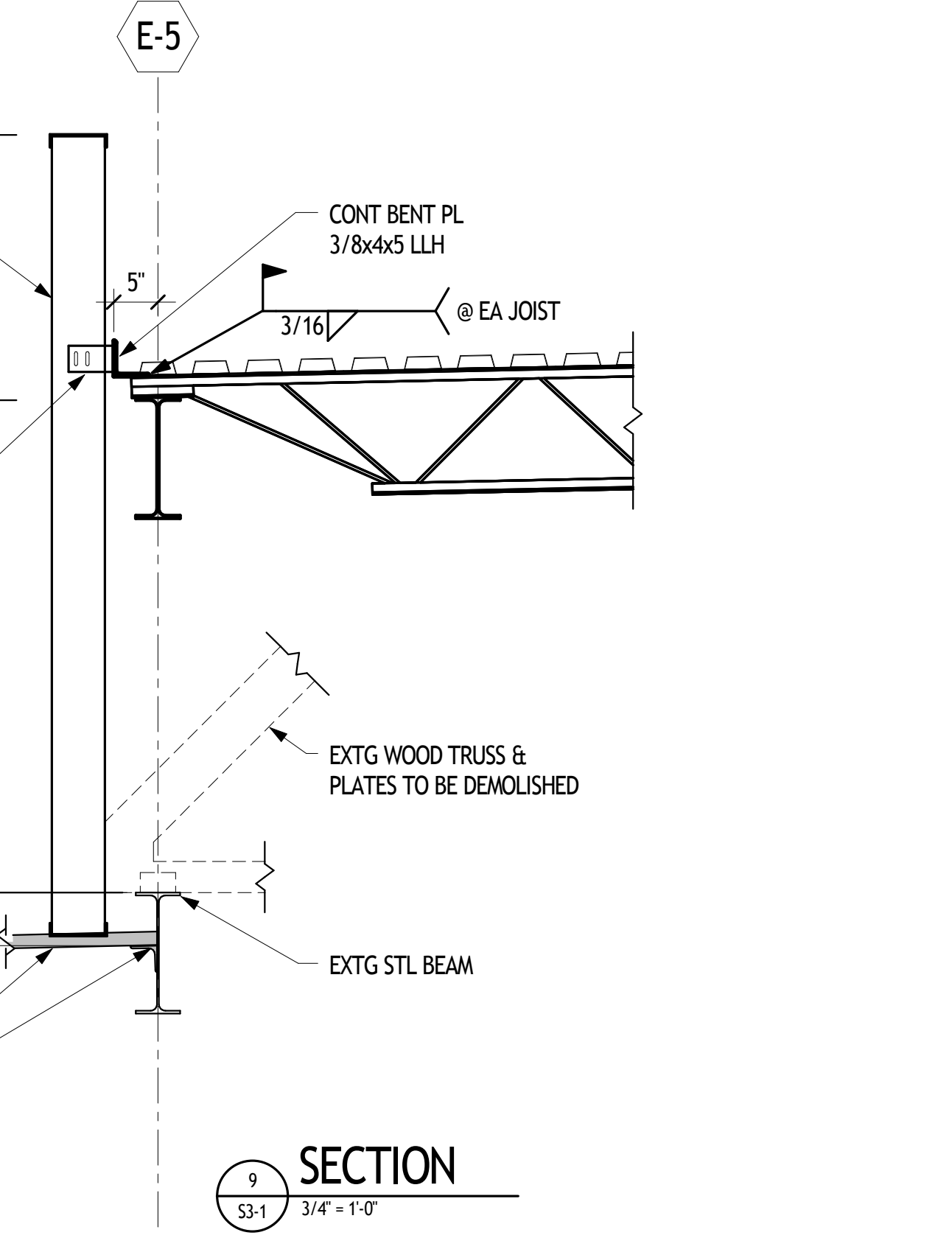
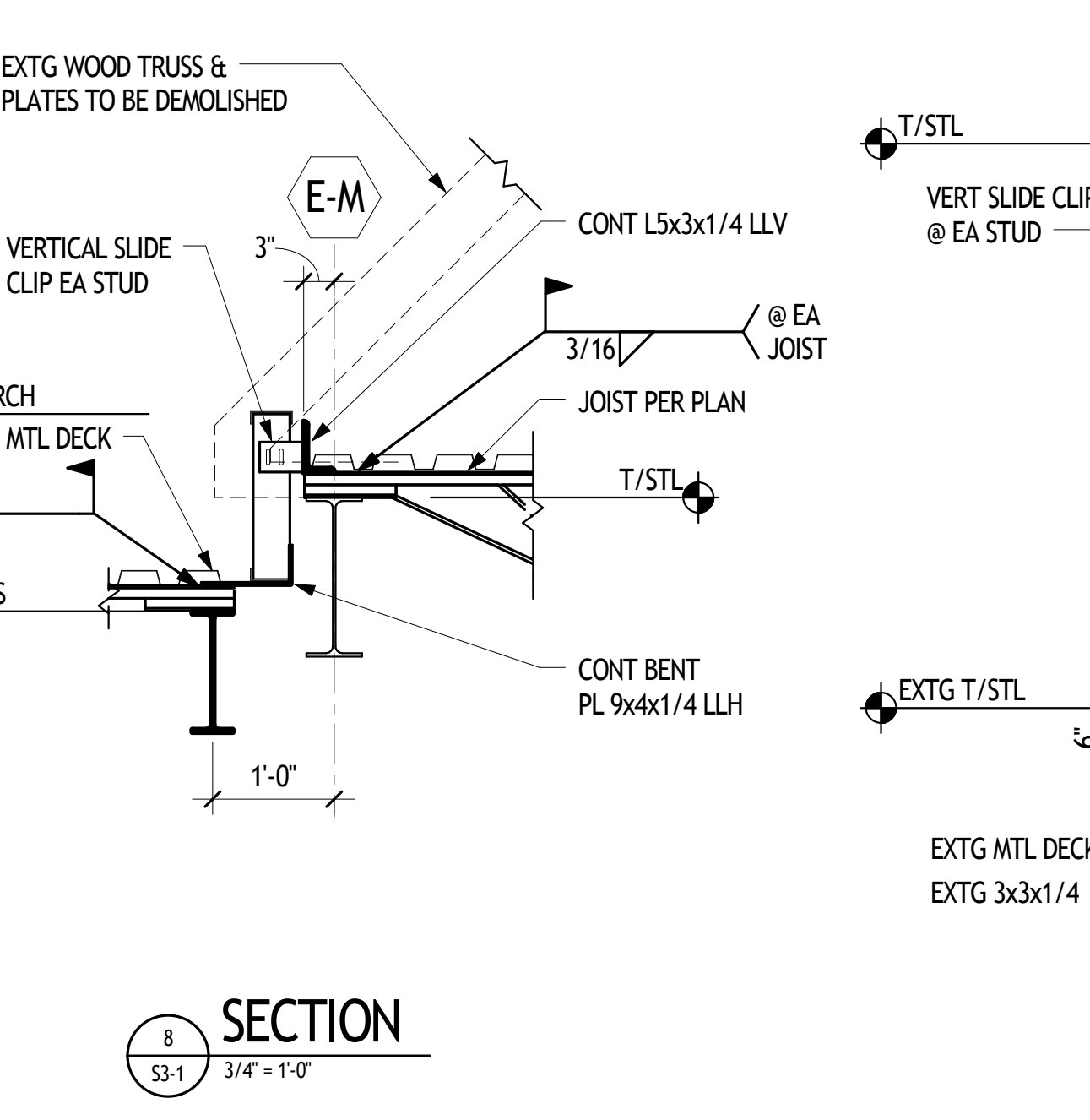
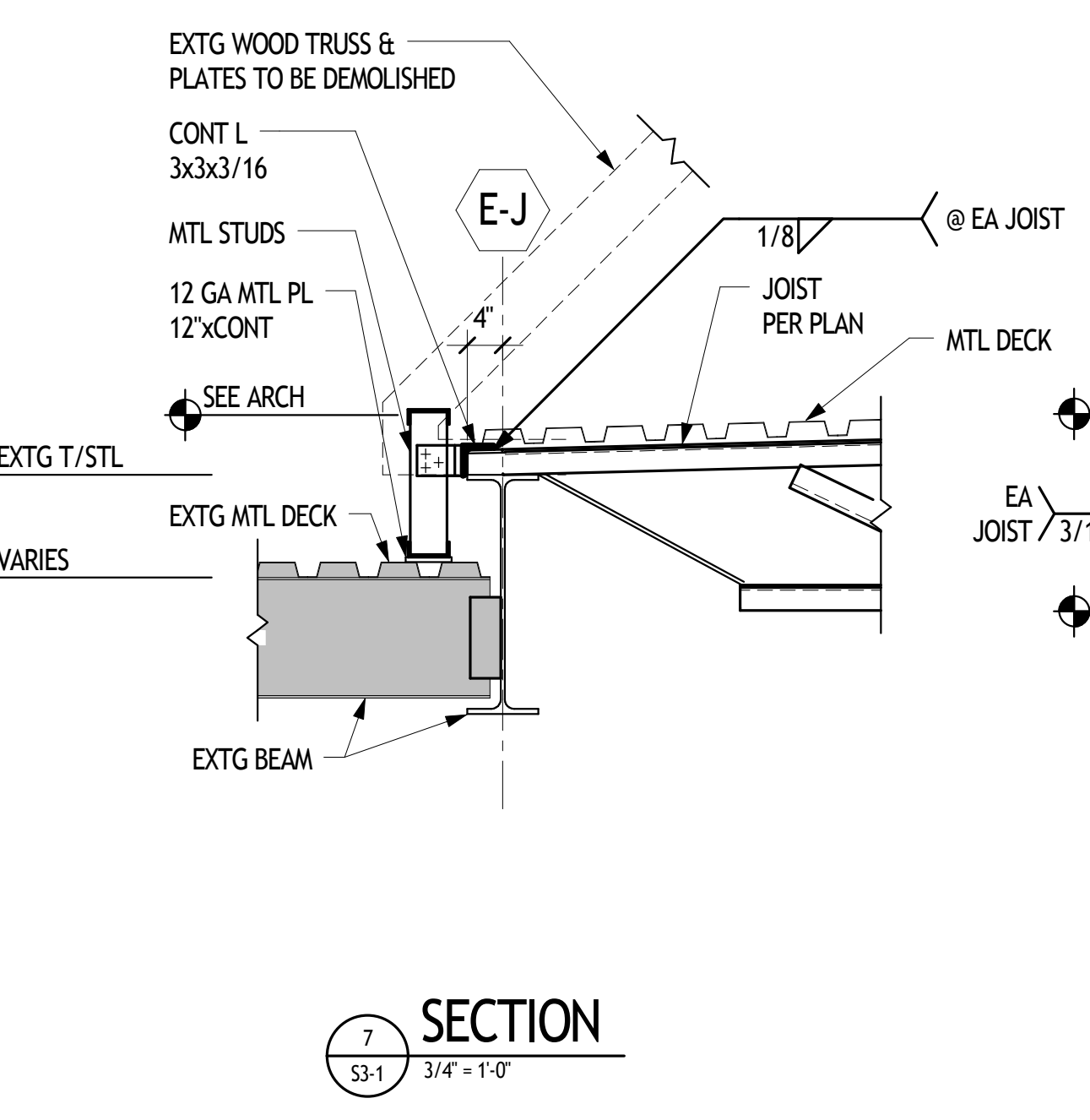
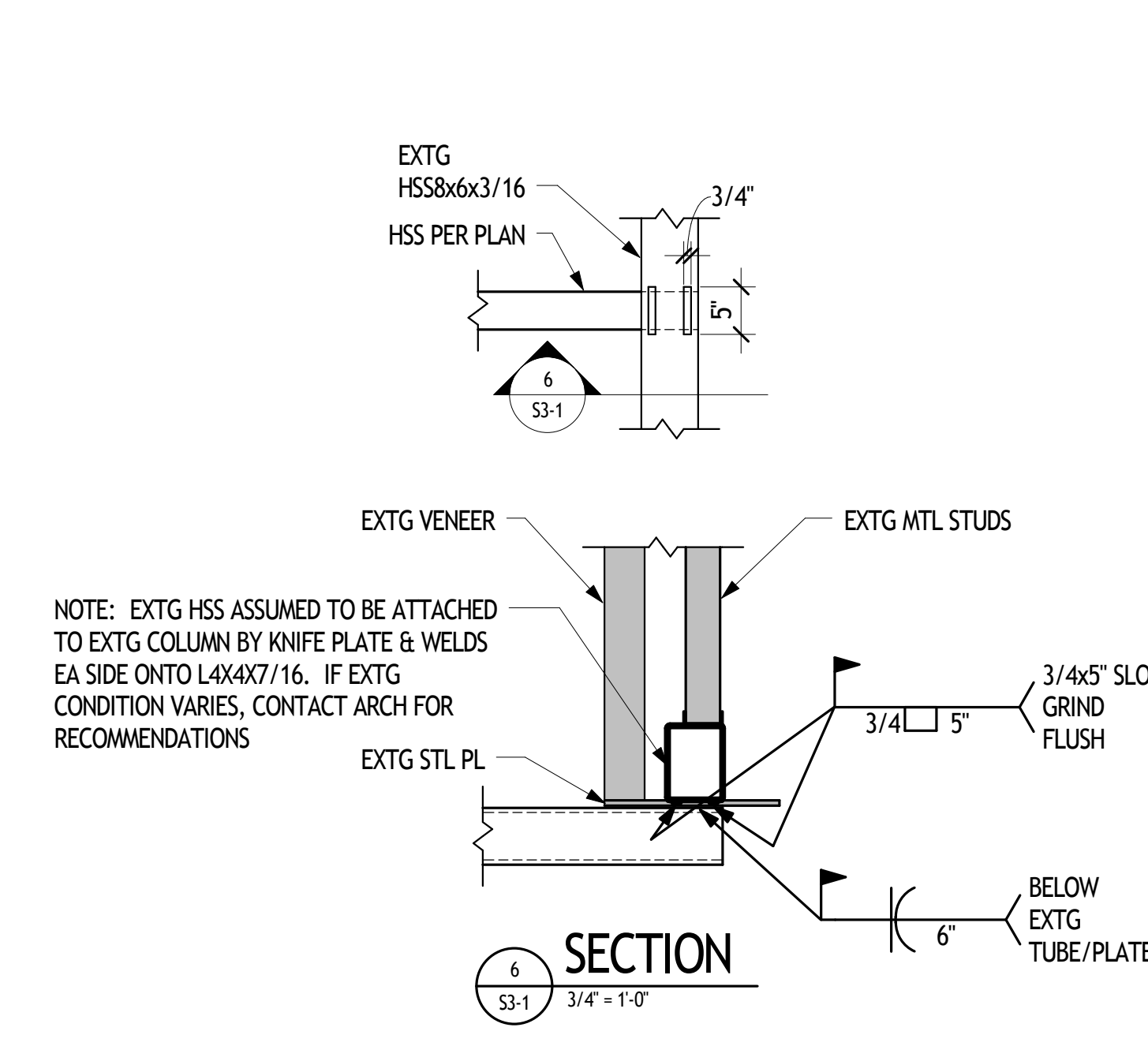
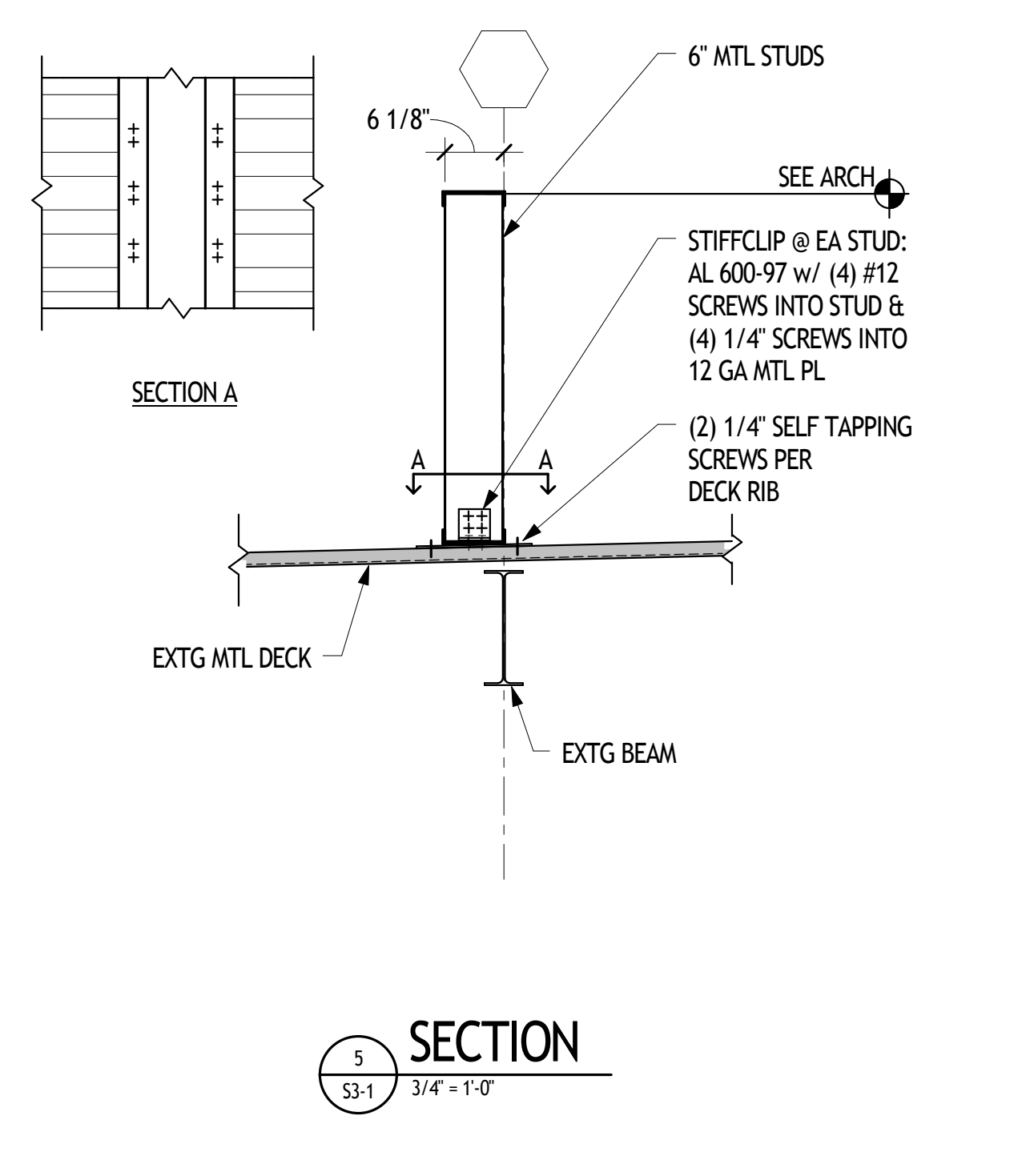
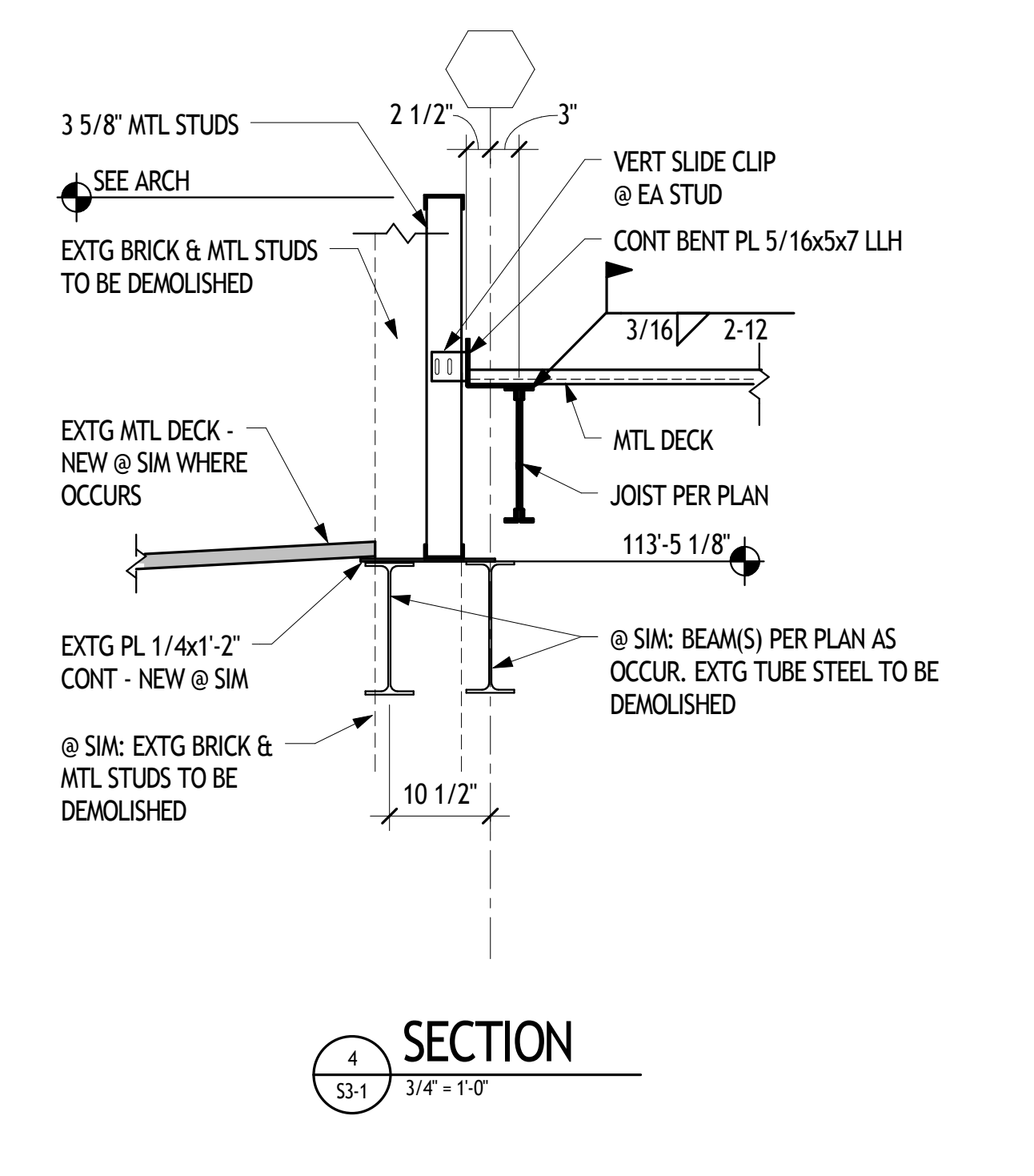
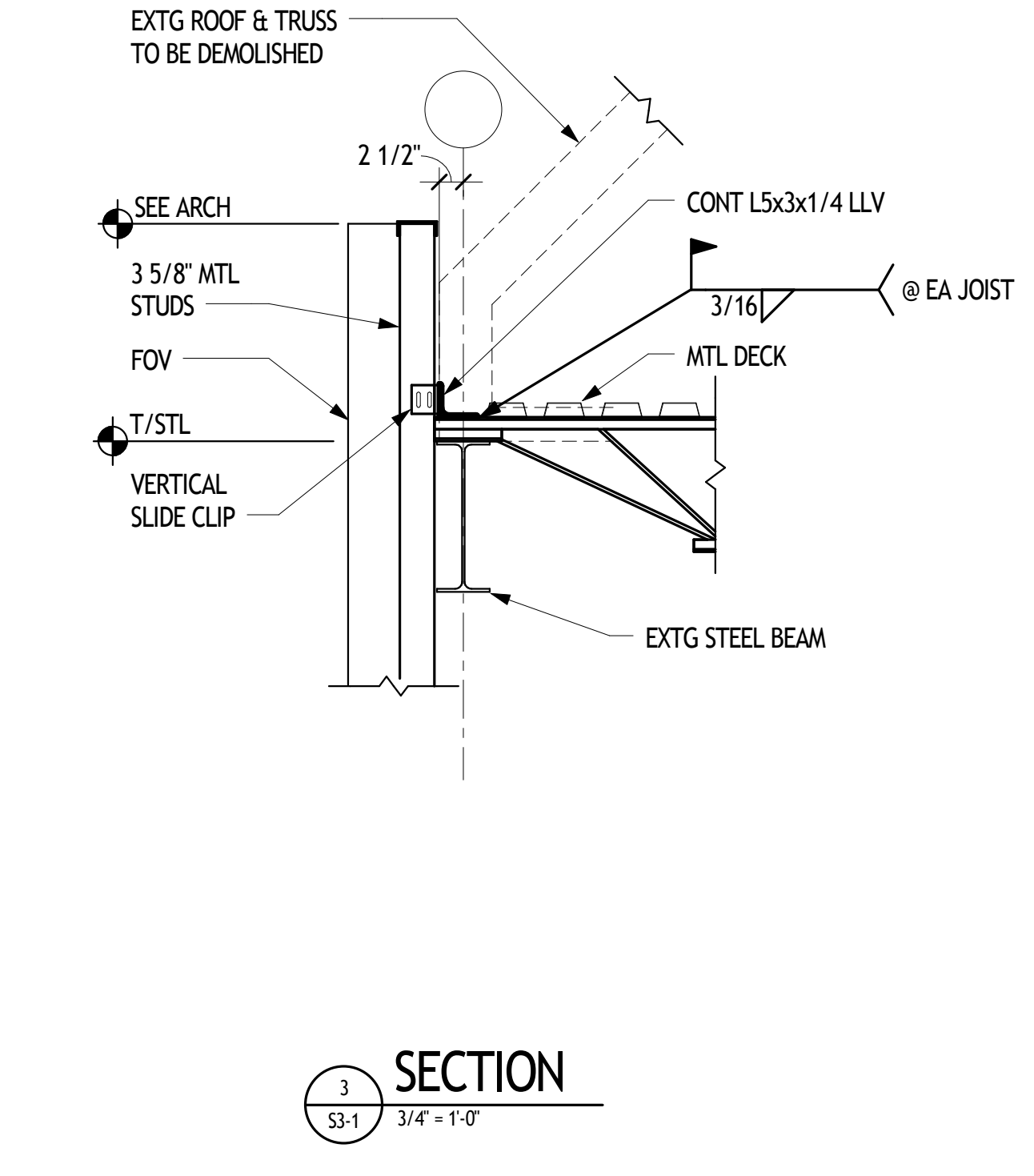
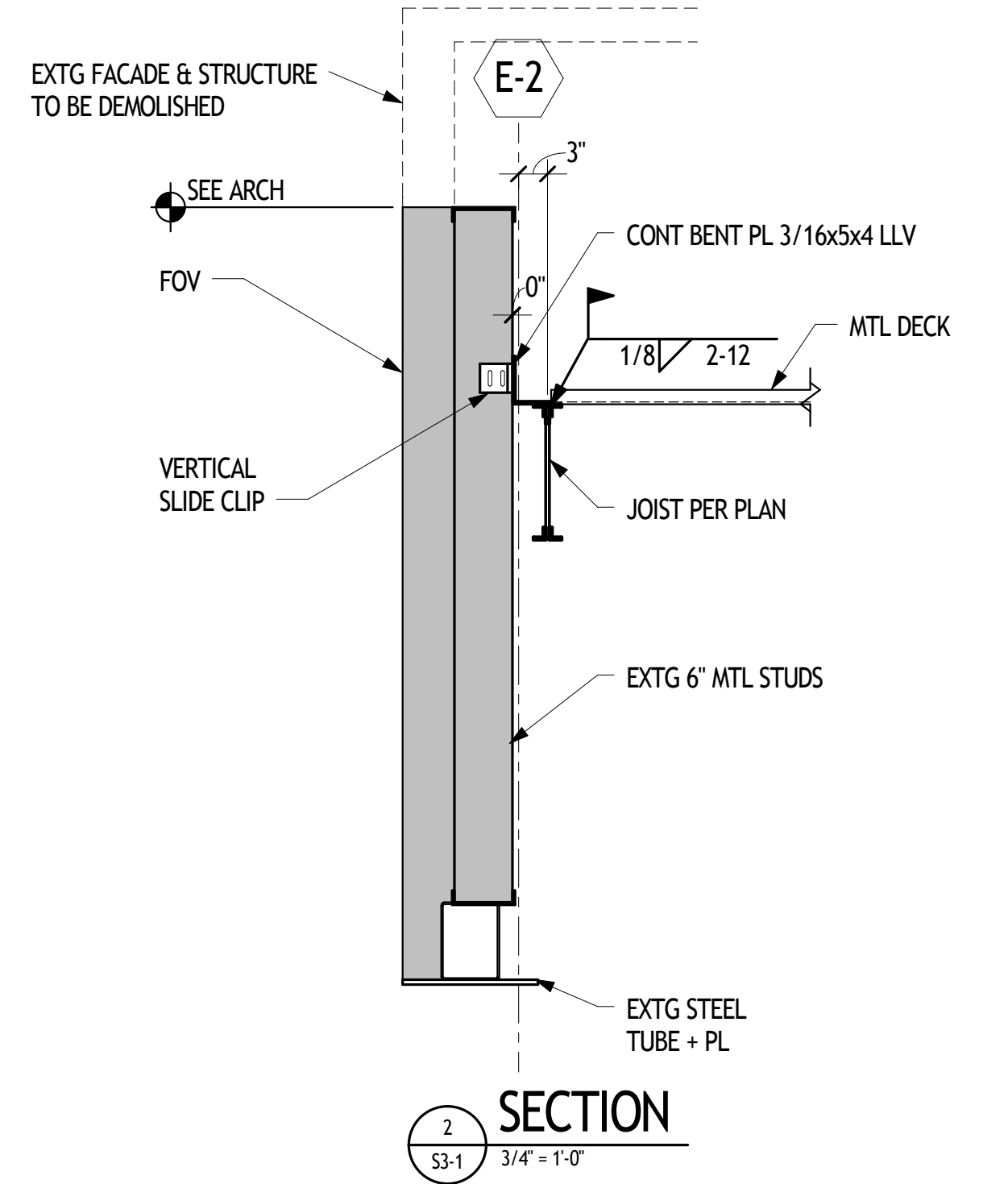
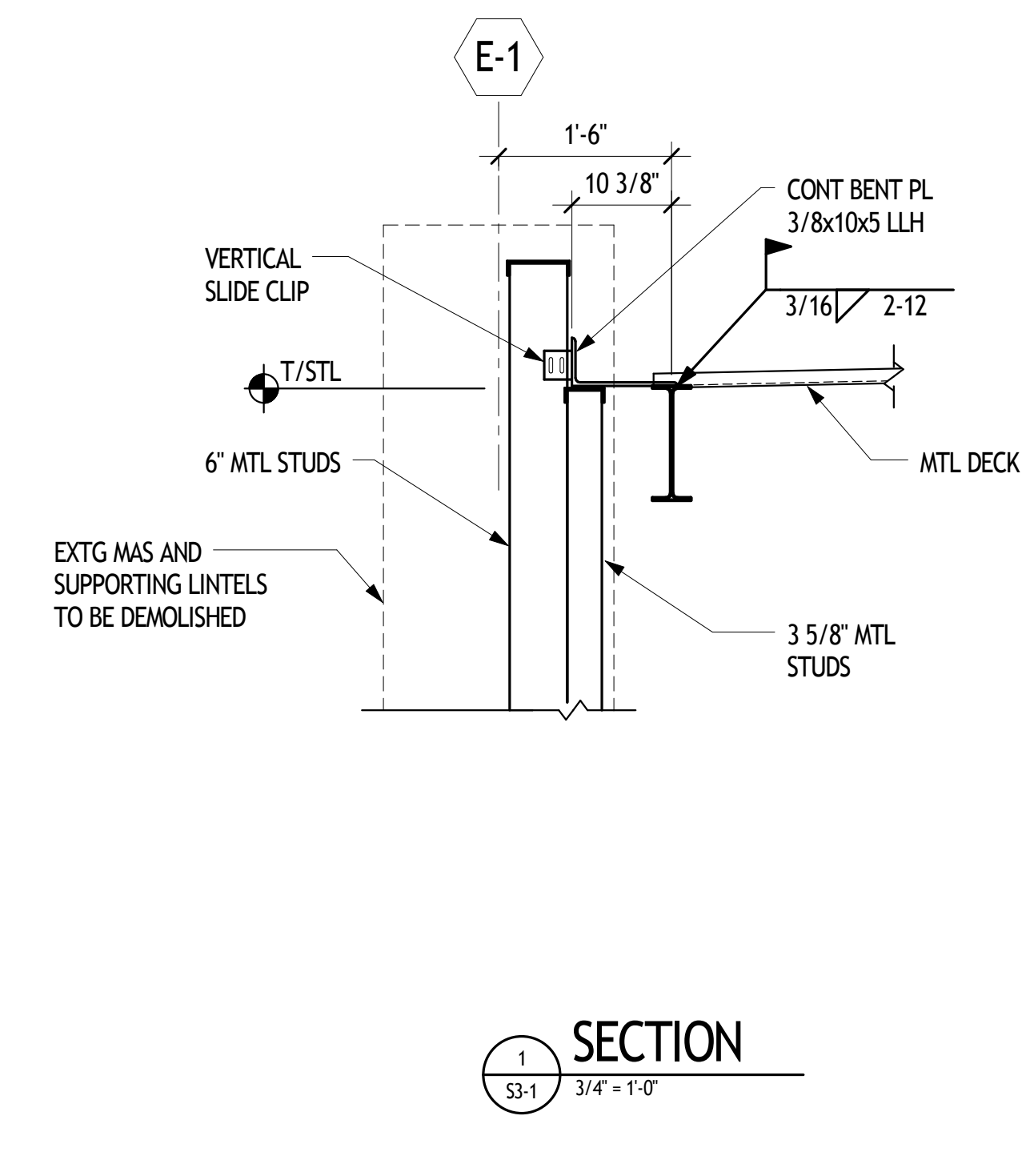
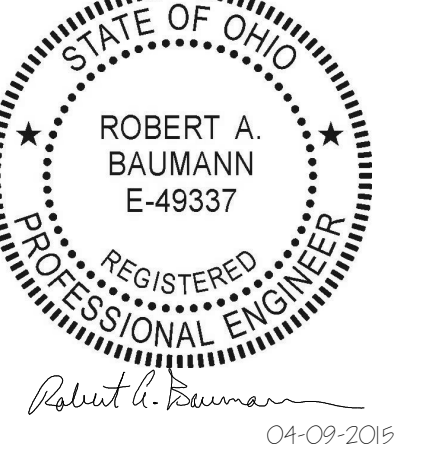
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BID DRAWINGS

S3-0



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NOTE: EXTG HSS ASSUMED TO BE ATTACHED TO EXTG COLUMN BY KNIFE PLATE & WELDS EA SIDE ONTO L4x4x7/16. IF EXTG CONDITION VARIES, CONTACT ARCH FOR RECOMMENDATIONS

NOTE: SEE 9/S3-1 FOR BALANCE OF NOTES.

NOTE: SEE 12/S3-1 FOR BALANCE OF NOTES

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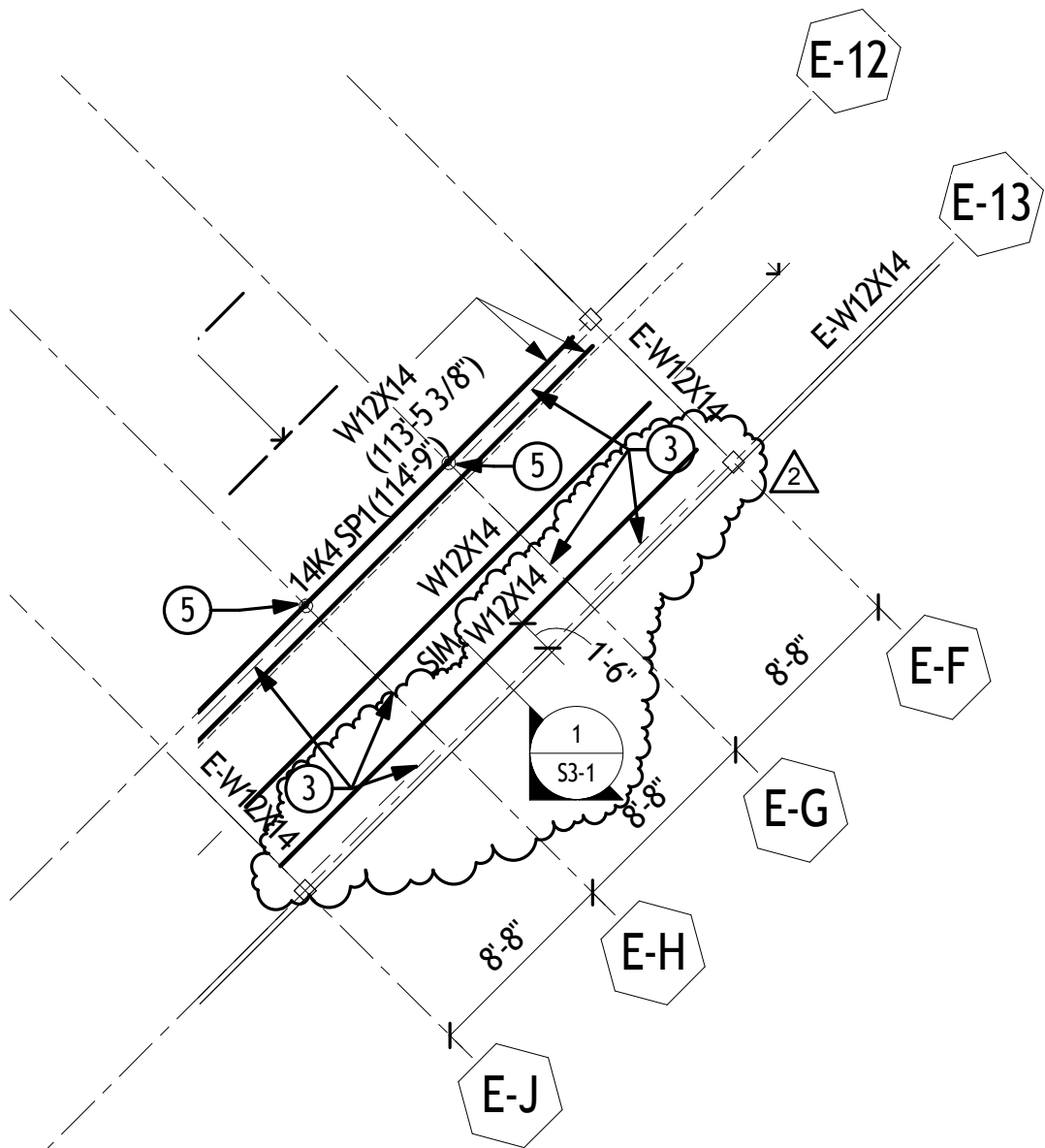
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PARTIAL LOW ROOF FRAMING PLAN

1/8" = 1'-0"

BULLETIN #1

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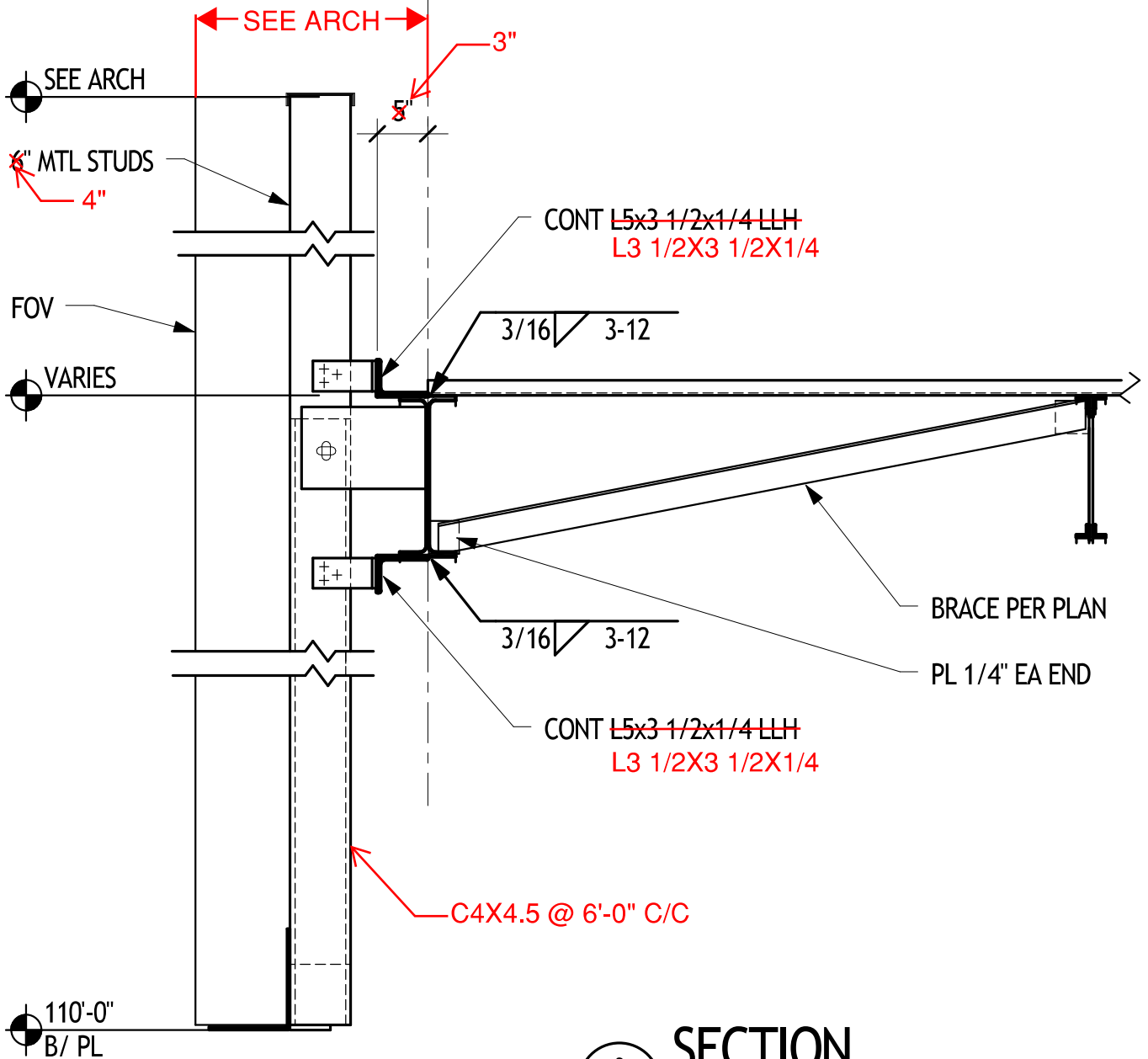


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SK1

P.8

BULLETIN #2



3
S3-0 ~~3/4" = 1'-0"~~ N.T.S.

SECTION

NOTE: SEE 5/S3-0 FOR BALANCE OF NOTES.

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